

2017 BASE CONDITIONS



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗	↕		↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	13	12	11	11	11	12
Grade (%)	-6%		2%			-5%
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	0.95		1.00	0.95
Fr _t	1.00	0.85	0.97		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1883	1631	3330		1753	3628
Flt Permitted	0.95	1.00	1.00		0.12	1.00
Satd. Flow (perm)	1883	1631	3330		217	3628
Volume (vph)	365	169	1580	343	142	1169
Peak-hour factor, PHF	0.83	0.83	0.98	0.98	0.83	0.83
Adj. Flow (vph)	440	204	1612	350	171	1408
RTOR Reduction (vph)	0	86	0	0	0	0
Lane Group Flow (vph)	440	118	1962	0	171	1408
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Turn Type		Perm			pm+pt	
Protected Phases	8		2		1	6
Permitted Phases		8			6	
Actuated Green, G (s)	21.4	21.4	26.1		39.0	39.0
Effective Green, g (s)	24.4	24.4	30.1		43.0	43.0
Actuated g/C Ratio	0.32	0.32	0.40		0.57	0.57
Clearance Time (s)	7.0	7.0	8.0		6.0	8.0
Vehicle Extension (s)	3.0	3.0	6.0		3.0	6.0
Lane Grp Cap (vph)	609	528	1329		305	2069
v/s Ratio Prot	c0.23		c0.59		0.07	c0.39
v/s Ratio Perm		0.07			0.25	
v/c Ratio	0.72	0.22	1.48		0.56	0.68
Uniform Delay, d1	22.5	18.6	22.7		32.5	11.4
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	4.2	0.2	218.5		2.4	1.4
Delay (s)	26.7	18.8	241.1		34.8	12.8
Level of Service	C	B	F		C	B
Approach Delay (s)	24.2		241.1			15.2
Approach LOS	C		F			B

Intersection Summary			
HCM Average Control Delay		122.5	HCM Level of Service F
HCM Volume to Capacity ratio		1.09	
Actuated Cycle Length (s)		75.4	Sum of lost time (s) 12.0
Intersection Capacity Utilization		92.7%	ICU Level of Service F
Analysis Period (min)		15	

c Critical Lane Group



Lane Group	WBL	WBR	NET	SBL	SBT
Lane Configurations	↖	↗	↕	↘	↕
Volume (vph)	365	169	1580	142	1169
Lane Group Flow (vph)	440	204	1962	171	1408
Turn Type	Perm		pm+pt		
Protected Phases	8		2	1	6
Permitted Phases		8		6	
Detector Phases	8	8	2	1	6
Minimum Initial (s)	1.0	1.0	15.0	1.0	15.0
Minimum Split (s)	8.0	8.0	23.0	7.0	23.0
Total Split (s)	33.0	33.0	34.0	13.0	47.0
Total Split (%)	41.3%	41.3%	42.5%	16.3%	58.8%
Yellow Time (s)	5.0	5.0	6.0	6.0	6.0
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0
Lead/Lag			Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	
Recall Mode	None	None	Min	None	Min
v/c Ratio	0.72	0.33	1.48	0.56	0.68
Control Delay	27.3	8.9	241.6	18.0	14.4
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	27.3	8.9	241.6	18.0	14.4
Queue Length 50th (ft)	180	25	-711	38	242
Queue Length 95th (ft)	247	61	#879	78	294
Internal Link Dist (ft)	1091		2024		1031
Turn Bay Length (ft)		72		175	
Base Capacity (vph)	684	673	1329	307	2070
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.64	0.30	1.48	0.56	0.68

Intersection Summary

Cycle Length: 80

Actuated Cycle Length: 75.5

Natural Cycle: 100

Control Type: Semi Act-Uncoord

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Route 314 (Eastern Leg) & Route 611

↖ ø1 13 s	↕ ø2 34 s		
↘ ø5 47 s		↘ ø8 56 s	



Lane Group	FBI	FBR	NBI	NBT	SBT	SBR
Lane Configurations	↖	↗	↖	↑↑	↑↑	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	12	12	12	12
Grade (%)	4%			7%	-6%	
Storage Length (ft)	50	0	143			0
Storage Lanes	1	1	1			0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95
Frts		0.850			0.993	
Flt Protected	0.950		0.950			
Satd. Flow (prot)	1587	1420	1708	3415	3585	0
Flt Permitted	0.950		0.950			
Satd. Flow (perm)	1587	1420	1708	3415	3585	0
Headway Factor	1.12	1.12	1.05	1.05	0.96	0.96
Link Speed (mph)	40			45	45	
Link Distance (ft)	3960			1111	2283	
Travel Time (s)	67.5			16.8	34.6	
Volume (vph)	14	197	401	1348	1114	51
Peak Hour Factor	0.67	0.67	0.95	0.95	0.76	0.76
Heavy Vehicles (%)	4%	4%	2%	2%	3%	3%
Adj. Flow (vph)	21	294	422	1419	1466	67
Lane Group Flow (vph)	21	294	422	1419	1533	0
Sign Control	Stop			Free	Free	

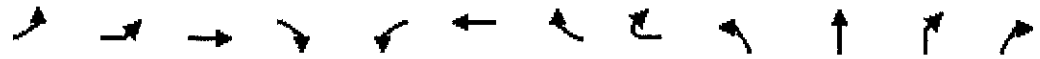
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	68.0%
	ICU Level of Service C
Analysis Period (min)	15



Movement	EBL	EBS	NBL	NET	SBI	SBR	
Lane Configurations	↙	↗	↙	↑↑	↑↑		
Sign Control	Stop			Free	Free		
Grade	4%			7%	-6%		
Volume (veh/h)	14	197	401	1348	1114	51	
Peak Hour Factor	0.67	0.67	0.95	0.95	0.76	0.76	
Hourly flow rate (vph)	21	294	422	1419	1466	67	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None						
Median storage (veh)							
Upstream signal (ft)	1111						
pX, platoon unblocked	0.68						
vC, conflicting volume	3053	766	1533				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	3556	766	1533				
tC, single (s)	6.9	7.0	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	0	14	2				
cM capacity (veh/h)	0	340	430				
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	21	294	422	709	709	977	556
Volume Left	21	0	422	0	0	0	0
Volume Right	0	294	0	0	0	0	67
cSH	0	340	430	1700	1700	1700	1700
Volume to Capacity	407.38	0.86	0.98	0.42	0.42	0.57	0.33
Queue Length 95th (ft)	Err	200	302	0	0	0	0
Control Delay (s)	Err	55.9	70.1	0.0	0.0	0.0	0.0
Lane LOS	F	F	F				
Approach Delay (s)	715.7		16.1		0.0		
Approach LOS	F						
Intersection Summary							
Average Delay	69.1						
Intersection Capacity Utilization	68.0%			ICU Level of Service		C	
Analysis Period (min)	15						

2017 Base Conditions 3: Woodland Road/Private Driveway/Stricklands Road & Route 611
 Friday P.M. Peak Hour



Volume	EBL2	EB	EBL	EBR	WB	WB2	WBR	WBR2	NBL	NB	NBL2	NBR
Lane Configurations			↕				↕			↖	↗	
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	15	15	15	15	13	13	13	13	11	12	12	12
Grade (%)			6%				5%				3%	
Total Lost time (s)			4.0				4.0			4.0	4.0	
Lane Util. Factor			1.00				1.00			1.00	0.95	
Frt			0.96				0.94			1.00	0.99	
Flt Protected			0.98				0.98			0.95	1.00	
Satd. Flow (prot)			1861				1719			1685	3445	
Flt Permitted			0.87				0.83			0.18	1.00	
Satd. Flow (perm)			1661				1460			315	3445	
Volume (vph)	5	1	3	4	107	9	94	1	6	1248	1	107
Peak-hour factor, PHF	0.56	0.56	0.56	0.56	0.89	0.89	0.89	0.89	0.99	0.99	0.99	0.99
Adj. Flow (vph)	9	2	5	7	120	10	106	1	6	1261	1	108
RTOR Reduction (vph)	0	0	5	0	0	0	0	0	0	5	0	0
Lane Group Flow (vph)	0	0	18		0	0	237		0	0	6	
Turn Type	Perm	Perm			Perm				Perm			
Protected Phases			4				8				2	
Permitted Phases	4	4			8				2			
Actuated Green, G (s)			16.4				16.4			32.3	32.3	
Effective Green, g (s)			18.4				18.4			35.8	35.8	
Actuated g/C Ratio			0.23				0.23			0.45	0.45	
Clearance Time (s)			6.0				6.0			7.5	7.5	
Vehicle Extension (s)			3.0				3.0			5.0	5.0	
Lane Grp Cap (vph)			383				337			141	1546	
v/s Ratio Prot										c0.40		
v/s Ratio Perm			0.01				c0.16			0.02		
v/c Ratio			0.05				0.70			0.04	0.88	
Uniform Delay, d1			23.9				28.2			12.4	20.1	
Progression Factor			1.00				1.00			1.00	1.00	
Incremental Delay, d2			0.0				6.5			0.3	6.8	
Delay (s)			23.9				34.7			12.6	26.9	
Level of Service			C				C			B	C	
Approach Delay (s)			23.9				34.7			26.8		
Approach LOS			C				C			C		
Intersection Summary												
HCM Average Control Delay			20.6				HCM Level of Service			C		
HCM Volume to Capacity ratio			0.78									
Actuated Cycle Length (s)			79.8				Sum of lost time (s)			16.0		
Intersection Capacity Utilization			76.7%				ICU Level of Service			D		
Analysis Period (min)			15									
c Critical Lane Group												

2017 Base Conditions 3: Woodland Road/Private Driveway/Stricklands Road & Route 611
 Friday P.M. Peak Hour



Movement	SBL1	SBL	SBL	SBL	SWL1	SWL	SWR	SWR1
Lane Configurations		↖	↕	↗		↖	↕	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	12	12	11	11	11	11
Grade (%)			-7%				-2%	
Total Lost time (s)		4.0	4.0			4.0		
Lane Util. Factor		1.00	0.95			1.00		
Friction		1.00	1.00			0.93		
Flt Protected		0.95	1.00			0.98		
Satd. Flow (prot)		1770	3658			1655		
Flt Permitted		0.10	1.00			0.98		
Satd. Flow (perm)		187	3658			1655		
Volume (vph)	1	63	1054	9	1	1	1	1
Peak-hour factor, PHF	0.82	0.82	0.82	0.82	1.00	1.00	1.00	1.00
Adj. Flow (vph)	1	77	1285	11	1	1	1	1
RTOR Reduction (vph)	0	0	0	0	0	1	0	0
Lane Group Flow (vph)	0	78	1296	0	0	3	0	0
Turn Type		pm+pt	pm+pt			Perm		
Protected Phases	1	1	6			9		
Permitted Phases	6	6			9			
Actuated Green, G (s)		42.8	42.8			1.1		
Effective Green, g (s)		46.3	46.3			3.1		
Actuated g/C Ratio		0.58	0.58			0.04		
Clearance Time (s)		5.5	7.5			6.0		
Vehicle Extension (s)		3.0	5.0			3.0		
Lane Grp Cap (vph)		237	2122			64		
v/s Ratio Prot		0.03	0.35					
v/s Ratio Perm		0.16				0.00		
v/c Ratio		0.33	0.61			0.05		
Uniform Delay, d1		13.4	10.9			36.9		
Progression Factor		1.00	1.00			1.00		
Incremental Delay, d2		0.8	0.8			0.3		
Delay (s)		14.3	11.6			37.2		
Level of Service		B	B			D		
Approach Delay (s)			11.8			37.2		
Approach LOS			B			D		

Intersection Summary

2017 Base Conditions 3: Woodland Road/Private Driveway/Stricklands Road & Route 611
 Friday P.M. Peak Hour



Lane Group	EBL2	EBL	EBT	WBL	WBT	NBL	NBL2	SBL2	SBL	SBL	SWL
Lane Configurations			↔		↔	↖	↕		↖	↕	↗
Volume (vph)	5	1	3	107	9	6	1248	1	63	1054	1
Lane Group Flow (vph)	0	0	23	0	237	6	1370	0	78	1296	4
Turn Type	Perm	Perm		Perm		Perm		pm+pt	pm+pt		
Protected Phases			4		8		2	1	1	6	9
Permitted Phases	4	4		8		2		6	6		
Detector Phases	4	4	4	8	8	2	2	1	1	6	9
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	10.0	10.0	4.0	4.0	10.0	7.0
Minimum Split (s)	13.0	13.0	13.0	13.0	13.0	17.5	17.5	9.5	9.5	17.5	13.0
Total Split (s)	33.0	33.0	33.0	33.0	33.0	36.0	36.0	12.0	12.0	48.0	19.0
Total Split (%)	33.0%	33.0%	33.0%	33.0%	33.0%	36.0%	36.0%	12.0%	12.0%	48.0%	19.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	5.5	5.5	5.5	5.5	5.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	0.0	2.0	2.0
Lead/Lag						Lag	Lag	Lead	Lead		
Lead-Lag Optimize?						Yes	Yes	Yes	Yes		
Recall Mode	None	None	None	None	None	Min	Min	None	None	Min	None
v/c Ratio			0.06		0.65	0.06	0.82		0.28	0.58	0.02
Control Delay			17.1		27.6	19.0	25.4		11.3	12.2	34.0
Queue Delay			0.0		0.0	0.0	0.0		0.0	0.0	0.0
Total Delay			17.1		27.6	19.0	25.4		11.3	12.2	34.0
Queue Length 50th (ft)			5		90	1	267		12	150	1
Queue Length 95th (ft)			14		189	12	650		46	349	12
Internal Link Dist (ft)			105		2012		2203			2327	625
Turn Bay Length (ft)						73			183		
Base Capacity (vph)			574		507	108	1675		288	2241	277
Starvation Cap Reductn			0		0	0	0		0	0	0
Spillback Cap Reductn			0		0	0	0		0	0	0
Storage Cap Reductn			0		0	0	0		0	0	0
Reduced v/c Ratio			0.04		0.47	0.06	0.82		0.27	0.58	0.01

Intersection Summary

Cycle Length: 100
 Actuated Cycle Length: 73.8
 Natural Cycle: 80
 Control Type: Semi-Act-Uncoord
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

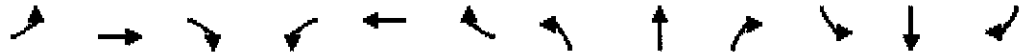
Splits and Phases: 3: Woodland Road/Private Driveway/Stricklands Road & Route 611

↖ ø1 12 s	↕ ø2 33 s	↗ ø4 33 s	↘ ø9 19 s
↘ ø6 48 s	↖ ø8 33 s		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↙	↕	↕	↙	↕	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	13	13	12	12	12	11	12	12	11	12	12
Grade (%)	2%			8%			1%			-1%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	0.865			0.883								
F/I Protected					0.997		0.950			0.950		
Satd. Flow (prot)	0	1648	0	0	1574	0	1702	3522	0	1670	3455	0
F/I Permitted					0.997		0.950			0.950		
Satd. Flow (perm)	0	1648	0	0	1574	0	1702	3522	0	1670	3455	0
Headway Factor	0.97	0.97	0.97	1.05	1.05	1.05	1.05	1.01	1.01	1.04	0.99	0.99
Link Speed (mph)	35			35			45			45		
Link Distance (ft)	158			1027			2407			3261		
Travel Time (s)	3.1			20.0			36.5			49.4		
Volume (vph)	0	0	4	1	1	13	4	1342	1	6	1121	1
Peak Hour Factor	0.75	0.75	0.75	0.50	0.50	0.50	0.97	0.97	0.97	0.83	0.83	0.83
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	5%	5%	5%
Adj. Flow (vph)	0	0	5	2	2	26	4	1384	1	7	1351	1
Lane Group Flow (vph)	0	5	0	0	30	0	4	1385	0	7	1352	0
Sign Control	Stop			Stop			Free			Free		

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	47.1%
ICU Level of Service	A
Analysis Period (min)	15



Movement	EB1	EB2	EB3	WB1	WB2	WB3	NB1	NB2	NB3	SB1	SB2	SB3
Lane Configurations	↕			↕			↗	↕		↗	↕	
Sign Control	Stop			Stop			Free		Free			
Grade	2%			8%			1%		-1%			
Volume (veh/h)	0	0	4	1	1	13	4	1342	1	6	1121	1
Peak Hour Factor	0.75	0.75	0.75	0.50	0.50	0.50	0.97	0.97	0.97	0.83	0.83	0.83
Hourly flow rate (vph)	0	0	5	2	2	26	4	1384	1	7	1351	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2093	2758	676	2087	2759	692	1352			1385		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2093	2758	676	2087	2759	692	1352			1385		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.2		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	93	89	93	99			98		
cM capacity (veh/h)	25	19	396	29	19	386	505			475		
Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	5	30	4	922	462	7	900	451				
Volume Left	0	2	4	0	0	7	0	0				
Volume Right	5	26	0	0	1	0	0	1				
cSH	396	123	505	1700	1700	475	1700	1700				
Volume to Capacity	0.01	0.24	0.01	0.54	0.27	0.02	0.53	0.27				
Queue Length 95th (ft)	1	22	1	0	0	1	0	0				
Control Delay (s)	14.2	43.4	12.2	0.0	0.0	12.7	0.0	0.0				
Lane LOS	B	E	B			B						
Approach Delay (s)	14.2	43.4	0.0			0.1						
Approach LOS	B	E										
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			47.1%		ICU Level of Service		A					
Analysis Period (min)	15											



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↖	↕↖		↖	↕↖	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	10	10	10	11	12	12	11	12	12
Grade (%)	0%			9%			2%			-5%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	0.938			0.934			0.998			0.996		
Flt Protected	0.978			0.983			0.950			0.950		
Satd. Flow (prot)	0	1709	0	0	1524	0	1694	3497	0	1753	3613	0
Flt Permitted	0.978			0.983			0.950			0.950		
Satd. Flow (perm)	0	1709	0	0	1524	0	1694	3497	0	1753	3613	0
Headway Factor	1.00	1.00	1.00	1.16	1.16	1.16	1.06	1.01	1.01	1.01	0.97	0.97
Link Speed (mph)	30			30			45			45		
Link Distance (ft)	294			1492			3261			2754		
Travel Time (s)	6.7			33.9			49.4			41.7		
Volume (vph)	20	4	20	13	7	18	31	1304	20	22	1095	31
Peak Hour Factor	0.90	0.90	0.90	0.75	0.90	0.75	0.90	0.97	0.97	0.77	0.77	0.90
Adj. Flow (vph)	22	4	22	17	8	24	34	1344	21	29	1422	34
Lane Group Flow (vph)	0	48	0	0	49	0	34	1365	0	29	1456	0
Sign Control	Stop			Stop			Free			Free		

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 47.1% ICU Level of Service A

Analysis Period (min) 15



Movement	EB1	EB2	EBR	WB1	WB2	WBR	NB1	NB2	NBR	SB1	SB2	SB3
Lane Configurations	↕			↕			↖	↕		↖	↕	
Sign Control	Stop			Stop			Free			Free		
Grade	0%			9%			2%			-5%		
Volume (veh/h)	20	4	20	13	7	18	31	1304	20	22	1095	31
Peak Hour Factor	0.90	0.90	0.90	0.75	0.90	0.75	0.90	0.97	0.97	0.77	0.77	0.90
Hourly flow rate (vph)	22	4	22	17	8	24	34	1344	21	29	1422	34
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2265	2930	728	2216	2937	682	1457			1365		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2265	2930	728	2216	2937	682	1457			1365		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	66	94	0	38	94	93			94		
CM capacity (veh/h)	10	13	366	15	13	391	460			499		

Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	49	49	34	896	469	29	948	508
Volume Left	22	17	34	0	0	29	0	0
Volume Right	22	24	0	0	21	0	0	34
cSH	18	27	460	1700	1700	499	1700	1700
Volume to Capacity	2.71	1.84	0.07	0.53	0.28	0.06	0.56	0.30
Queue Length 95th (ft)	166	148	6	0	0	5	0	0
Control Delay (s)	1213.2	711.5	13.5	0.0	0.0	12.6	0.0	0.0
Lane LOS	F	F	B			B		
Approach Delay (s)	1213.2	711.5	0.3			0.2		
Approach LOS	F	F						

Intersection Summary		
Average Delay		31.9
Intersection Capacity Utilization	47.1%	ICU Level of Service
Analysis Period (min)		15
		A



Lane Group	EBL	EBF	WBL	WBT	NBL	NBR
Lane Configurations	↔		↔		↔	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	10	10	11	11
Grade (%)	1%			-3%	0%	
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.964				0.962	
Flt Protected			0.994		0.965	
Satd. Flow (prot)	1590	0	0	1688	1672	0
Flt Permitted			0.994		0.965	
Satd. Flow (perm)	1590	0	0	1688	1672	0
Headway Factor	1.10	1.10	1.07	1.07	1.04	1.04
Link Speed (mph)	40		40		25	
Link Distance (ft)	2092		650		499	
Travel Time (s)	35.7		11.1		13.6	
Volume (vph)	126	47	20	150	60	24
Peak Hour Factor	0.82	0.82	0.79	0.79	0.53	0.53
Heavy Vehicles (%)	7%	7%	6%	6%	2%	2%
Adj. Flow (vph)	154	57	25	190	113	45
Lane Group Flow (vph)	211	0	0	215	158	0
Sign Control	Free		Free		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	33.3% ICU Level of Service A
Analysis Period (min)	15



Movement	EB	EB	WB	WB	NB	NB
Lane Configurations	↕		↕		↕	
Sign Control	Free			Free Stop		
Grade	1%			-3%		0%
Volume (veh/h)	126	47	20	150	60	24
Peak Hour Factor	0.82	0.82	0.79	0.79	0.53	0.53
Hourly flow rate (vph)	154	57	25	190	113	45
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			211		423	182
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			211		423	182
tC, single (s)			4.2		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.3		3.5	3.3
p0 queue free %			98		80	95
cM capacity (veh/h)			1336		577	860

Direction Lane #	EB	WB	NB
Volume Total	211	215	158
Volume Left	0	25	113
Volume Right	57	0	45
cSH	1700	1336	637
Volume to Capacity	0.12	0.02	0.25
Queue Length 95th (ft)	0	1	24
Control Delay (s)	0.0	1.1	12.5
Lane LOS		A	B
Approach Delay (s)	0.0	1.1	12.5
Approach LOS			B

Intersection Summary			
Average Delay	3.8		
Intersection Capacity Utilization	33.3%	ICU Level of Service	A
Analysis Period (min)	15		

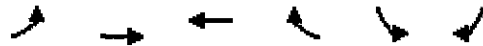


Lane Group	EBL	EBR	WBL	WBR	NBL	NBR
Lane Configurations	↕		↕		↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	10	10
Grade (%)	1%		1%		-1%	
Turning Speed (mph)	9		15		15	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.970				0.964	
Flt Protected			0.997		0.965	
Satd. Flow (prot)	1738	0	0	1752	1625	0
Flt Permitted			0.997		0.965	
Satd. Flow (perm)	1738	0	0	1752	1625	0
Headway Factor	1.05	1.05	1.05	1.05	1.09	1.09
Link Speed (mph)	40		40		35	
Link Distance (ft)	650		936		704	
Travel Time (s)	11.1		16.0		13.7	
Volume (vph)	117	33	10	132	38	14
Peak Hour Factor	0.72	0.72	0.80	0.80	0.25	0.25
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	163	46	13	165	152	56
Lane Group Flow (vph)	208	0	0	177	208	0
Sign Control	Free		Free		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	25.2%
	ICU Level of Service A
Analysis Period (min)	15

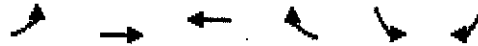


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔		↔		↔	
Sign Control	Free		Free		Stop	
Grade	1%		1%		-1%	
Volume (veh/h)	117	33	10	132	38	14
Peak Hour Factor	0.72	0.72	0.80	0.80	0.25	0.25
Hourly flow rate (vph)	162	46	12	165	152	56
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			208			375 185
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			208			375 185
IC, single (s)			4.1			6.4 6.2
tC, 2 stage (s)						
IF (s)			2.2			3.5 3.3
p0 queue free %			99			75 93
cM capacity (veh/h)			1351			620 857
Direction Lane #						
	EB	WB	NB			
Volume Total	208	178	208			
Volume Left	0	12	152			
Volume Right	46	0	56			
cSH	1700	1351	670			
Volume to Capacity	0.12	0.01	0.31			
Queue Length 95th (ft)	0	1	33			
Control Delay (s)	0.0	0.6	12.8			
Lane LOS			A			B
Approach Delay (s)	0.0	0.6	12.8			
Approach LOS			B			
Intersection Summary						
Average Delay			4.7			
Intersection Capacity Utilization			25.2%	ICU Level of Service		A
Analysis Period (min)			15			



Lane Group	EBL	EBT	WBL	WBT	SEB	SEB
Lane Configurations		↕	↕		↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	10	10
Grade (%)		-4%	2%		-6%	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.999		0.932	
Frt Protected		0.999			0.976	
Satd. Flow (prot)	0	1835	1747	0	1629	0
Frt Permitted		0.999			0.976	
Satd. Flow (perm)	0	1835	1747	0	1629	0
Headway Factor	1.02	1.02	1.06	1.06	1.05	1.05
Link Speed (mph)		40	40		35	
Link Distance (ft)		936	819		1342	
Travel Time (s)		16.0	14.0		26.1	
Volume (vph)	3	128	139	1	3	3
Peak Hour Factor	0.84	0.84	0.77	0.77	0.50	0.50
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	4	152	181	1	6	6
Lane Group Flow (vph)	0	156	182	0	12	0
Sign Control		Free	Free		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	19.1%
ICU Level of Service	A
Analysis Period (min)	15



Movement	EB	WB	SB
Lane Configurations	↔	↔	↔
Sign Control	Free	Free	Stop
Grade	-4%	2%	-6%
Volume (veh/h)	3	128	139
Peak Hour Factor	0.84	0.84	0.77
Hourly flow rate (vph)	4	152	181
Pedestrians			
Lane Width (ft)			
Walking Speed (ft/s)			
Percent Blockage			
Right turn flare (veh)			
Median type			None
Median storage (veh)			
Upstream signal (ft)			
pX, platoon unblocked			
vC, conflicting volume	182		341
vC1, stage 1 conf vol			181
vC2, stage 2 conf vol			
vCu, unblocked vol	182		341
tC, single (s)	4.1		6.4
tC, 2 stage (s)			6.2
tF (s)	2.2		3.5
p0 queue free %	100		99
cM capacity (veh/h)	1393		654
Direction Lane #	EB 1	WB 1	SB 1
Volume Total	156	182	12
Volume Left	4	0	6
Volume Right	0	1	6
cSH	1393	1700	744
Volume to Capacity	0.00	0.11	0.02
Queue Length 95th (ft)	0	0	1
Control Delay (s)	0.2	0.0	9.9
Lane LOS	A		A
Approach Delay (s)	0.2	0.0	9.9
Approach LOS			A
Intersection Summary			
Average Delay		0.4	
Intersection Capacity Utilization		19.1%	ICU Level of Service
Analysis Period (min)		15	A



Lane Group	EB	EBR	NB	NBT	SB	SBR
Lane Configurations	↘		↑		↙	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	-3%		-5%		4%	
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.992			0.938		
Flt Protected	0.955			0.998		
Satd. Flow (prot)	1672	0	0	1842	1639	0
Flt Permitted	0.955			0.998		
Satd. Flow (perm)	1672	0	0	1842	1639	0
Headway Factor	1.07	1.07	1.01	1.01	1.07	1.07
Link Speed (mph)	40		45		45	
Link Distance (ft)	1794		1439		1446	
Travel Time (s)	30.6		21.8		21.9	
Volume (vph)	124	7	7	190	159	133
Peak Hour Factor	0.82	0.82	0.96	0.96	0.81	0.81
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Adj. Flow (vph)	151	9	7	198	196	164
Lane Group Flow (vph)	160	0	0	205	360	0
Sign Control	Stop			Free	Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	30.5%
	ICU Level of Service A
Analysis Period (min)	15



Mo/Opent	EBL	EBR	NBL	NBT	SEB	SEB
Lane Configurations	Y			↑	↑	
Sign Control	Stop			Free	Free	
Grade	-3%			-5%	4%	
Volume (veh/h)	124	7	7	190	159	133
Peak Hour Factor	0.82	0.82	0.96	0.96	0.81	0.81
Hourly flow rate (vph)	151	9	7	198	196	164
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	491	278	360			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	491	278	360			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	72	99	99			
cM capacity (veh/h)	534	761	1198			
Direction Lane #	EBL	NBL	SEB			
Volume Total	160	205	360			
Volume Left	151	7	0			
Volume Right	9	0	164			
cSH	543	1198	1700			
Volume to Capacity	0.29	0.01	0.21			
Queue Length 95th (ft)	31	0	0			
Control Delay (s)	14.4	0.3	0.0			
Lane LOS	B	A				
Approach Delay (s)	14.4	0.3	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			3.3			
Intersection Capacity Utilization			30.5%	ICU Level of Service	A	
Analysis Period (min)			15			



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NB	NBT	NBL	SB	SBT	SBR	
Lane Configurations	↕			↕			↕			↕			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	11	11	11	11	11	11	11	11	11	8	8	8	
Grade (%)		-4%			4%			1%			-1%		
Total Lost time (s)	4.0			4.0			4.0			4.0			
Lane Util. Factor	1.00			1.00			1.00			1.00			
Frt	0.99			1.00			0.88			0.96			
Flt Protected	1.00			0.98			0.99			0.99			
Satd. Flow (prot)	1815			1738			1543			1551			
Flt Permitted	1.00			0.52			0.95			0.83			
Satd. Flow (perm)	1808			921			1473			1299			
Volume (vph)	4	578	53	230	523	1	41	3	269	3	9	4	
Peak-hour factor, PHF	0.86	0.86	0.86	0.98	0.98	0.98	0.87	0.87	0.87	0.60	0.60	0.60	
Adj. Flow (vph)	5	672	62	235	534	1	47	3	309	5	15	7	
RTOR Reduction (vph)	0	4	0	0	0	0	0	269	0	0	6	0	
Lane Group Flow (vph)	0	735	0	0	770	0	0	90	0	0	21	0	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	4%	4%	4%	2%	2%	2%	
Turn Type	Perm			pm+pt			Perm			Perm			
Protected Phases	2			1			6			8			
Permitted Phases	2			6			8			4			
Actuated Green, G (s)	42.1			61.1			9.7			9.7			
Effective Green, g (s)	44.1			63.1			10.7			10.7			
Actuated g/C Ratio	0.54			0.77			0.13			0.13			
Clearance Time (s)	6.0			6.0			5.0			5.0			
Vehicle Extension (s)	6.0			6.0			3.0			3.0			
Lane Grp Cap (vph)	975			860			193			170			
v/s Ratio Prot				c0.16									
v/s Ratio Perm	0.41			c0.53			c0.06			0.02			
v/c Ratio	0.75			0.90			0.47			0.12			
Uniform Delay, d1	14.6			6.9			32.9			31.4			
Progression Factor	1.00			1.00			1.00			1.00			
Incremental Delay, d2	4.4			13.0			1.8			0.3			
Delay (s)	19.1			19.9			34.7			31.7			
Level of Service	B			B			C			C			
Approach Delay (s)	19.1			19.9			34.7			31.7			
Approach LOS	B			B			C			C			

Intersection Summary			
HCM Average Control Delay	22.6	HCM Level of Service	C
HCM Volume to Capacity ratio	0.82		
Actuated Cycle Length (s)	81.8	Sum of lost time (s)	8.0
Intersection Capacity Utilization	107.3%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group



Lane Group	EB	WB	NB	SB
Lane Configurations	↕		↕	
Volume (vph)	4	578	230	523
Lane Group Flow (vph)	0	739	0	770
Turn Type	Perm	pm+pt	Perm	Perm
Protected Phases	2	1	6	8
Permitted Phases	2	6	8	4
Detector Phases	2	2	1	6
Minimum Initial (s)	10.0	10.0	4.0	10.0
Minimum Split (s)	16.0	16.0	10.0	16.0
Total Split (s)	48.0	48.0	19.0	67.0
Total Split (%)	55.2%	55.2%	21.8%	77.0%
Yellow Time (s)	4.5	4.5	4.0	4.5
All-Red Time (s)	1.5	1.5	2.0	1.5
Lead/Lag	Lag	Lag	Lead	
Lead-Lag Optimize?	Yes	Yes	Yes	
Recall Mode	Min	Min	None	Min
v/c Ratio		0.76		0.94
Control Delay		21.8		28.5
Queue Delay		0.0		0.0
Total Delay		21.8		28.5
Queue Length 50th (ft)		264		114
Queue Length 95th (ft)		439		457
Internal Link Dist (ft)		1322		1070
Turn Bay Length (ft)				
Base Capacity (vph)		978		821
Starvation Cap Reductn		0		0
Spillback Cap Reductn		0		0
Storage Cap Reductn		0		0
Reduced v/c Ratio		0.76		0.94

Inter-Section Summary

Cycle Length: 87
 Actuated Cycle Length: 81.9
 Natural Cycle: 75
 Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 940 & Carlton Road/Private Driveway

↙ φ1 18%	↔ φ2 48%	↘ φ4 20%
↙ φ6 67%		↗ φ8 20%



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	11	11	11	11	14	14
Grade (%)		-2%	2%		-4%	
Total Lost time (s)		4.0	4.0		4.0	
Lane Util. Factor		1.00	1.00		1.00	
Fr't		1.00	0.99		0.87	
Flt Protected		0.97	1.00		1.00	
Satd. Flow (prot)		1765	1770		1724	
Flt Permitted		0.45	1.00		1.00	
Satd. Flow (perm)		815	1770		1724	
Volume (vph)	513	337	326	18	12	428
Peak-hour factor, PHF	0.89	0.89	0.93	0.93	0.80	0.80
Adj. Flow (vph)	576	379	351	19	15	535
RTOR Reduction (vph)	0	0	2	0	480	0
Lane Group Flow (vph)	0	955	368	0	70	0
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%
Turn Type	pm+pt					
Protected Phases	5	2	6		4	
Permitted Phases	2					
Actuated Green, G (s)		81.0	45.0		8.4	
Effective Green, g (s)		83.0	47.0		10.4	
Actuated g/C Ratio		0.82	0.46		0.10	
Clearance Time (s)		6.0	6.0		6.0	
Vehicle Extension (s)		6.0	6.0		3.0	
Lane Grp Cap (vph)		967	820		177	
v/s Ratio Prot		c0.31	0.21		c0.04	
v/s Ratio Perm		c0.50				
v/c Ratio		0.99	0.45		0.39	
Uniform Delay, d1		8.7	18.4		42.6	
Progression Factor		1.00	1.00		1.00	
Incremental Delay, d2		25.9	1.1		1.5	
Delay (s)		34.6	19.5		44.0	
Level of Service		C	B		D	
Approach Delay (s)		34.6	19.5		44.0	
Approach LOS		C	B		D	

Intersection Summary			
HCM Average Control Delay		34.4	HCM Level of Service C
HCM Volume to Capacity ratio		0.91	
Actuated Cycle Length (s)		101.4	Sum of lost time (s) 8.0
Intersection Capacity Utilization		101.5%	ICU Level of Service G
Analysis Period (min)		15	

c Critical Lane Group



Lane Group	EBL	EBT	WBT	SBL
Lane Configurations		↕	↕	↕
Volume (vph)	513	337	326	12
Lane Group Flow (vph)	0	955	370	550
Turn Type	pm+pl			
Protected Phases	5	2	6	4
Permitted Phases	2			
Detector Phases	5	2 5	6	4
Minimum Initial (s)	4.0	10.0	10.0	7.0
Minimum Split (s)	10.0	16.0	16.0	13.0
Total Split (s)	36.0	87.0	51.0	16.0
Total Split (%)	35.0%	84.5%	49.5%	15.5%
Yellow Time (s)	4.5	4.5	4.5	4.5
All-Red Time (s)	1.5	1.5	1.5	1.5
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	Min	Min	None
v/c Ratio		0.99	0.45	0.84
Control Delay		38.2	20.7	15.6
Queue Delay		0.0	0.0	0.0
Total Delay		38.2	20.7	15.6
Queue Length 50th (ft)		287	154	9
Queue Length 95th (ft)		#531	241	54
Internal Link Dist (ft)		491	1298	1509
Turn Bay Length (ft)				
Base Capacity (vph)		963	822	674
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.99	0.45	0.82

Intersection Summary

Cycle Length: 103

Actuated Cycle Length: 101.4

Natural Cycle: 150

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 11: Route 940 & Route 390

ø2	ø4
67 s	16 s
ø5	ø6
36 s	51 s



Lane Group	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Lane Configurations	↕			↕			↕			↕		
Ideal Flow (vohpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	8	8	8	11	11	11
Grade (%)	-2%			2%			-3%			-1%		
Turning Speed (mph)	15	9		15	9		15	9		15	9	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frts	0.998			0.961			0.979			0.974		
Frts Protected	0.993			0.999			0.989			0.963		
Satd. Flow (prot)	0	1802	0	0	1711	0	0	1587	0	0	1697	0
Frts Permitted	0.993			0.999			0.989			0.963		
Satd. Flow (perm)	0	1802	0	0	1711	0	0	1587	0	0	1697	0
Headway Factor	1.03	1.03	1.03	1.06	1.06	1.06	1.18	1.18	1.18	1.04	1.04	1.04
Link Speed (mph)	45			45			35			45		
Link Distance (ft)	1662			865			282			1220		
Travel Time (s)	25.2			13.1			5.5			18.5		
Volume (vph)	46	298	5	6	313	132	5	13	3	103	5	26
Peak Hour Factor	0.90	0.90	0.90	0.84	0.84	0.84	0.80	0.80	0.80	0.87	0.87	0.87
Adj. Flow (vph)	51	331	6	7	373	157	6	16	4	118	6	30
Lane Group Flow (vph)	0	388	0	0	537	0	0	26	0	0	154	0
Sign Control	Free			Free			Stop			Stop		

Intersection Summary
 Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization 65.7% ICU Level of Service C
 Analysis Period (min) 15

	↖		→		↘		↙		←		↖		↘		↑		↖		↘		↓		↙									
Movement	EB1	EB2	EBR	WB1	WB2	WBR	NB1	NB2	NBR	SBL	SBR	SBL	SBR	SBL	SBR	SBL	SBR	SBL	SBR	SBL	SBR	SBL	SBR									
Lane Configurations	↕						↕				↕				↕				↕				↕									
Sign Control	Free						Free				Stop				Stop				Stop				Stop									
Grade	-2%						2%				-3%				-3%				-3%				-1%									
Volume (veh/h)	46	298	5	6	313	132	5	13	3	103	5	26	5	13	3	103	5	26	5	13	3	103	5	26								
Peak Hour Factor	0.90	0.90	0.90	0.84	0.84	0.84	0.80	0.80	0.80	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87								
Hourly flow rate (vph)	51	331	6	7	373	157	6	16	4	118	6	30	6	16	4	118	6	30	6	16	4	118	6	30								
Pedestrians																																
Lane Width (ft)																																
Walking Speed (ft/s)																																
Percent Blockage																																
Right turn flare (veh)																																
Median type													None						None													
Median storage (veh)																																
Upstream signal (ft)																																
pX, platoon unblocked																																
vC, conflicting volume	530				337				934		980		334		913		904		451													
vC1, stage 1 conf vol																																
vC2, stage 2 conf vol																																
vCu, unblocked vol	530				337				934		980		334		913		904		451													
tC, single (s)	4.1				4.1				7.1		6.5		6.2		7.1		6.5		6.2													
tC, 2 stage (s)																																
IF (s)	2.2				2.2				3.5		4.0		3.3		3.5		4.0		3.3													
p0 queue free %	95				99				97		93		99		48		98		95													
cM capacity (veh/h)	1037				1223				221		236		708		230		262		608													
Direction Lane #	EB 1		WB 1		NB 1		SB 1																									
Volume Total	388		537		26		154																									
Volume Left	51		7		6		118																									
Volume Right	6		157		4		30																									
cSH	1037		1223		256		263																									
Volume to Capacity	0.05		0.01		0.10		0.59																									
Queue Length 95th (ft)	4		0		8		85																									
Control Delay (s)	1.6		0.2		20.6		36.5																									
Lane LOS	A		A		C		E																									
Approach Delay (s)	1.6		0.2		20.6		36.5																									
Approach LOS					C		E																									
Intersection Summary																																
Average Delay	6.2																															
Intersection Capacity Utilization	65.7%																															
ICU Level of Service	C																															
Analysis Period (min)	15																															



Movement	WBL	WBR	NBT	NBR	SEB	SEB
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	13	12	11	11	11	12
Grade (%)	-6%		-2%			-5%
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	0.95		1.00	0.95
Frnt	1.00	0.85	0.98		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1883	1631	3393		1753	3628
Flt Permitted	0.95	1.00	1.00		0.09	1.00
Satd. Flow (perm)	1883	1631	3393		167	3628
Volume (vph)	122	75	1435	197	79	1201
Peak-hour factor, PHF	0.90	0.90	0.94	0.94	0.94	0.94
Adj. Flow (vph)	136	83	1527	210	84	1278
RTOR Reduction (vph)	0	69	0	0	0	0
Lane Group Flow (vph)	136	14	1737	0	84	1278
Turn Type		Perm			pm+pt	
Protected Phases	8		2		1	6
Permitted Phases		8			6	
Actuated Green, G (s)	9.5	9.5	36.1		47.9	47.9
Effective Green, g (s)	12.5	12.5	40.1		51.9	51.9
Actuated g/C Ratio	0.17	0.17	0.55		0.72	0.72
Clearance Time (s)	7.0	7.0	8.0		6.0	8.0
Vehicle Extension (s)	3.0	3.0	6.0		3.0	6.0
Lane Grp Cap (vph)	325	282	1879		291	2601
v/s Ratio Prot	c0.07		c0.51		0.03	c0.35
v/s Ratio Perm		0.01			0.18	
v/c Ratio	0.42	0.05	0.92		0.29	0.49
Uniform Delay, d1	26.7	25.0	14.8		11.8	4.5
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	0.9	0.1	8.9		0.6	0.4
Delay (s)	27.6	25.1	23.6		12.3	4.9
Level of Service	C	C	C		B	A
Approach Delay (s)	26.6		23.6			5.4
Approach LOS	C		C			A

Intersection Summary

HCM Average Control Delay	16.3	HCM Level of Service	B
HCM Volume to Capacity ratio	0.78		
Actuated Cycle Length (s)	72.4	Sum of lost time (s)	12.0
Intersection Capacity Utilization	67.1%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			



Lane Group	WBL	WBR	NBT	SBL	SBT
Lane Configurations	↵	↗	↕	↵	↕
Volume (vph)	122	75	1435	79	1201
Lane Group Flow (vph)	136	83	1737	84	1278
Turn Type	Perm		pm+pl		
Protected Phases	8		2	1	6
Permitted Phases		8		6	
Detector Phases	8	8	2	1	6
Minimum Initial (s)	1.0	1.0	15.0	1.0	15.0
Minimum Split (s)	8.0	8.0	23.0	7.0	23.0
Total Split (s)	33.0	33.0	34.0	13.0	47.0
Total Split (%)	41.3%	41.3%	42.5%	16.3%	58.8%
Yellow Time (s)	5.0	5.0	6.0	6.0	6.0
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0
Lead/Lag			Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	
Recall Mode	None	None	Min	None	Min
v/c Ratio	0.38	0.22	0.89	0.25	0.48
Control Delay	22.2	6.1	26.5	5.8	5.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	22.2	6.1	26.5	5.8	5.9
Queue Length 50th (ft)	45	0	~404	9	106
Queue Length 95th (ft)	89	30	#590	26	184
Internal Link Dist (ft)	1091		2024		1031
Turn Bay Length (ft)		72		175	
Base Capacity (vph)	632	602	1943	347	2652
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.22	0.14	0.89	0.24	0.48

Intersection Summary

Cycle Length: 80
 Actuated Cycle Length: 71.2
 Natural Cycle: 60
 Control Type: Semi-Act-Uncoord

~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Route 314 (Eastern Leg) & Route 611

↵ ø1	↕ ø2		
13 s	34 s		
↵ ø6		↵ ø8	
47 s		39 s	



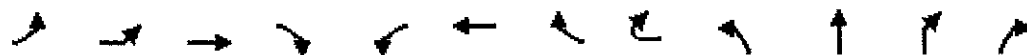
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↙	↗	↙	↑↑	↑↑	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	12	12	12	12
Grade (%)	4%			7%	-6%	
Storage Length (ft)	50	0	143			0
Storage Lanes	1	1	1			0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95
Fr _t		0.850			0.997	
Flt Protected	0.950		0.950			
Satd. Flow (prot)	1587	1420	1708	3415	3634	0
Flt Permitted	0.950		0.950			
Satd. Flow (perm)	1587	1420	1708	3415	3634	0
Headway Factor	1.12	1.12	1.05	1.05	0.96	0.96
Link Speed (mph)	40			45	45	
Link Distance (ft)	3960			1111	2283	
Travel Time (s)	67.5			16.8	34.6	
Volume (vph)	22	160	187	1323	1120	23
Peak Hour Factor	0.96	0.96	0.96	0.96	0.97	0.97
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Adj. Flow (vph)	23	167	195	1378	1155	24
Lane Group Flow (vph)	23	167	195	1378	1179	0
Sign Control	Stop			Free	Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.4%
	ICU Level of Service B
Analysis Period (min)	15



Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	↖	↗	↖	↑↑	↑↑		
Sign Control	Stop			Free	Free		
Grade	4%			7%	-6%		
Volume (veh/h)	22	160	187	1323	1120	23	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.97	0.97	
Hourly flow rate (vph)	23	167	195	1378	1155	24	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None						
Median storage (veh)							
Upstream signal (ft)	1111						
pX, platoon unblocked	0.68						
vC, conflicting volume	2245	589	1178				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	2360	589	1178				
tC, single (s)	6.9	7.0	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	0	63	67				
cM capacity (veh/h)	13	446	588				
Direction, Lane #							
	EB 1	EB 2	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	23	167	195	689	689	770	409
Volume Left	23	0	195	0	0	0	0
Volume Right	0	167	0	0	0	0	24
cSH	13	446	588	1700	1700	1700	1700
Volume to Capacity	1.76	0.37	0.33	0.41	0.41	0.45	0.24
Queue Length 95th (ft)	90	43	36	0	0	0	0
Control Delay (s)	949.2	17.8	14.1	0.0	0.0	0.0	0.0
Lane LOS	F	C	B				
Approach Delay (s)	130.4		1.7	0.0			
Approach LOS	F						
Intersection Summary							
Average Delay			9.3				
Intersection Capacity Utilization			55.4%	ICU Level of Service		B	
Analysis Period (min)							15

2017 Base Conditions 3: Woodland Road/Private Driveway/Stricklands Road & Route 611
 Saturday P.M. Peak Hour



Movement	EBL	EB	EBR	WBL	WB	WBR	NBL	NB	NBR			
Lane Configurations	↕			↕			↑		↑↑			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Lane Width	15	15	15	15	13	13	13	13	11	12	12	12
Grade (%)	6%			5%			3%					
Total Lost time (s)	4.0			4.0			4.0		4.0			
Lane Util. Factor	1.00			1.00			1.00		0.95			
Frt	0.91			0.98			1.00		0.99			
Flt Protected	0.99			0.96			0.95		1.00			
Satd. Flow (prot)	1786			1774			1685		3452			
Flt Permitted	0.90			0.74			0.25		1.00			
Satd. Flow (perm)	1637			1373			436		3452			
Volume (vph)	6	1	1	16	66	5	8	1	17	1242	3	83
Peak-hour factor, PHF	0.61	0.61	0.61	0.61	0.92	0.92	0.92	0.92	0.98	0.98	0.98	0.98
Adj. Flow (vph)	10	2	2	26	72	5	9	1	17	1267	3	85
RTOR Reduction (vph)	0	0	22	0	0	1	0	0	0	3	0	0
Lane Group Flow (vph)	0	0	18	0	0	86	0	0	17	1352	0	0
Turn Type	Perm	Perm		Perm				Perm				
Protected Phases			4			8				2		
Permitted Phases	4	4			8			2				
Actuated Green, G (s)			10.7			10.7			47.7	47.7		
Effective Green, g (s)			12.7			12.7			51.2	51.2		
Actuated g/C Ratio			0.15			0.15			0.60	0.60		
Clearance Time (s)			6.0			6.0			7.5	7.5		
Vehicle Extension (s)			3.0			3.0			5.0	5.0		
Lane Grp Cap (vph)			242			203			260	2060		
v/s Ratio Prot										c0.39		
v/s Ratio Perm			0.01			c0.06			0.04			
v/c Ratio			0.07			0.42			0.07	0.66		
Uniform Delay, d1			31.5			33.2			7.3	11.5		
Progression Factor			1.00			1.00			1.00	1.00		
Incremental Delay, d2			0.1			1.4			0.2	1.0		
Delay (s)			31.6			34.7			7.5	12.5		
Level of Service			C			C			A	B		
Approach Delay (s)			31.6			34.7			12.4			
Approach LOS			C			C			B			

Intersection Summary

HCM Average Control Delay	11.1	HCM Level of Service	B
HCM Volume to Capacity ratio	0.59		
Actuated Cycle Length (s)	85.8	Sum of lost time (s)	16.0
Intersection Capacity Utilization	63.7%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

2017 Base Conditions 3: Woodland Road/Private Driveway/Stricklands Road & Route 611
 Saturday P.M. Peak Hour



Move	SB1	SB1	SB1	SB1	SW1	SW1	SW1	SW1
Lane Configurations		↕	↕			↕	↕	↕
Ideal Flow (vohpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	12	12	11	11	11	11
Grade (%)			-7%			-2%		
Total Lost time (s)		4.0	4.0			4.0		
Lane Util. Factor		1.00	0.95			1.00		
Frt		1.00	1.00			0.93		
Flt Protected		0.95	1.00			0.98		
Satd. Flow (prot)		1770	3655			1655		
Flt Permitted		0.12	1.00			0.98		
Satd. Flow (perm)		220	3655			1655		
Volume (vph)	1	5	1061	17	1	1	1	1
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	1.00	1.00	1.00	1.00
Adj. Flow (vph)	1	5	1083	17	1	1	1	1
RTOR Reduction (vph)	0	0	1	0	0	1	0	0
Lane Group Flow (vph)	0	6	1099	0	0	3	0	0
Turn Type	pm+pt	pm+pt			Perm			
Protected Phases	1	1	6			9		
Permitted Phases	6	6			9			
Actuated Green, G (s)		54.3	54.3			1.3		
Effective Green, g (s)		57.8	57.8			3.3		
Actuated g/C Ratio		0.67	0.67			0.04		
Clearance Time (s)		5.5	7.5			6.0		
Vehicle Extension (s)		3.0	5.0			3.0		
Lane Grp Cap (vph)		195	2462			64		
v/s Ratio Prot		0.00	0.30					
v/s Ratio Perm		0.02				0.00		
v/c Ratio		0.03	0.45			0.05		
Uniform Delay, d1		7.6	6.5			39.7		
Progression Factor		1.00	1.00			1.00		
Incremental Delay, d2		0.1	0.3			0.3		
Delay (s)		7.7	6.8			40.0		
Level of Service		A	A			D		
Approach Delay (s)			6.8			40.0		
Approach LOS			A			D		

Intersection Summary

2017 Base Conditions 3: Woodland Road/Private Driveway/Stricklands Road & Route 611
 Saturday P.M. Peak Hour



Lane Group	NB1/2	EB1	EB3	WB1	WB3	NB1	NB3	SBL/2	SBL	SBL	SW
Lane Configurations			↔		↔	↖	↕		↖	↕	↗
Volume (vph)	6	1	1	66	5	17	1242	1	5	1061	1
Lane Group Flow (vph)	0	0	40	0	87	17	1355	0	6	1100	4
Turn Type	Perm	Perm		Perm		Perm		pm+pt	pm+pt		
Protected Phases			4		8		2	1	1	6	9
Permitted Phases	4	4		8		2		6	6		
Detector Phases	4	4	4	8	8	2	2	1	1	6	9
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	10.0	10.0	4.0	4.0	10.0	7.0
Minimum Split (s)	13.0	13.0	13.0	13.0	13.0	17.5	17.5	9.5	9.5	17.5	13.0
Total Split (s)	30.0	30.0	30.0	30.0	30.0	39.0	39.0	12.0	12.0	51.0	19.0
Total Split (%)	30.0%	30.0%	30.0%	30.0%	30.0%	39.0%	39.0%	12.0%	12.0%	51.0%	19.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	5.5	5.5	5.5	5.5	5.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	0.0	2.0	2.0
Lead/Lag						Lag	Lag	Lead	Lead		
Lead-Lag Optimize?						Yes	Yes	Yes	Yes		
Recall Mode	None	None	None	None	None	Min	Min	None	None	Min	None
v/c Ratio			0.13		0.37	0.08	0.59		0.02	0.44	0.02
Control Delay			13.3		23.6	11.0	11.6		8.2	6.8	28.2
Queue Delay			0.0		0.0	0.0	0.0		0.0	0.0	0.0
Total Delay			13.3		23.6	11.0	11.6		8.2	6.8	28.2
Queue Length 50th (ft)			4		25	2	103		1	72	1
Queue Length 95th (ft)			18		81	20	506		7	238	11
Internal Link Dist (ft)			105		2012		2203			2327	625
Turn Bay Length (ft)						73			183		
Base Capacity (vph)			524		423	226	2294		271	2617	279
Starvation Cap Reductn			0		0	0	0		0	0	0
Spillback Cap Reductn			0		0	0	0		0	0	0
Storage Cap Reductn			0		0	0	0		0	0	0
Reduced v/c Ratio			0.08		0.21	0.08	0.59		0.02	0.42	0.01

Intersection Summary

Cycle Length: 100

Actuated Cycle Length: 77.5

Natural Cycle: 70

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Woodland Road/Private Driveway/Stricklands Road & Route 611

↖ 01 12 s	↕ 02 33 s	↖ 04 30 s	↖ 03 19 s
↖ 06 51 s		↖ 08 30 s	



Lane Group	EB	EB	EBR	WB	WB	WBR	NB	NB	NBR	SB	SB	SB
Lane Configurations	↕			↕			↙	↕		↙	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	13	13	12	12	12	11	12	12	11	12	12
Grade (%)	2%			8%			1%		-1%			
Storage Length (ft)	0	0		0	0		100	0		100	0	
Storage Lanes	0	0		0	0		1	0		1	0	
Turning Speed (mph)	15	9		15	9		15	9		15	9	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	0.892			0.938								
Flt Protected	0.990			0.991			0.950		0.950			
Satd. Flow (prot)	0	1683	0	0	1662	0	1702	3522	0	1719	3557	0
Flt Permitted	0.990			0.991			0.950		0.950			
Satd. Flow (perm)	0	1683	0	0	1662	0	1702	3522	0	1719	3557	0
Headway Factor	0.97	0.97	0.97	1.05	1.05	1.05	1.05	1.01	1.01	1.04	0.99	0.99
Link Speed (mph)	35			35			45		45			
Link Distance (ft)	158			1027			2407		3261			
Travel Time (s)	3.1			20.0			36.5		49.4			
Volume (vph)	1	0	4	4	8	10	8	1247	1	1	1075	1
Peak Hour Factor	0.50	0.50	0.50	0.39	0.39	0.39	0.92	0.92	0.92	0.89	0.89	0.89
Adj. Flow (vph)	2	0	8	10	21	26	9	1355	1	1	1208	1
Lane Group Flow (vph)	0	10	0	0	57	0	9	1356	0	1	1209	0
Sign Control	Stop			Stop			Free		Free			

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 44.5% ICU Level of Service A

Analysis Period (min) 15

2017 Base Conditions
Saturday P.M. Peak Hour

4: Meadowside Road/Trinity Hill Road & Route 611

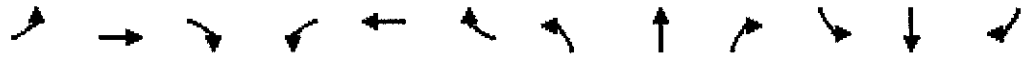


Movement	FB1	FB2	FB3	WBL	WBL	WBL	NBL	NBL	NBL	SB1	SB1	SB1
Lane Configurations	↕			↕			↗	↕		↗	↕	
Sign Control	Stop			Stop			Free			Free		
Grade	2%			8%			1%			-1%		
Volume (veh/h)	1	0	4	4	8	10	8	1247	1	1	1075	1
Peak Hour Factor	0.50	0.50	0.50	0.39	0.39	0.39	0.92	0.92	0.92	0.89	0.89	0.89
Hourly flow rate (vph)	2	0	8	10	21	26	9	1355	1	1	1208	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1942	2585	604	1988	2585	678	1209				1357	
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1942	2585	604	1988	2585	678	1209				1357	
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1				4.1	
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	81	100	98	70	16	93	98				100	
cM, capacity (veh/h)	10	25	441	35	24	394	573				503	
Direction Lane #	FB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	10	56	9	904	453	1	805	404				
Volume Left	2	10	9	0	0	1	0	0				
Volume Right	8	26	0	0	1	0	0	1				
cSH	48	47	573	1700	1700	503	1700	1700				
Volume to Capacity	0.21	1.21	0.02	0.53	0.27	0.00	0.47	0.24				
Queue Length 95th (ft)	17	131	1	0	0	0	0	0				
Control Delay (s)	99.6	337.5	11.4	0.0	0.0	12.2	0.0	0.0				
Lane LOS	F	F	B				B					
Approach Delay (s)	99.6	337.5	0.1				0.0					
Approach LOS	F	F										
Intersection Summary												
Average Delay			7.6									
Intersection Capacity Utilization			44.5%	ICU Level of Service				A				
Analysis Period (min)			15									



Lane Group	EB	EBT	EBR	WB	WBT	WBR	NB	NBT	NBR	SB	SBT	SBR
Lane Configurations	↕			↕			↗	↕		↗	↕	↗
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	10	10	10	11	12	12	11	12	12
Grade (%)	0%			9%			2%			-5%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	0.940			0.958			0.996			0.996		
Flt Protected	0.978			0.971			0.950			0.950		
Satd. Flow (prot)	0	1712	0	0	1544	0	1694	3490	0	1753	3613	0
Flt Permitted	0.978			0.971			0.950			0.950		
Satd. Flow (perm)	0	1712	0	0	1544	0	1694	3490	0	1753	3613	0
Headway Factor	1.00	1.00	1.00	1.16	1.16	1.16	1.06	1.01	1.01	1.01	0.97	0.97
Link Speed (mph)	30			30			45			45		
Link Distance (ft)	485			1492			3261			2754		
Travel Time (s)	11.0			33.9			49.4			41.7		
Volume (vph)	28	6	27	34	6	17	25	1197	36	16	1016	26
Peak Hour Factor	0.90	0.90	0.90	0.72	0.90	0.72	0.90	0.94	0.94	0.95	0.95	0.90
Adj Flow (vph)	31	7	30	47	7	24	28	1273	38	17	1069	29
Lane Group Flow (vph)	0	68	0	0	78	0	28	1311	0	17	1098	0
Sign Control	Stop			Stop			Free			Free		

Intersection Summary	
Area Type	Other
Control Type	Unsignalized
Intersection Capacity Utilization	45.5%
ICU Level of Service	A
Analysis Period (min)	15



Movement	EB	EB1	EBR	WB	WB1	WBR	NB	NB1	NBR	SB	SB1	SB2
Lane Configurations		↕			↕		↗	↕		↗	↕	↕
Sign Control		Stop			Stop			Free				Free
Grade		0%			9%			2%				-5%
Volume (veh/h)	28	6	27	34	6	17	25	1197	36	16	1016	26
Peak Hour Factor	0.90	0.90	0.90	0.72	0.90	0.72	0.90	0.94	0.94	0.95	0.95	0.90
Hourly flow rate (vph)	31	7	30	47	7	24	28	1273	38	17	1069	29
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1837	2485	549	1950	2480	656	1098			1312		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1837	2485	549	1950	2480	656	1098			1312		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	8	75	94	0	75	94	96			97		
cM capacity (veh/h)	34	27	479	27	27	407	631			523		

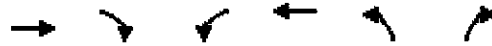
Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	68	78	28	849	463	17	713	385
Volume Left	31	47	28	0	0	17	0	0
Volume Right	30	24	0	0	38	0	0	29
cSH	55	38	631	1700	1700	523	1700	1700
Volume to Capacity	1.23	2.03	0.04	0.50	0.27	0.03	0.42	0.23
Queue Length 95th (ft)	147	210	3	0	0	2	0	0
Control Delay (s)	319.8	706.6	11.0	0.0	0.0	12.1	0.0	0.0
Lane LOS	F	F	B			B		
Approach Delay (s)	319.8	706.6	0.2			0.2		
Approach LOS	F	F						

Intersection Summary			
Average Delay		29.6	
Intersection Capacity Utilization	45.5%	ICU Level of Service	A
Analysis Period (min)	15		



Lane Group	EBL	EBR	WBL	WBF	NBL	NBR
Lane Configurations	↕		↕		↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	10	10	11	11
Grade (%)	1%		-3%		0%	
Turning Speed (mph)	9		15		15	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Fr _t	0.987					
Fl _t Protected			0.998		0.950	
Satd. Flow (prot)	1707	0	0	1761	1711	0
Fl _t Permitted			0.998		0.950	
Satd. Flow (perm)	1707	0	0	1761	1711	0
Headway Factor	1.10	1.10	1.07	1.07	1.04	1.04
Link Speed (mph)	40		40		25	
Link Distance (ft)	2092		650		499	
Travel Time (s)	35.7		11.1		13.6	
Volume (vph)	80	8	3	73	6	0
Peak Hour Factor	0.92	0.92	0.90	0.90	0.63	0.63
Adj. Flow (vph)	87	9	3	81	10	0
Lane Group Flow (vph)	96	0	0	84	10	0
Sign Control	Free		Free		Stop	

Intersection Summary	
Area Type	Other
Control Type	Unsignalized
Intersection Capacity Utilization	16.3%
ICU Level of Service	A
Analysis Period (min)	15



Movement	EBT	EBL	WB	WB	NB	NBL
Lane Configurations	↔			↕	↕	
Sign Control	Free			Free	Stop	
Grade	1%			-3%	0%	
Volume (veh/h)	80	8	3	73	6	0
Peak Hour Factor	0.92	0.92	0.90	0.90	0.63	0.63
Hourly flow rate (vph)	87	9	3	81	10	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			96		179	91
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			96		179	91
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	100
cM capacity (veh/h)			1498		809	966

Direction Lane #	EB 1	WB 1	NB 1
Volume Total	96	84	10
Volume Left	0	3	10
Volume Right	9	0	0
cSH	1700	1498	809
Volume to Capacity	0.06	0.00	0.01
Queue Length 95th (ft)	0	0	1
Control Delay (s)	0.0	0.3	9.5
Lane LOS		A	A
Approach Delay (s)	0.0	0.3	9.5
Approach LOS		A	

Intersection Summary			
Average Delay	0.6		
Intersection Capacity Utilization	16.3%	ICU Level of Service	A
Analysis Period (min)	15		



Lane Group	EBL	EBR	WBL	WBR	NBL	NBR
Lane Configurations	↖		↗		↘	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	10	10
Grade (%)	1%			1%	-1%	
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.995				0.910	
Flt Protected				0.997	0.984	
Satd. Flow (prot)	1783	0	0	1786	1565	0
Flt Permitted				0.997	0.984	
Satd. Flow (perm)	1783	0	0	1786	1565	0
Headway Factor	1.05	1.05	1.05	1.05	1.09	1.09
Link Speed (mph)	40		40		35	
Link Distance (ft)	650		936		704	
Travel Time (s)	11.1		16.0		13.7	
Volume (vph)	77	3	5	73	3	6
Peak Hour Factor	0.81	0.81	0.85	0.85	0.50	0.50
Adj. Flow (vph)	95	4	6	86	6	12
Lane Group Flow (vph)	99	0	0	92	18	0
Sign Control	Free		Free		Stop	

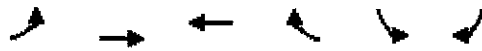
Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	17.9%
	ICU Level of Service A
Analysis Period (min)	15



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Sign Control	Free			Free	Stop	
Grade	1%			1%	-1%	
Volume (veh/h)	77	3	5	73	3	6
Peak Hour Factor	0.81	0.81	0.85	0.85	0.50	0.50
Hourly flow rate (vph)	95	4	6	86	6	12
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			99		195	97
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			99		195	97
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	99
cM capacity (veh/h)			1494		791	959

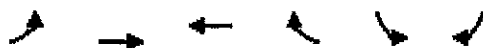
Direction Lane #	EBT	WBT	NBT
Volume Total	99	92	18
Volume Left	0	6	6
Volume Right	4	0	12
cSH	1700	1494	896
Volume to Capacity	0.06	0.00	0.02
Queue Length 95th (ft)	0	0	2
Control Delay (s)	0.0	0.5	9.1
Lane LOS		A	A
Approach Delay (s)	0.0	0.5	9.1
Approach LOS			A

Intersection Summary			
Average Delay		1.0	
Intersection Capacity Utilization		17.9%	ICU Level of Service A
Analysis Period (min)		15	



Lane Group	EB	EBL	WB	WBL	SB	SBR
Lane Configurations		↕	↕		↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	10	10
Grade (%)		-4%	2%		-6%	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.984		0.939	
Frt Protected		0.998			0.973	
Satd. Flow (prot)	0	1833	1754	0	1636	0
Frt Permitted		0.998			0.973	
Satd. Flow (perm)	0	1833	1754	0	1636	0
Headway Factor	1.02	1.02	1.06	1.06	1.05	1.05
Link Speed (mph)		40	40		35	
Link Distance (ft)		936	819		1342	
Travel Time (s)		16.0	14.0		26.1	
Volume (vph)	3	80	75	10	4	3
Peak Hour Factor	0.95	0.95	0.93	0.93	0.63	0.63
Adj. Flow (vph)	3	84	81	11	6	5
Lane Group Flow (vph)	0	87	92	0	11	0
Sign Control		Free	Free		Stop	

Intersection Summary
 Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization: 16.6% ICU Level of Service: A
 Analysis Period (min): 15



Control	EB	WB	SB	SB	SB	
Lane Configurations	↕	↕	↕	↕	↕	
Sign Control	Free	Free	Stop	Stop	Stop	
Grade	-4%	2%	-6%	-6%	-6%	
Volume (veh/h)	3	80	75	10	4	3
Peak Hour Factor	0.95	0.95	0.93	0.93	0.63	0.63
Hourly flow rate (vph)	3	84	81	11	6	5
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	91		177		86	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	91		177		86	
tC, single (s)	4.1		6.4		6.2	
tC, 2 stage (s)						
tF (s)	2.2		3.5		3.3	
p0 queue free %	100		99		100	
cM capacity (veh/h)	1503		812		973	
Direction Lane #	EB	WB	SB	SB	SB	
Volume Total	87	91	11			
Volume Left	3	0	6			
Volume Right	0	11	5			
cSH	1503	1700	874			
Volume to Capacity	0.00	0.05	0.01			
Queue Length 95th (ft)	0	0	1			
Control Delay (s)	0.3	0.0	9.2			
Lane LOS	A		A			
Approach Delay (s)	0.3	0.0	9.2			
Approach LOS			A			
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization		16.6%		ICU Level of Service	A	
Analysis Period (min)			15			



Lane Group	EBL	EBR	NBL	NBT	SBL	SBR
Lane Configurations	↔		↕		↔	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	-3%		-5%		4%	
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.999			0.948		
Flt Protected	0.953		0.999			
Satd. Flow (prot)	1680	0	0	1844	1673	0
Flt Permitted	0.953		0.999			
Satd. Flow (perm)	1680	0	0	1844	1673	0
Headway Factor	1.07	1.07	1.01	1.01	1.07	1.07
Link Speed (mph)	40		45		45	
Link Distance (ft)	1794		1439		1446	
Travel Time (s)	30.6		21.8		21.9	
Volume (vph)	83	1	4	136	128	81
Peak Hour Factor	0.82	0.82	0.75	0.75	0.86	0.86
Adj. Flow (vph)	101	1	5	181	149	94
Lane Group Flow (vph)	102	0	0	186	243	0
Sign Control	Stop		Free		Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	23.0%
ICU Level of Service	A
Analysis Period (min)	15



Movement	EB	EB	NB	NB	SB	SB
Lane Configurations	T		↑		↑	
Sign Control	Stop		Free		Free	
Grade	-3%		-5%		4%	
Volume (veh/h)	83	1	4	136	128	81
Peak Hour Factor	0.82	0.82	0.75	0.75	0.86	0.86
Hourly flow rate (vph)	101	1	5	181	149	94
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	388	196	243			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	388	196	243			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	84	100	100			
cM capacity (veh/h)	613	845	1323			
Direction, Lane #	EB, 1	NB, 1	SB, 1			
Volume Total	102	187	243			
Volume Left	101	5	0			
Volume Right	1	0	94			
cSH	615	1323	1700			
Volume to Capacity	0.17	0.00	0.14			
Queue Length 95th (ft)	15	0	0			
Control Delay (s)	12.0	0.3	0.0			
Lane LOS	B	A				
Approach Delay (s)	12.0	0.3	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			2.4			
Intersection Capacity Utilization			23.0%	ICU Level of Service	A	
Analysis Period (min)			15			



Movement	EB	SB	WB	NB	SB	SE
Lane Configurations	↕		↕		↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	11	11	11	11	11	8
Grade (%)		-4%		4%		1%
Total Lost time (s)	4.0		4.0		4.0	
Lane Util. Factor	1.00		1.00		1.00	
Frt	0.99		1.00		0.88	
Flt Protected	1.00		0.99		1.00	
Satd. Flow (prot)	1825		1740		1566	
Flt Permitted	0.99		0.60		0.97	
Satd. Flow (perm)	1815		1053		1522	
Volume (vph)	4	459	21	188	469	0
Peak-hour factor, PHF	0.84	0.84	0.84	0.93	0.93	0.71
Adj. Flow (vph)	5	546	25	202	504	0
RTOR Reduction (vph)	0	2	0	0	0	0
Lane Group Flow (vph)	0	574	0	0	706	0
Turn Type	Perm		pm+pt		Perm	
Protected Phases	2		1		6	
Permitted Phases	2		6		8	
Actuated Green, G (s)	26.8		45.0		9.0	
Effective Green, g (s)	28.8		47.0		10.0	
Actuated g/C Ratio	0.44		0.72		0.15	
Clearance Time (s)	6.0		6.0		5.0	
Vehicle Extension (s)	6.0		6.0		3.0	
Lane Grp Cap (vph)	804		911		234	
v/s Ratio Prot			c0.17			
v/s Ratio Perm	0.32		c0.39		c0.05	
v/c Ratio	0.71		0.77		0.31	
Uniform Delay, d1	14.7		5.7		24.5	
Progression Factor	1.00		1.00		1.00	
Incremental Delay, d2	4.3		5.3		0.8	
Delay (s)	19.1		10.9		25.2	
Level of Service	B		B		C	
Approach Delay (s)	19.1		10.9		25.2	
Approach LOS	B		B		C	

Intersection Summary			
HCM Average Control Delay	16.7	HCM Level of Service	B
HCM Volume to Capacity ratio	0.69		
Actuated Cycle Length (s)	65.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	87.8%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

2017 Base Conditions
Saturday P.M. Peak Hour

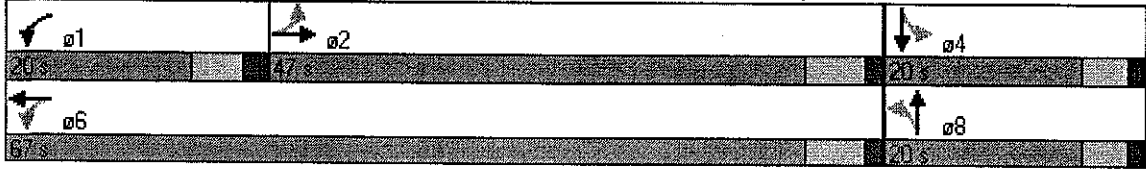
10: Route 940 & Carlton Road/Private Driveway



Lane Group	WB	EB	WBL	WBT	NB	NBT	SB	SBL	SBT
Lane Configurations	↕		↕		↕		↕		
Volume (vph)	4	459	188	469	21	1	1	0	
Lane Group Flow (vph)	0	576	0	706	0	308	0	10	
Turn Type	Perm	pm+pt		Perm	Perm		Perm		
Protected Phases		2	1	6		8		4	
Permitted Phases	2		6		8		4		
Detector Phases	2	2	1	6	1	8	4	4	
Minimum Initial (s)	10.0	10.0	4.0	10.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	16.0	16.0	10.0	16.0	11.0	11.0	11.0	11.0	
Total Split (s)	47.0	47.0	20.0	67.0	20.0	20.0	20.0	20.0	
Total Split (%)	54.0%	54.0%	23.0%	77.0%	23.0%	23.0%	23.0%	23.0%	
Yellow Time (s)	4.5	4.5	4.0	4.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	1.5	1.5	2.0	1.5	1.5	1.5	1.5	1.5	
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	Min	Min	None	Min	None	None	None	None	
v/c Ratio		0.73		0.78		0.66		0.05	
Control Delay		17.8		9.1		10.4		17.9	
Queue Delay		0.0		0.0		0.0		0.0	
Total Delay		17.8		9.1		10.4		17.9	
Queue Length 50th (ft)		175		84		12		0	
Queue Length 95th (ft)		297		#230		35		13	
Internal Link Dist (ft)		1322		1070		1366		73	
Turn Bay Length (ft)									
Base Capacity (vph)		994		1055		570		327	
Starvation Cap Reductn		0		0		0		0	
Spillback Cap Reductn		0		0		0		0	
Storage Cap Reductn		0		0		0		0	
Reduced v/c Ratio		0.58		0.67		0.54		0.03	

Intersection Summary
 Cycle Length: 87
 Actuated Cycle Length: 66
 Natural Cycle: 60
 Control Type: Semi Act-Uncoord
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 940 & Carlton Road/Private Driveway





Movement	EBL	EBR	WBL	WBR	SEB	SEB
Lane Configurations		↕	↕		↕	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	11	11	11	11	14	14
Grade (%)		-2%	2%		-4%	
Total Lost time (s)		4.0	4.0		4.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	0.99		0.88	
Flt Protected		0.98	1.00		0.99	
Satd. Flow (prot)		1756	1768		1774	
Flt Permitted		0.46	1.00		0.99	
Satd. Flow (perm)		828	1768		1774	
Volume (vph)	336	322	305	19	45	352
Peak-hour factor, PHF	0.79	0.79	0.84	0.84	0.81	0.81
Adj. Flow (vph)	424	408	363	23	56	435
RTOR Reduction (vph)	0	0	2	0	280	0
Lane Group Flow (vph)	0	832	384	0	211	0
Heavy Vehicles (%)	3%	3%	2%	2%	2%	2%
Turn Type	pm+pt					
Protected Phases	5	2	6		4	
Permitted Phases	2					
Actuated Green, G (s)		74.1	44.1		13.9	
Effective Green, g (s)		76.1	46.1		15.9	
Actuated g/C Ratio		0.76	0.46		0.16	
Clearance Time (s)		6.0	6.0		6.0	
Vehicle Extension (s)		6.0	6.0		3.0	
Lane Grp Cap (vph)		871	815		282	
v/s Ratio Prot		c0.25	0.22		c0.12	
v/s Ratio Perm		c0.48				
v/c Ratio		0.96	0.47		0.75	
Uniform Delay, d1		10.5	18.6		40.1	
Progression Factor		1.00	1.00		1.00	
Incremental Delay, d2		21.0	1.2		10.3	
Delay (s)		31.4	19.8		50.5	
Level of Service		C	B		D	
Approach Delay (s)		31.4	19.8		50.5	
Approach LOS		C	B		D	

Intersection Summary			
HCM Average Control Delay	34.3	HCM Level of Service	C
HCM Volume to Capacity ratio	0.91		
Actuated Cycle Length (s)	100.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	86.9%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

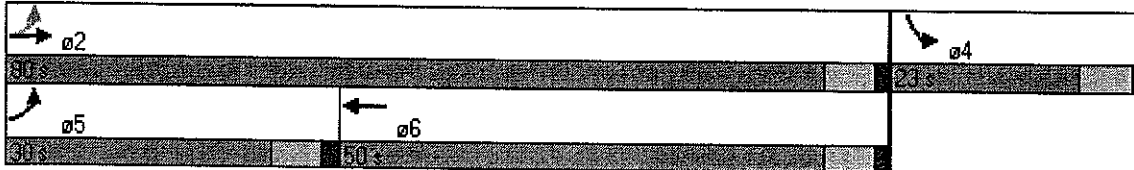


Lane Group	EB	WB	WB	SB
Lane Configurations		↕	↕	↕
Volume (vph)	335	322	305	45
Lane Group Flow (vph)	0	832	386	491
Turn Type	pm+pl			
Protected Phases	5	2	6	4
Permitted Phases	2			
Detector Phases	5	2 5	6	4
Minimum Initial (s)	4.0	10.0	10.0	7.0
Minimum Split (s)	10.0	16.0	16.0	13.0
Total Split (s)	30.0	80.0	50.0	23.0
Total Split (%)	29.1%	77.7%	48.5%	22.3%
Yellow Time (s)	4.5	4.5	4.5	4.5
All-Red Time (s)	1.5	1.5	1.5	1.5
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	Min	Min	None
v/c Ratio		0.97	0.47	0.87
Control Delay		33.9	21.5	25.6
Queue Delay		0.0	0.0	0.0
Total Delay		33.9	21.5	25.6
Queue Length 50th (ft)		248	173	99
Queue Length 95th (ft)		#307	234	171
Internal Link Dist (ft)		491	1298	1730
Turn Bay Length (ft)				
Base Capacity (vph)		861	816	599
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.97	0.47	0.82

Phase Sequence Summary

Cycle Length: 103
 Actuated Cycle Length: 100.1
 Natural Cycle: 80
 Control Type: Semi Act-Uncoord
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

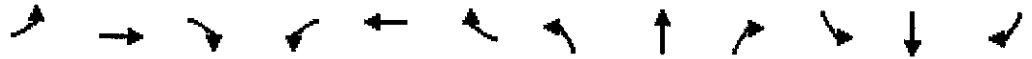
Splits and Phases: 11: Route 940 & Route 390





Lane Group	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Lane Configurations	↕			↕			↕			↕		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	8	8	8	11	11	11
Grade (%)	-2%			2%			-3%			-1%		
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.998			0.964			0.965			0.969		
Flt Protected	0.994			0.999			0.984			0.967		
Satd. Flow (prot)	0	1787	0	0	1717	0	0	1556	0	0	1696	0
Flt Permitted	0.994			0.999			0.984			0.967		
Satd. Flow (perm)	0	1787	0	0	1717	0	0	1556	0	0	1696	0
Headway Factor	1.03	1.03	1.03	1.06	1.06	1.06	1.18	1.18	1.18	1.04	1.04	1.04
Link Speed (mph)	45			45			35			45		
Link Distance (ft)	1662			865			282			1220		
Travel Time (s)	25.2			13.1			5.5			18.5		
Volume (vph)	45	317	5	5	285	106	4	5	3	103	14	35
Peak Hour Factor	0.89	0.89	0.89	0.81	0.81	0.81	0.45	0.45	0.45	0.88	0.88	0.88
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	51	356	6	6	352	131	9	11	7	117	16	40
Lane Group Flow (vph)	0	413	0	0	489	0	0	27	0	0	173	0
Sign Control	Free			Free			Stop			Stop		

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	66.4%
	ICU Level of Service C
Analysis Period (min)	15



Movement	EB1	EB2	EB3	WB1	WB2	WB3	NB1	NB2	NB3	SB1	SB2	SB3
Lane Configurations	↔			↔			↔			↔		
Sign Control	Free			Free			Stop			Stop		
Grade	-2%			2%			-3%			-1%		
Volume (veh/h)	45	317	5	5	285	106	4	5	3	103	14	35
Peak Hour Factor	0.89	0.89	0.89	0.81	0.81	0.81	0.45	0.45	0.45	0.88	0.88	0.88
Hourly flow rate (vph)	51	356	6	6	352	131	9	11	7	117	16	40
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type							None			None		
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	483			362			937	955	359	902	893	417
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	483			362			937	955	359	902	893	417
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	95			99			96	95	99	51	94	94
cM capacity (veh/h)	1075			1197			211	245	686	238	267	636
Direction Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	412	489	27	173								
Volume Left	51	6	9	117								
Volume Right	6	131	7	40								
cSH	1075	1197	274	281								
Volume to Capacity	0.05	0.01	0.10	0.61								
Queue Length 95th (ft)	4	0	8	94								
Control Delay (s)	1.5	0.2	19.5	36.3								
Lane LOS	A	A	C	E								
Approach Delay (s)	1.5	0.2	19.5	36.3								
Approach LOS			C	E								
Intersection Summary												
Average Delay			6.8									
Intersection Capacity Utilization			66.4%	ICU Level of Service	C							
Analysis Period (min)	15											

2017 PROJECTED CONDITIONS

2017 Projected Conditions - With Site-Related Recommendations
 Friday P.M. Peak Hour

1: Route 314 (Eastern Leg) & Route 611



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗	↕		↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	13	12	11	11	11	12
Grade (%)	-6%		-2%			-5%
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	0.95		1.00	0.95
Fr _t	1.00	0.85	0.97		1.00	1.00
Fl _t Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1883	1631	3332		1753	3628
Fl _t Permitted	0.95	1.00	1.00		0.08	1.00
Satd. Flow (perm)	1883	1631	3332		142	3628
Volume (vph)	408	169	1905	403	142	1404
Peak-hour factor, PHF	0.83	0.83	0.98	0.98	0.83	0.83
Adj. Flow (vph)	492	204	1944	411	171	1692
RTOR Reduction (vph)	0	59	0	0	0	0
Lane Group Flow (vph)	492	145	2355	0	171	1692
Heavy Vehicles (%)	2%	2%	3%	3%	2%	2%
Turn Type		Perm			pm+pt	
Protected Phases	8		2		1	6
Permitted Phases		8			6	
Actuated Green, G (s)	26.1	26.1	44.1		57.9	57.9
Effective Green, g (s)	29.1	29.1	48.1		61.9	61.9
Actuated g/C Ratio	0.29	0.29	0.49		0.63	0.63
Clearance Time (s)	7.0	7.0	8.0		6.0	8.0
Vehicle Extension (s)	3.0	3.0	6.0		3.0	6.0
Lane Grp Cap. (vph)	553	479	1619		248	2268
v/s Ratio Prot	c0.26		c0.71		0.07	c0.47
v/s Ratio Perm		0.09			0.36	
v/c Ratio	0.89	0.30	1.45		0.69	0.75
Uniform Delay, d ₁	33.4	27.1	25.4		25.2	13.0
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d ₂	16.0	0.4	208.1		7.8	1.9
Delay (s)	49.4	27.5	233.5		32.9	14.9
Level of Service	D	C	F		C	B
Approach Delay (s)	43.0		233.5			16.5
Approach LOS	D		F			B

Intersection Summary:			
HCM Average Control Delay	124.3	HCM Level of Service	F
HCM Volume to Capacity ratio	1.20		
Actuated Cycle Length (s)	99.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	106.0%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

2017 Projected Conditions - With Site-Related Recommendations
 Friday P.M. Peak Hour

1: Route 314 (Eastern Leg) & Route 611



Lane Group	WBL	WBR	NBT	SBL	SBT
Lane Configurations	↙	↗	↕	↘	↕
Volume (vph)	408	169	1905	142	1404
Lane Group Flow (vph)	492	204	2355	171	1692
Turn Type	Perm		pm+pt		
Protected Phases	8		2	1	6
Permitted Phases		8		6	
Detector Phases	8	8	2	1	6
Minimum Initial (s)	1.0	1.0	15.0	1.0	15.0
Minimum Split (s)	8.0	8.0	23.0	7.0	23.0
Total Split (s)	34.0	34.0	52.0	14.0	66.0
Total Split (%)	34.0%	34.0%	52.0%	14.0%	66.0%
Yellow Time (s)	5.0	5.0	6.0	6.0	6.0
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0
Lead/Lag			Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	
Recall Mode	None	None	Min	None	Min
v/c Ratio	0.89	0.38	1.46	0.69	0.75
Control Delay	51.0	18.2	233.5	32.3	15.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	51.0	18.2	233.5	32.3	15.8
Queue Length 50th (ft)	296	58	~1092	54	371
Queue Length 95th (ft)	#410	105	#1229	109	391
Internal Link Dist (ft)	1091		2024		1031
Turn Bay Length (ft)		72		175	
Base Capacity (vph)	566	548	1618	251	2270
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.87	0.37	1.46	0.68	0.75

Intersection Summary

Cycle Length: 100

Actuated Cycle Length: 99

Natural Cycle: 110

Control Type: Semi Act-Uncoord

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Route 314 (Eastern Leg) & Route 611

↙ a1 14 s	↕ a2 52 s		
↘ a6 66 s		↘ a8 34 s	

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	12	12	12	12
Grade (%)	4%			7%	6%	
Storage Length (ft)	50	0	143			0
Storage Lanes	1	1	1			0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95
Frnt		0.850			0.992	
Flt Protected	0.950		0.950			
Satd. Flow (prot)	1587	1420	1708	3415	3581	0
Flt Permitted	0.950		0.950			
Satd. Flow (perm)	1587	1420	1708	3415	3581	0
Headway Factor	1.12	1.12	1.05	1.05	0.96	0.96
Link Speed (mph)	40			45	45	
Link Distance (ft)	3960			1111	2283	
Travel Time (s)	67.5			16.8	34.6	
Volume (vph)	44	197	401	1673	1349	73
Peak Hour Factor	0.67	0.67	0.95	0.95	0.76	0.76
Heavy Vehicles (%)	4%	4%	2%	2%	3%	3%
Adj. Flow (vph)	66	294	422	1761	1775	96
Lane Group Flow (vph)	66	294	422	1761	1871	0
Sign Control	Stop			Free	Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	75.2%
	ICU Level of Service D
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	↙	↘	↙	↑↑	↑↑	↙	
Sign Control	Stop		Free		Free		
Grade	4%		7%		-6%		
Volume (veh/h)	44	197	401	1673	1349	73	
Peak Hour Factor	0.67	0.67	0.95	0.95	0.76	0.76	
Hourly flow rate (vph)	66	294	422	1761	1775	96	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None						
Median storage (veh)							
Upstream signal (ft)	1111						
pX, platoon unblocked	0.45						
vC, conflicting volume	3548	936	1871				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	5450	936	1871				
tC, single (s)	6.9	7.0	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	0	0	0				
cM capacity (veh/h)	0	263	318				
Direction Lane #	EB 1	EB 2	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	66	294	422	881	881	1183	688
Volume Left	66	0	422	0	0	0	0
Volume Right	0	294	0	0	0	0	96
cSH	0	263	318	1700	1700	1700	1700
Volume to Capacity	Err	1.12	1.33	0.52	0.52	0.70	0.40
Queue Length 95th (ft)	Err	316	517	0	0	0	0
Control Delay (s)	Err	133.0	200.7	0.0	0.0	0.0	0.0
Lane LOS	F	F	F				
Approach Delay (s)	Err	38.8		0.0			
Approach LOS	F						
Intersection Summary							
Average Delay	Err						
Intersection Capacity Utilization	75.2%		ICU Level of Service			D	
Analysis Period (min)	15						

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↙	↘	↗	↖	↑↑	↗	↖	↑↑	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	15	15	15	12	12	14	11	12	14	11	12	12
Grade (%)		6%			5%			3%				-7%
Total Lost time (s)		4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00		0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	
Frt		0.96		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	
Flt Protected		0.98		0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1858		1639	1647	1647	1685	3486	1664	1770	3658	
Flt Permitted		0.98		0.95	0.95	1.00	0.21	1.00	1.00	0.13	1.00	
Satd. Flow (perm)		1858		1639	1647	1647	364	3486	1664	245	3658	
Volume (vph)	5	3	4	364	9	154	6	1249	462	146	1054	9
Peak-hour factor, PHF	0.56	0.56	0.56	0.89	0.89	0.89	0.99	0.99	0.99	0.82	0.82	0.82
Adj. Flow (vph)	9	5	7	409	10	173	6	1262	467	178	1285	11
RTOR Reduction (vph)	0	7	0	0	0	103	0	0	193	0	1	0
Lane Group Flow (vph)	0	14	0	205	214	70	6	1262	274	178	1295	0
Turn Type	Split		Split		pm+ov	Perm	pm+ov		pm+pt			
Protected Phases	4	4	8		8	1	2		8	1	6	
Permitted Phases					8	2	2		6			
Actuated Green, G (s)	1.4		10.3		10.3	16.7	22.9	22.9	33.2	34.8	34.8	
Effective Green, g (s)	3.4		12.3		12.3	20.2	26.4	26.4	38.7	38.3	38.3	
Actuated g/C Ratio	0.05		0.19		0.19	0.31	0.40	0.40	0.59	0.58	0.58	
Clearance Time (s)	6.0		6.0		6.0	5.5	7.5	7.5	6.0	5.5	7.5	
Vehicle Extension (s)	3.0		3.0		3.0	3.0	5.0	5.0	3.0	3.0	5.0	
Lane Grp Cap (vph)	96		305		307	604	146	1394	976	325	2123	
v/s Ratio Prot	c0.01		0.13		c0.13	0.01		c0.36	0.05	0.07	c0.35	
v/s Ratio Perm						0.03	0.02		0.11	0.25		
v/c Ratio	0.15		0.67		0.70	0.12	0.04	0.91	0.28	0.55	0.61	
Uniform Delay, d1	29.9		25.0		25.1	16.5	12.1	18.6	6.8	11.4	9.0	
Progression Factor	1.00		1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.7		5.7		6.7	0.1	0.2	9.1	0.2	1.9	0.8	
Delay (s)	30.6		30.7		31.8	16.6	12.3	27.8	6.9	13.3	9.8	
Level of Service	C		C		C	B	B	C	A	B	A	
Approach Delay (s)	30.6				27.0			22.1			10.2	
Approach LOS	C				C			C			B	

Intersection Summary			
HCM Average Control Delay	18.3	HCM Level of Service	B
HCM Volume to Capacity ratio	0.77		
Actuated Cycle Length (s)	66.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	69.6%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Lane Group	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↔	↖	↗	↗	↖	↕	↗	↖	↕
Volume (vph)	3	364	9	154	6	1249	462	146	1054
Lane Group Flow (vph)	21	205	214	173	6	1262	467	178	1296
Turn Type		Split		pm+ov	Perm		pm+ov	pm+pt	
Protected Phases	4	8	8	1		2	8	1	6
Permitted Phases				8	2		2	6	
Detector Phases	4	8	8	1	2	2	8	1	6
Minimum Initial (s)	4.0	7.0	7.0	4.0	10.0	10.0	7.0	4.0	10.0
Minimum Split (s)	10.0	13.0	13.0	9.5	17.5	17.5	13.0	9.5	17.5
Total Split (s)	10.0	17.0	17.0	12.0	31.0	31.0	17.0	12.0	43.0
Total Split (%)	14.3%	24.3%	24.3%	17.1%	44.3%	44.3%	24.3%	17.1%	61.4%
Yellow Time (s)	4.0	4.0	4.0	5.5	5.5	5.5	4.0	5.5	5.5
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0
Lead/Lag				Lead	Lag	Lag		Lead	
Lead-Lag Optimize?				Yes	Yes	Yes		Yes	
Recall Mode:	None	None	None	None	Min	Min	None	None	Min
v/c Ratio	0.12	0.63	0.65	0.24	0.04	0.86	0.37	0.52	0.58
Control Delay	25.4	33.6	34.6	5.2	14.2	24.7	1.3	13.5	9.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.4	33.6	34.6	5.2	14.2	24.7	1.3	13.5	9.4
Queue Length 50th (ft)	5	70	73	5	1	195	0	21	110
Queue Length 95th (ft)	14	#174	#184	42	9	#405	14	66	210
Internal Link Dist (ft)	105		2012			2203			2327
Turn Bay Length (ft)		250		250	73		350	183	
Base Capacity (vph)	170	341	343	713	145	1502	1265	345	2271
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.60	0.62	0.24	0.04	0.84	0.37	0.52	0.57

Intersection Summary

Cycle Length: 70

Actuated Cycle Length: 62.4

Natural Cycle: 70

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Woodland Road/Private Driveway & Route 611

01	02	04	08
12 s	31 s	10 s	17 s
06			
49 s			

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

4: Meadowside Road/Trinity Hill Road & Route 611



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↗	↕		↗	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	13	13	12	12	12	11	12	12	11	12	12
Grade (%)	2%			8%			1%			-1%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	0.865			0.879								
Flt Protected					0.997		0.950			0.950		
Satd. Flow (prot)	0	1648	0	0	1567	0	1702	3522	0	1670	3455	0
Flt Permitted					0.997		0.950			0.950		
Satd. Flow (perm)	0	1648	0	0	1567	0	1702	3522	0	1670	3455	0
Headway Factor	0.97	0.97	0.97	1.05	1.05	1.05	1.05	1.01	1.01	1.04	0.99	0.99
Link Speed (mph)	35			35			45			45		
Link Distance (ft)	158			1027			2407			3261		
Travel Time (s)	3.1			20.0			36.5			49.4		
Volume (vph)	0	0	4	1	1	17	4	1402	1	12	1204	1
Peak Hour Factor	0.75	0.75	0.75	0.50	0.50	0.50	0.97	0.97	0.97	0.83	0.83	0.83
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	5%	5%	5%
Adj. Flow (vph)	0	0	5	2	2	34	4	1445	1	14	1451	1
Lane Group Flow (vph)	0	5	0	0	38	0	4	1446	0	14	1452	0
Sign Control	Stop			Stop			Free			Free		

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	48.8%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

4: Meadowside Road/Trinity Hill Road & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↗		↕		↗	
Sign Control	Stop			Stop			Free		Free		Free	
Grade	2%			8%			1%				-1%	
Volume (veh/h)	0	0	4	1	1	17	4	1402	1	12	1204	1
Peak Hour Factor	0.75	0.75	0.75	0.50	0.50	0.50	0.97	0.97	0.97	0.83	0.83	0.83
Hourly flow rate (vph)	0	0	5	2	2	34	4	1445	1	14	1451	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2246	2935	726	2214	2935	723	1452			1446		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2246	2935	726	2214	2935	723	1452			1446		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.2		
tC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	91	86	91	99			97		
cM capacity (veh/h)	18	14	367	23	14	368	462			450		
Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	5	38	4	964	483	14	967	485				
Volume Left	0	2	4	0	0	14	0	0				
Volume Right	5	34	0	0	1	0	0	1				
cSH	367	118	462	1700	1700	450	1700	1700				
Volume to Capacity	0.01	0.32	0.01	0.57	0.28	0.03	0.57	0.29				
Queue Length 95th (ft)	1	32	1	0	0	2	0	0				
Control Delay (s)	15.0	49.5	12.9	0.0	0.0	13.3	0.0	0.0				
Lane LOS	B	E	B				B					
Approach Delay (s)	15.0	49.5	0.0			0.1						
Approach LOS	B	E										
Intersection Summary												
Average Delay			0.7									
Intersection Capacity Utilization			48.8%		ICU Level of Service				A			
Analysis Period (min)			15									

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Lane Group	EB1	EB2	EB3	WB1	WB2	WB3	NB1	NB2	NB3	SB1	SB2	SB3
Lane Configurations	↕			↕			↖	↕		↖	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	10	10	10	11	12	12	11	12	12
Grade (%)		0%			9%			2%				-5%
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.938			0.934			0.998			0.997	
Flt Protected		0.978			0.983		0.950			0.950		
Satd. Flow (prot)	0	1709	0	0	1524	0	1694	3497	0	1753	3617	0
Flt Permitted		0.978			0.983		0.950			0.950		
Satd. Flow (perm)	0	1709	0	0	1524	0	1694	3497	0	1753	3617	0
Headway Factor	1.00	1.00	1.00	1.16	1.16	1.16	1.06	1.01	1.01	1.01	0.97	0.97
Link Speed (mph)		30			30			45			45	
Link Distance (ft)		294			1492			3261			2754	
Travel Time (s)		6.7			33.9			49.4			41.7	
Volume (vph)	20	4	20	13	7	18	31	1368	20	22	1184	31
Peak Hour Factor	0.90	0.90	0.90	0.75	0.90	0.75	0.90	0.97	0.97	0.77	0.77	0.90
Adj. Flow (vph)	22	4	22	17	8	24	34	1410	21	29	1538	34
Lane Group Flow (vph)	0	48	0	0	49	0	34	1431	0	29	1572	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary	
Area Type	Other
Control Type	Unsignalized
Intersection Capacity Utilization	48.9%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Movement	EB	EBT	EBR	WB	WBT	WBR	NB	NBT	NBR	SB	SBT	SBR
Lane Configurations	↕			↕			↖	↕		↖	↕	
Sign Control	Stop			Stop			Free		Free		Free	
Grade	0%			9%			2%		-5%			
Volume (veh/h)	20	4	20	13	7	18	31	1368	20	22	1184	31
Peak Hour Factor	0.90	0.90	0.90	0.75	0.90	0.75	0.90	0.97	0.97	0.77	0.77	0.90
Hourly flow rate (vph)	22	4	22	17	8	24	34	1410	21	29	1538	34
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2414	3112	786	2340	3119	715	1572			1431		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2414	3112	786	2340	3119	715	1572			1431		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	54	93	0	18	94	92			94		
cM capacity (veh/h)	5	10	335	10	9	372	415			471		
Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	49	49	34	940	491	29	1025	547				
Volume Left	22	17	34	0	0	29	0	0				
Volume Right	22	24	0	0	21	0	0	34				
cSH	9	19	415	1700	1700	471	1700	1700				
Volume to Capacity	5.43	2.56	0.08	0.55	0.29	0.06	0.60	0.32				
Queue Length 95th (ft)	Err	164	7	0	0	5	0	0				
Control Delay (s)	Err	1127.9	14.4	0.0	0.0	13.1	0.0	0.0				
Lane LOS	F	F	B				B					
Approach Delay (s)	Err	1127.9	0.3				0.2					
Approach LOS	F	F										
Intersection Summary												
Average Delay	172.3											
Intersection Capacity Utilization	48.9%			ICU Level of Service				A				
Analysis Period (min)	15											

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

6: Woodland Road & School Access



Lane Group	EBL	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	14	11	11
Grade (%)	1%			3%	0%	
Storage Length (ft)		250	100		0	0
Storage Lanes		1	1		1	0
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850			0.962	
Flt Protected			0.950		0.965	
Satd. Flow (prot)	1767	1602	1728	1941	1672	0
Flt Permitted			0.950		0.965	
Satd. Flow (perm)	1767	1602	1728	1941	1672	0
Headway Factor	1.01	0.92	0.98	0.90	1.04	1.04
Link Speed (mph)	40			40	25	
Link Distance (ft)	2092			650	499	
Travel Time (s)	35.7			11.1	13.6	
Volume (vph)	564	47	20	467	60	24
Peak Hour Factor	0.82	0.82	0.79	0.79	0.53	0.53
Heavy Vehicles (%)	7%	7%	6%	6%	2%	2%
Adj. Flow (vph)	688	57	25	591	113	45
Lane Group Flow (vph)	688	57	25	591	158	0
Sign Control	Free			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	41.1%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

6: Woodland Road & School Access



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Sign Control	Free			Free	Stop	
Grade	1%			-3%	0%	
Volume (veh/h)	564	47	20	467	60	24
Peak Hour Factor	0.82	0.82	0.79	0.79	0.53	0.53
Hourly flow rate (vph)	688	57	25	591	113	45

Pedestrians

Lane Width (ft)

Walking Speed (ft/s)

Percent Blockage

Right turn flare (veh)

Median type

TWLTTL

Median storage veh

1

Upstream signal (ft)

pX, platoon unblocked

vC, conflicting volume

745 1330 688

vC1, stage 1 conf vol

688

vC2, stage 2 conf vol

642

vCu, unblocked vol

745 1330 688

tC, single (s)

4.2 6.4 6.2

tC, 2 stage (s)

5.4

tF (s)

2.3 3.5 3.3

p0 queue free %

97 63 90

cM capacity (veh/h)

845 305 446

Direction Lane #	EB 1	EB 2	WB 1	WB 2	NB 1
Volume Total	688	57	25	591	158
Volume Left	0	0	25	0	113
Volume Right	0	57	0	0	45
cSH	1700	1700	845	1700	336
Volume to Capacity	0.40	0.03	0.03	0.35	0.47
Queue Length 95th (ft)	0	0	2	0	60
Control Delay (s)	0.0	0.0	9.4	0.0	25.0
Lane LOS			A		C
Approach Delay (s)	0.0		0.4		25.0
Approach LOS					C

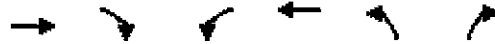
Intersection Summary

Average Delay	2.8
Intersection Capacity Utilization	41.1%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

7: Woodland Road & Bowman Road



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↑	↘	↙
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	14	10	10
Grade (%)	1%			1%	-1%	
Storage Length (ft)		250	100		0	0
Storage Lanes		1	1		1	0
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850			0.908	
Flt Protected			0.950		0.984	
Satd. Flow (prot)	1853	1680	1727	1939	1561	0
Flt Permitted			0.950		0.984	
Satd. Flow (perm)	1853	1680	1727	1939	1561	0
Headway Factor	1.01	0.92	1.01	0.92	1.09	1.09
Link Speed (mph)	40			40	35	
Link Distance (ft)	650			936	704	
Travel Time (s)	11.1			16.0	13.7	
Volume (vph)	554	33	57	449	38	80
Peak Hour Factor	0.72	0.72	0.80	0.80	0.90	0.90
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	769	46	71	561	42	89
Lane Group Flow (vph)	769	46	71	561	131	0
Sign Control	Free			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	49.5%
	ICU Level of Service A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

7: Woodland Road & Bowman Road



Movement	EB1	EB2	WB1	WB2	NB1	NB2
Lane Configurations	↑	↗	↖	↑	↘	↙
Sign Control	Free			Free	Stop	
Grade	1%			1%	-1%	
Volume (veh/h)	554	33	57	449	38	80
Peak Hour Factor	0.72	0.72	0.80	0.80	0.90	0.90
Hourly flow rate (vph)	769	46	71	561	42	89
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLT		
Median storage (veh)				1		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			815	1473	769	
vC1, stage 1 conf vol				769		
vC2, stage 2 conf vol				704		
vCu, unblocked vol			815	1473	769	
tC, single (s)			4.1	6.4	6.2	
tC, 2 stage (s)				5.4		
tF (s)			2.2	3.5	3.3	
p0 queue free %			91	84	78	
cM capacity (veh/h)			803	264	401	

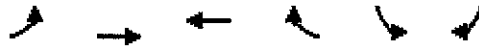
Direction Lane #	EB 1	EB 2	WB 1	WB 2	NB 1
Volume Total	769	46	71	561	131
Volume Left	0	0	71	0	42
Volume Right	0	46	0	0	89
cSH	1700	1700	803	1700	344
Volume to Capacity	0.45	0.03	0.09	0.33	0.38
Queue Length 95th (ft)	0	0	7	0	43
Control Delay (s)	0.0	0.0	9.9	0.0	21.8
Lane LOS			A		C
Approach Delay (s)	0.0		1.1		21.8
Approach LOS					C

Intersection Summary	
Average Delay	2.3
Intersection Capacity Utilization	49.5% ICU Level of Service A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

8: Woodland Road & Meadowside Road



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑	↑		↗	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	14	14	10	10
Grade (%)		-4%	2%		-6%	
Storage Length (ft)	100			0	0	0
Storage Lanes	1			0	1	0
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Fr _t			0.999		0.966	
Flt Protected	0.950				0.964	
Satd. Flow (prot)	1805	2027	1927	0	1668	0
Flt Permitted	0.950				0.964	
Satd. Flow (perm)	1805	2027	1927	0	1668	0
Headway Factor	0.97	0.89	0.93	0.93	1.05	1.05
Link Speed (mph)		40	40		35	
Link Distance (ft)		936	400		1342	
Travel Time (s)		16.0	6.8		26.1	
Volume (vph)	3	632	503	5	9	3
Peak Hour Factor	0.84	0.84	0.77	0.77	0.50	0.50
Heavy Vehicles (%)	2%	2%	4%	4%	2%	2%
Adj. Flow (vph)	4	752	653	6	18	6
Lane Group Flow (vph)	4	752	659	0	24	0
Sign Control		Free	Free		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	43.3%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

8: Woodland Road & Meadowside Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑	↗		↘	↙
Sign Control		Free	Free		Stop	
Grade		-4%	2%		-6%	
Volume (veh/h)	3	632	503	5	9	3
Peak Hour Factor	0.84	0.84	0.77	0.77	0.50	0.50
Hourly flow rate (vph)	4	752	653	6	18	6
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL		
Median storage veh				1		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	660				1416	656
vC1, stage 1 conf vol					656	
vC2, stage 2 conf vol					760	
vCu, unblocked vol	660				1416	656
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)					5.4	
tF (s)	2.2				3.5	3.3
p0 queue free %	100				94	99
cM capacity (veh/h)	928				291	466

Direction, Lane #	EB 1	EB 2	WB 1	SB 1
Volume Total	4	752	660	24
Volume Left	4	0	0	18
Volume Right	0	0	6	6
cSH	928	1700	1700	321
Volume to Capacity	0.00	0.44	0.39	0.07
Queue Length 95th (ft)	0	0	0	6
Control Delay (s)	8.9	0.0	0.0	17.1
Lane LOS	A			C
Approach Delay (s)	0.0		0.0	17.1
Approach LOS				C

Intersection Summary			
Average Delay	0.3		
Intersection Capacity Utilization	43.3%	ICU Level of Service	A
Analysis Period (min)	15		

2017 Projected Conditions - With Site-Related Recommendations
 Friday P.M. Peak Hour

9: Woodland Road & Carlton Road



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			↕	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	-3%			5%	4%	
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.992				0.923	
Frt Protected	0.955			0.997		
Satd. Flow (prot)	1672	0	0	1840	1613	0
Frt Permitted	0.955			0.997		
Satd. Flow (perm)	1672	0	0	1840	1613	0
Headway Factor	1.07	1.07	1.01	1.01	1.07	1.07
Link Speed (mph)	40			45	45	
Link Distance (ft)	1794			1439	1446	
Travel Time (s)	30.6			21.8	21.9	
Volume (vph)	180	11	12	190	159	210
Peak Hour Factor	0.82	0.82	0.96	0.96	0.81	0.81
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Adj. Flow (vph)	220	13	13	198	196	259
Lane Group Flow (vph)	233	0	0	210	455	0
Sign Control	Stop			Free	Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	38.5%
	ICU Level of Service A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

9: Woodland Road & Carlton Road



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔			↕	↕	
Sign Control	Stop			Free	Free	
Grade	-3%			-5%	4%	
Volume (veh/h)	180	11	12	190	159	210
Peak Hour Factor	0.82	0.82	0.96	0.96	0.81	0.81
Hourly flow rate (vph)	220	13	12	198	196	259

Pedestrians

Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	
Median type	None
Median storage (veh)	
Upstream signal (ft)	
pX, platoon unblocked	
vC, conflicting volume	549 326 456
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	549 326 456
tC, single (s)	6.4 6.2 4.1
tC, 2 stage (s)	
tF (s)	3.5 3.3 2.2
p0 queue free %	55 98 99
cM capacity (veh/h)	492 715 1105

Direction Lane #	EB 1	NB 1	SB 1
Volume Total	233	210	456
Volume Left	220	12	0
Volume Right	13	0	259
cSH	501	1105	1700
Volume to Capacity	0.47	0.01	0.27
Queue Length 95th (ft)	61	1	0
Control Delay (s)	18.3	0.6	0.0
Lane LOS	C	A	
Approach Delay (s)	18.3	0.6	0.0
Approach LOS	C		

Intersection Summary			
Average Delay	4.9		
Intersection Capacity Utilization	38.5%	ICU Level of Service	A
Analysis Period (min)	15		

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

10: Route 940 & Carlton Road/Private Driveway



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↕			↕		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	11	10	11	11	11	11	11	8	8	8
Grade (%)	-4%			4%			1%			-1%		
Total Lost time (s)	4.0			4.0			4.0			4.0		
Lane Util. Factor	1.00			1.00			1.00			1.00		
Fr _t	0.98			1.00			0.89			0.96		
Fl _t Protected	1.00			0.95			0.99			0.99		
Satd. Flow (prot)	1803			1619			1764			1551		
Fl _t Permitted	1.00			0.24			1.00			0.93		
Satd. Flow (perm)	1798			402			1764			1332		
Volume (vph)	4	578	89	271	523	1	67	3	299	3	9	4
Peak-hour factor, PHF	0.86	0.86	0.86	0.98	0.98	0.98	0.87	0.87	0.87	0.60	0.60	0.60
Adj. Flow (vph)	5	672	103	277	534	1	77	3	344	5	15	7
RTOR Reduction (vph)	0	9	0	0	0	0	0	261	0	0	6	0
Lane Group Flow (vph)	0	771	0	277	535	0	0	163	0	0	21	0
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	4%	4%	4%	2%	2%	2%
Turn Type	Perm			pm+pt			Perm			Perm		
Protected Phases	2			1			6			8		
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	25.4			37.4			37.4			8.9		
Effective Green, g (s)	27.4			39.4			39.4			9.9		
Actuated g/C Ratio	0.48			0.69			0.69			0.17		
Clearance Time (s)	6.0			6.0			6.0			5.0		
Vehicle Extension (s)	6.0			3.0			6.0			3.0		
Lane Grp Cap (vph)	860			446			1213			251		
v/s Ratio Prot				c0.09			0.30					
v/s Ratio Perm	c0.43			0.34						c0.11		
v/c Ratio	0.90			0.62			0.44			0.65		
Uniform Delay, d1	13.6			7.0			4.0			22.1		
Progression Factor	1.00			1.00			1.00			1.00		
Incremental Delay, d2	13.1			2.7			0.7			5.7		
Delay (s)	26.7			9.7			4.7			27.7		
Level of Service	C			A			A			C		
Approach Delay (s)	26.7						6.4			27.7		
Approach LOS	C						A			C		

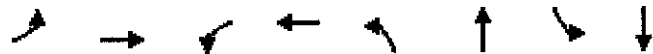
Intersection Summary			
HCM Average Control Delay	18.8	HCM Level of Service	B
HCM Volume to Capacity ratio	0.80		
Actuated Cycle Length (s)	57.3	Sum of lost time (s)	12.0
Intersection Capacity Utilization	102.6%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

10: Route 940 & Carlton Road/Private Driveway



Lane Group	EFL	EFT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		↕	↗	↖		↕		↕
Volume (vph)	4	578	271	523	67	3	3	9
Lane Group Flow (vph)	0	780	277	535	0	424	0	27
Turn Type	Perm		prn+pt		Perm		Perm	
Protected Phases		2	1	6		8		4
Permitted Phases	2		6		8		4	
Detector Phases	2	2	1	6	8	8	4	4
Minimum Initial (s)	10.0	10.0	4.0	10.0	6.0	6.0	6.0	6.0
Minimum Split (s)	16.0	16.0	10.0	16.0	11.0	11.0	11.0	11.0
Total Split (s)	33.0	33.0	12.0	45.0	15.0	15.0	15.0	15.0
Total Split (%)	55.0%	55.0%	20.0%	75.0%	25.0%	25.0%	25.0%	25.0%
Yellow Time (s)	4.5	4.5	4.0	4.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	2.0	1.5	1.5	1.5	1.5	1.5
Lead/Lag	Lag	Lag	Lead					
Lead-Lag Optimize?	Yes	Yes	Yes					
Recall Mode	Min	Min	None	Min	None	None	None	None
v/c Ratio		0.90	0.62	0.44		0.83		0.11
Control Delay		27.4	10.6	5.5		20.8		18.1
Queue Delay		0.0	0.0	0.0		0.0		0.0
Total Delay		27.4	10.6	5.5		20.8		18.1
Queue Length 50th (ft)		233	30	69		35		6
Queue Length 95th (ft)		#418	#66	117		#155		15
Internal Link Dist (ft)		1322		1070		1366		73
Turn Bay Length (ft)			100					
Base Capacity (vph)		893	449	1228		531		259
Starvation Cap Reductn		0	0	0		0		0
Spillback Cap Reductn		0	0	0		0		0
Storage Cap Reductn		0	0	0		0		0
Reduced v/c Ratio		0.87	0.62	0.44		0.80		0.10

Intersection Summary

Cycle Length: 60

Actuated Cycle Length: 57.4

Natural Cycle: 60

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 10: Route 940 & Carlton Road/Private Driveway

↖ a1	↗ a2	↕ a4
12s	12s	15s
↖ a6		↕ a8
15s		15s

2017 Projected Conditions - With Site-Related Recommendations
 Friday P.M. Peak Hour

11: Route 940 & Route 390



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	11	11	11	11	14	14
Grade (%)		-2%	2%		-4%	
Total Lost time (s)		4.0	4.0		4.0	
Lane Util. Factor		1.00	1.00		1.00	
Fr _t		1.00	0.99		0.87	
Fl _t Protected		0.97	1.00		1.00	
Satd. Flow (prot)		1765	1771		1724	
Fl _t Permitted		0.47	1.00		1.00	
Satd. Flow (perm)		857	1771		1724	
Volume (vph)	534	346	339	18	12	457
Peak-hour factor, PHF	0.89	0.89	0.93	0.93	0.80	0.80
Adj. Flow (vph)	600	389	365	19	15	571
RTOR Reduction (vph)	0	0	1	0	512	0
Lane Group Flow (vph)	0	989	383	0	74	0
Heavy Vehicles (%)	2%	2%	2%	2%	4%	4%
Turn Type	pm+pt					
Protected Phases	5	2	6		4	
Permitted Phases	2					
Actuated Green, G (s)		94.1	59.1		9.9	
Effective Green, g (s)		96.1	61.1		11.9	
Actuated g/C Ratio		0.83	0.53		0.10	
Clearance Time (s)		6.0	6.0		6.0	
Vehicle Extension (s)		6.0	6.0		3.0	
Lane Grp Cap (vph)		953	933		177	
v/s Ratio Prot		c0.28	0.22		c0.04	
v/s Ratio Perm		c0.58				
v/c Ratio		1.04	0.41		0.42	
Uniform Delay, d1		10.0	16.6		48.8	
Progression Factor		1.00	1.00		1.00	
Incremental Delay, d2		39.4	0.8		1.6	
Delay (s)		49.3	17.4		50.4	
Level of Service		D	B		D	
Approach Delay (s)		49.3	17.4		50.4	
Approach LOS		D	B		D	
Intersection Summary						
HCM Average Control Delay		43.4		HCM Level of Service		D
HCM Volume to Capacity ratio		0.96				
Actuated Cycle Length (s)		116.0		Sum of lost time (s)		8.0
Intersection Capacity Utilization		105.6%		ICU Level of Service		G
Analysis Period (min)		15				

c Critical Lane Group

2017 Projected Conditions - With Site-Related Recommendations
 Friday P.M. Peak Hour

11: Route 940 & Route 390

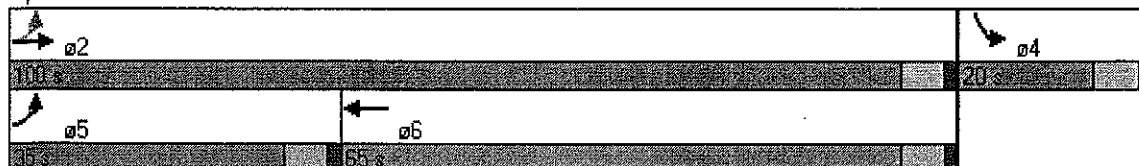


Lane Group	EBL	EBT	WBT	SBL
Lane Configurations		↕	↕	↕
Volume (vph)	534	346	339	12
Lane Group Flow (vph)	0	989	384	586
Turn Type	prn+pt			
Protected Phases	5	2	6	4
Permitted Phases	2			
Detector Phases	5	2.5	6	4
Minimum Initial (s)	4.0	10.0	10.0	7.0
Minimum Split (s)	10.0	16.0	16.0	13.0
Total Split (s)	35.0	100.0	65.0	20.0
Total Split (%)	29.2%	83.3%	54.2%	16.7%
Yellow Time (s)	4.5	4.5	4.5	4.5
All-Red Time (s)	1.5	1.5	1.5	1.5
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	Min	Min	None
v/c Ratio		1.05	0.41	0.85
Control Delay		54.5	18.6	14.5
Queue Delay		0.0	0.0	0.0
Total Delay		54.5	18.6	14.5
Queue Length 50th (ft)		~398	158	10
Queue Length 95th (ft)		#784	259	51
Internal Link Dist (ft)		491	1298	1509
Turn Bay Length (ft)				
Base Capacity (vph)		943	933	725
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		1.05	0.41	0.81

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 115.9
 Natural Cycle: 150
 Control Type: Semi Act-Uncoord
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

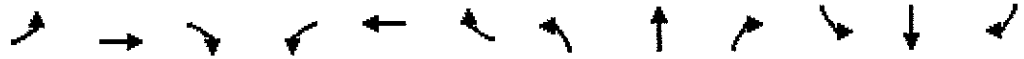
Splits and Phases: 11: Route 940 & Route 390



2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

12: Route 940 & Route 191/Red Rock Road



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↕			↕		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	8	8	8	11	11	11
Grade (%)	-2%			2%			-3%			-1%		
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frts		0.998			0.962			0.979			0.974	
Flt Protected		0.994			0.999			0.989			0.963	
Satd. Flow (prot)	0	1804	0	0	1713	0	0	1587	0	0	1697	0
Flt Permitted		0.994			0.999			0.989			0.963	
Satd. Flow (perm)	0	1804	0	0	1713	0	0	1587	0	0	1697	0
Headway Factor	1.03	1.03	1.03	1.06	1.06	1.06	1.18	1.18	1.18	1.04	1.04	1.04
Link Speed (mph)		45			45			35			45	
Link Distance (ft)		1662			865			282			1220	
Travel Time (s)		25.2			13.1			5.5			18.5	
Volume (vph)	46	307	5	6	325	132	5	13	3	103	5	26
Peak Hour Factor	0.90	0.90	0.90	0.84	0.84	0.84	0.80	0.80	0.80	0.87	0.87	0.87
Adj. Flow (vph)	51	341	6	7	387	157	6	16	4	118	6	30
Lane Group Flow (vph)	0	398	0	0	551	0	0	26	0	0	154	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 66.3% ICU Level of Service C

Analysis Period (min) 15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

12: Route 940 & Route 191/Red Rock Road



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Free			Free			Stop			Stop	
Grade		-2%			2%			-3%			-1%	
Volume (veh/h)	46	307	5	6	325	132	5	13	3	103	5	26
Peak Hour Factor	0.90	0.90	0.90	0.84	0.84	0.84	0.80	0.80	0.80	0.87	0.87	0.87
Hourly flow rate (vph)	51	341	6	7	387	157	6	16	4	118	6	30
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	544			347			959	1004	344	938	929	465
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	544			347			959	1004	344	938	929	465
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	95			99			97	93	99	46	98	95
cM capacity (veh/h)	1025			1212			212	229	699	221	253	597
Direction Lane #												
	EB 1	WB 1	NB 1	SB 1								
Volume Total	398	551	26	154								
Volume Left	51	7	6	118								
Volume Right	6	157	4	30								
cSH	1025	1212	248	253								
Volume to Capacity	0.05	0.01	0.11	0.61								
Queue Length 95th (ft)	4	0	9	91								
Control Delay (s)	1.6	0.2	21.2	39.2								
Lane LOS	A	A	C	E								
Approach Delay (s)	1.6	0.2	21.2	39.2								
Approach LOS			C	E								
Intersection Summary												
Average Delay			6.5									
Intersection Capacity Utilization		66.3%		ICU Level of Service		C						
Analysis Period (min)			15									

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

13: Woodland Road & Western Site Driveway



Lane Group	EBL	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	14	12	14
Grade (%)	-3%			2%	0%	
Storage Length (ft)		250	100		0	0
Storage Lanes		1	1		1	1
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Fr _t		0.850				0.850
Fl _t Protected			0.950		0.950	
Satd. Flow (prot)	1891	1714	1752	1967	1770	1689
Fl _t Permitted			0.950		0.950	
Satd. Flow (perm)	1891	1714	1752	1967	1770	1689
Headway Factor	0.98	0.90	1.01	0.93	1.00	0.92
Link Speed (mph)	40			40	20	
Link Distance (ft)	863			1227	233	
Travel Time (s)	14.7			20.9	7.9	
Volume (vph)	261	380	24	325	184	30
Peak Hour Factor	0.82	0.90	0.90	0.75	0.90	0.90
Adj. Flow (vph)	318	422	27	433	204	33
Lane Group Flow (vph)	318	422	27	433	204	33
Sign Control	Free			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	36.8%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

13: Woodland Road & Western Site Driveway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Sign Control	Free			Free	Stop	
Grade	-3%			2%	0%	
Volume (veh/h)	261	380	24	325	184	30
Peak Hour Factor	0.82	0.90	0.90	0.75	0.90	0.90
Hourly flow rate (vph)	318	422	27	433	204	33
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			318		805	318
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			318		805	318
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
IF (s)			2.2		3.5	3.3
p0 queue free %			98		41	95
cM capacity (veh/h)			1242		344	722

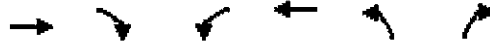
Direction Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volume Total	318	422	27	433	204	33
Volume Left	0	0	27	0	204	0
Volume Right	0	422	0	0	0	33
cSH	1700	1700	1242	1700	344	722
Volume to Capacity	0.19	0.25	0.02	0.25	0.59	0.05
Queue Length 95th (ft)	0	0	2	0	91	4
Control Delay (s)	0.0	0.0	8.0	0.0	29.7	10.2
Lane LOS			A		D	B
Approach Delay (s)	0.0		0.5		26.9	
Approach LOS					D	

Intersection Summary		
Average Delay		4.6
Intersection Capacity Utilization	36.8%	ICU Level of Service A
Analysis Period (min)		15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

14: Woodland Road & Eastern Site Driveway



Lane Group	EBT	EBE	WBL	WBT	NBL	NBE
Lane Configurations	↑	↑	↑	↑	↑	↑
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	14	12	14
Grade (%)	-3%			2%	0%	
Storage Length (ft)		250	100		0	0
Storage Lanes		1	1		1	1
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Fr _t		0.850			0.850	
Fr _t Protected			0.950		0.950	
Satd. Flow (prot)	1891	1714	1752	1967	1770	1689
Fr _t Permitted			0.950		0.950	
Satd. Flow (perm)	1891	1714	1752	1967	1770	1689
Headway Factor	0.98	0.90	1.01	0.93	1.00	0.92
Link Speed (mph)	40			40	20	
Link Distance (ft)	1227			2146	250	
Travel Time (s)	20.9			36.6	8.5	
Volume (vph)	161	130	58	165	184	30
Peak Hour Factor	0.82	0.90	0.90	0.75	0.90	0.90
Adj. Flow (vph)	196	144	64	220	204	33
Lane Group Flow (vph)	196	144	64	220	204	33
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type	Other
Control Type	Unsignalized
Intersection Capacity Utilization	32.0%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Friday P.M. Peak Hour

14: Woodland Road & Eastern Site Driveway



Movement	EB1	EB2	WB1	WB2	NB1	NB2
Lane Configurations	↑	↑	↑	↑	↑	↑
Sign Control	Free			Free	Stop	
Grade	-3%			2%	0%	
Volume (veh/h)	161	130	58	165	184	30
Peak Hour Factor	0.82	0.90	0.90	0.75	0.90	0.90
Hourly flow rate (vph)	196	144	64	220	204	33
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			196		545	196
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			196		545	196
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			95		57	96
cM capacity (veh/h)			1376		476	845
Direction Lane #						
	EB1	EB2	WB1	WB2	NB1	NB2
Volume Total	196	144	64	220	204	33
Volume Left	0	0	64	0	204	0
Volume Right	0	144	0	0	0	33
cSH	1700	1700	1376	1700	476	845
Volume to Capacity	0.12	0.08	0.05	0.13	0.43	0.04
Queue Length 95th (ft)	0	0	4	0	53	3
Control Delay (s)	0.0	0.0	7.7	0.0	18.1	9.4
Lane LOS			A		C	A
Approach Delay (s)	0.0		1.8		16.9	
Approach LOS					C	
Intersection Summary						
Average Delay			5.2			
Intersection Capacity Utilization			32.0%		ICU Level of Service	A
Analysis Period (min)			15			

2017 Projected Conditions - With Site-Related Recommendations
 Saturday P.M. Peak Hour

1: Route 314 (Eastern Leg) & Route 611



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗	↕		↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	13	12	11	11	11	12
Grade (%)	-6%		-2%			-5%
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	0.95		1.00	0.95
Fr _t	1.00	0.85	0.98		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1883	1631	3389		1753	3628
Flt Permitted	0.95	1.00	1.00		0.06	1.00
Satd. Flow (perm)	1883	1631	3389		116	3628
Volume (vph)	172	75	1786	261	79	1477
Peak-hour factor, PHF	0.90	0.90	0.94	0.94	0.94	0.94
Adj. Flow (vph)	191	83	1900	278	84	1571
RTOR Reduction (vph)	0	68	0	0	0	0
Lane Group Flow (vph)	191	15	2178	0	84	1571
Turn Type		Perm			pm+pt	
Protected Phases	8		2		1	6
Permitted Phases		8			6	
Actuated Green, G (s)	9.0	9.0	55.4		65.3	65.3
Effective Green, g (s)	12.0	12.0	59.4		69.3	69.3
Actuated g/C Ratio	0.13	0.13	0.67		0.78	0.78
Clearance Time (s)	7.0	7.0	8.0		6.0	8.0
Vehicle Extension (s)	3.0	3.0	6.0		3.0	6.0
Lane Grp Cap (vph)	253	219	2254		198	2815
v/s Ratio Prot	c0.10		c0.64		0.03	c0.43
v/s Ratio Perm		0.01			0.30	
v/c Ratio	0.75	0.07	0.97		0.42	0.56
Uniform Delay, d1	37.2	33.8	14.0		19.9	4.0
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	12.1	0.1	12.3		1.5	0.5
Delay (s)	49.3	33.9	26.3		21.4	4.5
Level of Service	D	C	C		C	A
Approach Delay (s)	44.6		26.3			5.3
Approach LOS	D		C			A

Intersection Summary			
HCM Average Control Delay		19.1	HCM Level of Service B
HCM Volume to Capacity ratio		0.91	
Actuated Cycle Length (s)		89.3	Sum of lost time (s) 12.0
Intersection Capacity Utilization		81.6%	ICU Level of Service D
Analysis Period (min)		15	
c Critical Lane Group			

2017 Projected Conditions - With Site-Related Recommendations
 Saturday P.M. Peak Hour

1: Route 314 (Eastern Leg) & Route 611



Lane Group	WBL	WBR	NBT	SBL	SBT
Lane Configurations	↙	↗	↕	↘	↕
Volume (vph)	172	75	1786	79	1477
Lane Group Flow (vph)	191	83	2178	84	1571
Turn Type	Perm		pm+pt		
Protected Phases	8		2	1	6
Permitted Phases		8		6	
Detector Phases	8	8	2	1	6
Minimum Initial (s)	1.0	1.0	15.0	1.0	15.0
Minimum Split (s)	8.0	8.0	23.0	7.0	23.0
Total Split (s)	16.0	16.0	63.0	11.0	74.0
Total Split (%)	17.8%	17.8%	70.0%	12.2%	82.2%
Yellow Time (s)	5.0	5.0	6.0	6.0	6.0
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0
Lead/Lag			Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	
Recall Mode	None	None	Min	None	Min
v/c Ratio	0.75	0.29	0.95	0.39	0.56
Control Delay	55.9	12.7	25.6	11.5	4.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	55.9	12.7	25.6	11.5	4.9
Queue Length 50th (ft)	106	3	557	9	142
Queue Length 95th (ft)	#211	43	#809	39	180
Internal Link Dist (ft)	1091		2024		1031
Turn Bay Length (ft)		72		175	
Base Capacity (vph)	257	290	2288	215	2823
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.74	0.29	0.95	0.39	0.56

Intersection Summary
 Cycle Length: 90
 Actuated Cycle Length: 88
 Natural Cycle: 90
 Control Type: Semi Act-Uncoord
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Route 314 (Eastern Leg) & Route 611

↙ ø1	↕ ø2		
11 s	63 s		
↘ ø6			↘ ø8
74 s			16 s

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↙	↗	↙	↑↑	↑↑	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	12	12	12	12
Grade (%)	4%			7%	-6%	
Storage Length (ft)	50	0	143			0
Storage Lanes	1	1	1			0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95
Fr _t		0.850			0.995	
Flt Protected	0.950		0.950			
Satd. Flow (prot)	1587	1420	1708	3415	3627	0
Flt Permitted	0.950		0.950			
Satd. Flow (perm)	1587	1420	1708	3415	3627	0
Headway Factor	1.12	1.12	1.05	1.05	0.96	0.96
Link Speed (mph)	40			45	45	
Link Distance (ft)	3960			1111	2283	
Travel Time (s)	67.5			16.8	34.6	
Volume (vph)	54	160	187	1674	1396	48
Peak Hour Factor	0.96	0.96	0.96	0.96	0.97	0.97
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Adj. Flow (vph)	56	167	195	1744	1439	49
Lane Group Flow (vph)	56	167	195	1744	1488	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	63.8%
	ICU Level of Service B
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations
 Saturday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	↖	↗	↖	↑↑	↑↑		
Sign Control	Stop			Free	Free		
Grade	4%			7%	-6%		
Volume (veh/h)	54	160	187	1674	1396	48	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.97	0.97	
Hourly flow rate (vph)	56	167	195	1744	1439	49	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None						
Median storage (veh)							
Upstream signal (ft)	1111						
pX, platoon unblocked	0.46						
vC, conflicting volume	2725	744	1489				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	3587	744	1489				
tC, single (s)	6.9	7.0	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	0	53	56				
cM capacity (veh/h)	1	352	447				
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	56	167	195	872	872	959	529
Volume Left	56	0	195	0	0	0	0
Volume Right	0	167	0	0	0	0	49
cSH	1	352	447	1700	1700	1700	1700
Volume to Capacity	56.43	0.47	0.44	0.51	0.51	0.56	0.31
Queue Length 95th (ft)	Err	61	54	0	0	0	0
Control Delay (s)	Err	24.1	19.1	0.0	0.0	0.0	0.0
Lane LOS	F	C	C				
Approach Delay (s)	2541.1		1.9	0.0			
Approach LOS	F						
Intersection Summary							
Average Delay	156.2						
Intersection Capacity Utilization	63.8%			ICU Level of Service			B
Analysis Period (min)	15						

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↕	↕	↕	↕	↕	↕	↕	↕	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	15	15	15	12	12	14	11	12	14	11	12	12
Grade (%)		6%			5%			3%				-7%
Total Lost time (s)		4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor		1.00		0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	
Fr _t		0.91		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	
Fl _t Protected		0.99		0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1781		1639	1645	1647	1685	3486	1664	1770	3655	
Fl _t Permitted		0.99		0.95	0.95	1.00	0.26	1.00	1.00	0.13	1.00	
Satd. Flow (perm)		1781		1639	1645	1647	460	3486	1664	244	3655	
Volume (vph)	6	1	16	367	5	79	17	1245	466	94	1061	17
Peak-hour factor, PHF	0.61	0.61	0.61	0.92	0.92	0.92	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	10	2	26	399	5	86	17	1270	476	96	1083	17
RTOR Reduction (vph)	0	24	0	0	0	62	0	0	192	0	2	0
Lane Group Flow (vph)	0	14	0	200	204	24	17	1270	284	96	1098	0
Turn Type	Split		Split		pm+ov	Perm	pm+ov		pm+pt			
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases						8	2		2		6	
Actuated Green, G (s)		2.1		10.3	10.3	15.0	23.0	23.0	33.3	33.2	33.2	
Effective Green, g (s)		4.1		12.3	12.3	18.5	26.5	26.5	38.8	36.7	36.7	
Actuated g/C Ratio		0.06		0.19	0.19	0.28	0.41	0.41	0.60	0.56	0.56	
Clearance Time (s)		6.0		6.0	6.0	5.5	7.5	7.5	6.0	5.5	7.5	
Vehicle Extension (s)		3.0		3.0	3.0	3.0	5.0	5.0	3.0	3.0	5.0	
Lane Grp Cap (vph)		112		310	311	569	187	1419	992	283	2060	
v/s Ratio Prot		c0.01		0.12	c0.12	0.00		c0.36	0.05	0.03	c0.30	
v/s Ratio Perm						0.01	0.04		0.12	0.16		
v/c Ratio		0.12		0.65	0.66	0.04	0.09	0.89	0.29	0.34	0.53	
Uniform Delay, d1		28.8		24.4	24.4	16.9	11.9	18.0	6.4	11.0	8.9	
Progression Factor		1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2		0.5		4.6	4.9	0.0	0.4	8.2	0.2	0.7	0.5	
Delay (s)		29.3		28.9	29.4	16.9	12.3	26.2	6.6	11.7	9.3	
Level of Service		C		C	C	B	B	C	A	B	A	
Approach Delay (s)		29.3			27.0			20.8			9.5	
Approach LOS		C			C			C			A	
Intersection Summary												
HCM Average Control Delay			17.9									B
HCM Volume to Capacity ratio			0.74									
Actuated Cycle Length (s)			65.1						16.0			
Intersection Capacity Utilization			66.6%									C
Analysis Period (min)			15									
c	Critical Lane Group											

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Lane Group	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↕	↙	↕	↗	↙	↕	↗	↙	↕
Volume (vph)	1	367	5	79	17	1245	466	94	1061
Lane Group Flow (vph)	38	200	204	86	17	1270	476	96	1100
Turn Type		Split		pm+ov	Perm		pm+ov	pm+pt	
Protected Phases	4	8	8	1		2	8	1	6
Permitted Phases				8	2		2	6	
Detector Phases	4	8	8	1	2	2	8	1	6
Minimum Initial (s)	4.0	7.0	7.0	4.0	10.0	10.0	7.0	4.0	10.0
Minimum Split (s)	10.0	13.0	13.0	9.5	17.5	17.5	13.0	9.5	17.5
Total Split (s)	10.0	17.0	17.0	12.0	31.0	31.0	17.0	12.0	43.0
Total Split (%)	14.3%	24.3%	24.3%	17.1%	44.3%	44.3%	24.3%	17.1%	61.4%
Yellow Time (s)	4.0	4.0	4.0	5.5	5.5	5.5	4.0	5.5	5.5
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0
Lead/Lag				Lead	Lag	Lag		Lead	
Lead-Lag Optimize?				Yes	Yes	Yes		Yes	
Recall Mode	None	None	None	None	Min	Min	None	None	Min
v/c Ratio	0.20	0.60	0.61	0.13	0.09	0.84	0.37	0.30	0.52
Control Delay	19.1	33.2	33.6	4.7	15.4	24.5	1.3	9.3	9.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.1	33.2	33.6	4.7	15.4	24.5	1.3	9.3	9.5
Queue Length 50th (ft)	5	85	87	0	5	270	0	18	143
Queue Length 95th (ft)	17	#171	#175	26	17	#410	14	37	193
Internal Link Dist (ft)	105		2012			2203			2327
Turn Bay Length (ft)		250		250	73		350	183	
Base Capacity (vph)	190	354	355	635	204	1551	1296	326	2208
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.56	0.57	0.14	0.08	0.82	0.37	0.29	0.50

Intersection Summary

Cycle Length: 70

Actuated Cycle Length: 61.6

Natural Cycle: 65

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Woodland Road/Private Driveway & Route 611

ø1	ø2	ø4	ø8
12 s	31 s	10 s	17 s
ø6			
43 s			

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

4: Meadowside Road/Trinity Hill Road & Route 611



Lane Group	EBL	EBH	EBR	WB	WBT	WBR	NBL	NBT	NBR	SB	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	13	13	12	12	12	11	12	12	11	12	12
Grade (%)		2%			8%			1%				-1%
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Fr _t		0.892			0.923							
Flt Protected		0.990			0.993		0.950			0.950		
Satd. Flow (prot)	0	1683	0	0	1639	0	1702	3522	0	1719	3557	0
Flt Permitted		0.990			0.993		0.950			0.950		
Satd. Flow (perm)	0	1683	0	0	1639	0	1702	3522	0	1719	3557	0
Headway Factor	0.97	0.97	0.97	1.05	1.05	1.05	1.05	1.01	1.01	1.04	0.99	0.99
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		158			1027			2407			3261	
Travel Time (s)		3.1			20.0			36.5			49.4	
Volume (vph)	1	0	4	4	8	16	8	1317	1	7	1164	1
Peak Hour Factor	0.50	0.50	0.50	0.39	0.39	0.39	0.92	0.92	0.92	0.89	0.89	0.89
Adj. Flow (vph)	2	0	8	10	21	41	9	1432	1	8	1308	1
Lane Group Flow (vph)	0	10	0	0	72	0	9	1433	0	8	1309	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 46.4% ICU Level of Service A

Analysis Period (min) 15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

4: Meadowside Road/Trinity Hill Road & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↕			↕			↗	↕		↗	↕		
Sign Control	Stop			Stop			Free		Free				
Grade	2%			8%			1%		-1%				
Volume (veh/h)	1	0	4	4	8	16	8	1317	1	7	1164	1	
Peak Hour Factor	0.50	0.50	0.50	0.39	0.39	0.39	0.92	0.92	0.92	0.89	0.89	0.89	
Hourly flow rate (vph)	2	0	8	10	21	41	9	1432	1	8	1308	1	
Pedestrians													
Lane Width (ft)													
Walking Speed (ft/s)													
Percent Blockage													
Right turn flare (veh)													
Median type	None			None									
Median storage veh													
Upstream signal (ft)													
pX, platoon unblocked													
vC, conflicting volume	2109	2774	654	2127	2774	716	1309			1433			
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	2109	2774	654	2127	2774	716	1309			1433			
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1			
tC, 2 stage (s)													
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2			
p0 queue free %	0	100	98	62	0	89	98			98			
cM capacity (veh/h)	0	18	409	27	18	372	525			470			
Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3					
Volume Total	10	72	9	954	478	8	872	437					
Volume Left	2	10	9	0	0	8	0	0					
Volume Right	8	41	0	0	1	0	0	1					
cSH	0	44	525	1700	1700	470	1700	1700					
Volume to Capacity	Err	1.64	0.02	0.56	0.28	0.02	0.51	0.26					
Queue Length 95th (ft)	Err	180	1	0	0	1	0	0					
Control Delay (s)	Err	514.3	12.0	0.0	0.0	12.8	0.0	0.0					
Lane LOS	F	F	B					B					
Approach Delay (s)	Err	514.3	0.1					0.1					
Approach LOS	F	F											
Intersection Summary													
Average Delay			Err										
Intersection Capacity Utilization			46.4%					ICU Level of Service			A		
Analysis Period (min)			15										

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↗	↕↕		↗	↕↕	
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	10	10	10	11	12	12	11	12	12
Grade (%)	0%			9%			2%			-5%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	0.940			0.958			0.996			0.996		
Flt Protected	0.978			0.971			0.950			0.950		
Satd. Flow (prot)	0	1712	0	0	1544	0	1694	3490	0	1753	3613	0
Flt Permitted	0.978			0.971			0.950			0.950		
Satd. Flow (perm)	0	1712	0	0	1544	0	1694	3490	0	1753	3613	0
Headway Factor	1.00	1.00	1.00	1.16	1.16	1.16	1.06	1.01	1.01	1.01	0.97	0.97
Link Speed (mph)	30			30			45			45		
Link Distance (ft)	294			1492			3261			2754		
Travel Time (s)	6.7			33.9			49.4			41.7		
Volume (vph)	28	6	27	34	6	17	25	1273	36	16	1111	26
Peak Hour Factor	0.90	0.90	0.90	0.72	0.90	0.72	0.90	0.94	0.94	0.95	0.95	0.90
Adj. Flow (vph)	31	7	30	47	7	24	28	1354	38	17	1169	29
Lane Group Flow (vph)	0	68	0	0	78	0	28	1392	0	17	1198	0
Sign Control	Stop			Stop			Free			Free		

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	47.6%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			9%			2%			-5%	
Volume (veh/h)	28	6	27	34	6	17	25	1273	36	16	1111	26
Peak Hour Factor	0.90	0.90	0.90	0.72	0.90	0.72	0.90	0.94	0.94	0.95	0.95	0.90
Hourly flow rate (vph)	31	7	30	47	7	24	28	1354	38	17	1169	29
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1977	2666	599	2081	2661	696	1198			1393		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1977	2666	599	2081	2661	696	1198			1393		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	67	93	0	67	94	95			97		
cM capacity (veh/h)	24	20	445	20	20	383	578			487		

Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	68	78	28	903	490	17	780	419
Volume Left	31	47	28	0	0	17	0	0
Volume Right	30	24	0	0	38	0	0	29
cSH	40	28	578	1700	1700	487	1700	1700
Volume to Capacity	1.68	2.77	0.05	0.53	0.29	0.03	0.46	0.25
Queue Length 95th (ft)	176	233	4	0	0	3	0	0
Control Delay (s)	549.3	1095.2	11.5	0.0	0.0	12.7	0.0	0.0
Lane LOS	F	F	B			B		
Approach Delay (s)	549.3	1095.2	0.2			0.2		
Approach LOS	F	F						

Intersection Summary		
Average Delay		44.1
Intersection Capacity Utilization	47.6%	ICU Level of Service
Analysis Period (min)		15
		A

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

6: Woodland Road & School Access



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑	↓	↑	↓	↓
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	14	11	11
Grade (%)	1%			-3%	0%	
Storage Length (ft)		250	100		0	0
Storage Lanes		1	1		1	0
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Fr _t	0.850					
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1853	1680	1796	2017	1711	0
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1853	1680	1796	2017	1711	0
Headway Factor	1.01	0.92	0.98	0.90	1.04	1.04
Link Speed (mph)	40			40	25	
Link Distance (ft)	2092			650	499	
Travel Time (s)	35.7			11.1	13.6	
Volume (vph)	552	8	3	444	6	0
Peak Hour Factor	0.92	0.92	0.90	0.90	0.63	0.63
Adj. Flow (vph)	600	9	3	493	10	0
Lane Group Flow (vph)	600	9	3	493	10	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type	Other
Control Type	Unsignalized
Intersection Capacity Utilization	39.1%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

6: Woodland Road & School Access



Movement	EB1	EB2	WB1	WB2	NB1	NB2
Lane Configurations	↑	↑	↑	↑	↑	↑
Sign Control	Free			Free	Stop	
Grade	1%			-3%	0%	
Volume (veh/h)	552	8	3	444	6	0
Peak Hour Factor	0.92	0.92	0.90	0.90	0.63	0.63
Hourly flow rate (vph)	600	9	3	493	10	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					TWLT	
Median storage veh					1	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			609		1100	600
vC1, stage 1 conf vol					600	
vC2, stage 2 conf vol					500	
vCu, unblocked vol			609		1100	600
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)					5.4	
IF (s)			2.2		3.5	3.3
p0 queue free %			100		97	100
cM capacity (veh/h)			970		369	501
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	600	9	3	493	10	
Volume Left	0	0	3	0	10	
Volume Right	0	9	0	0	0	
cSH	1700	1700	970	1700	369	
Volume to Capacity	0.35	0.01	0.00	0.29	0.03	
Queue Length 95th (ft)	0	0	0	0	2	
Control Delay (s)	0.0	0.0	8.7	0.0	15.0	
Lane LOS			A		C	
Approach Delay (s)	0.0		0.1		15.0	
Approach LOS					C	
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			39.1%		ICU Level of Service	A
Analysis Period (min)			15			

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

7: Woodland Road & Bowman Road



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↑	↘	↙
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	14	10	10
Grade (%)	1%			1%	-1%	
Storage Length (ft)		250	100		0	0
Storage Lanes		1	1		1	0
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850			0.870	
Frt Protected			0.950		0.998	
Satd. Flow (prot)	1853	1680	1761	1977	1517	0
Frt Permitted			0.950		0.998	
Satd. Flow (perm)	1853	1680	1761	1977	1517	0
Headway Factor	1.01	0.92	1.01	0.92	1.09	1.09
Link Speed (mph)	40			40	35	
Link Distance (ft)	650			936	704	
Travel Time (s)	11.1			16.0	13.7	
Volume (vph)	549	3	60	444	3	76
Peak Hour Factor	0.81	0.81	0.85	0.85	0.50	0.50
Adj. Flow (vph)	678	4	71	522	6	152
Lane Group Flow (vph)	678	4	71	522	158	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type	Other
Control Type	Unsignalized
Intersection Capacity Utilization	47.1%
Analysis Period (min)	15
	ICU Level of Service A

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

7: Woodland Road & Bowman Road



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↑	↘	↙
Sign Control	Free			Free	Stop	
Grade	1%			1%	-1%	
Volume (veh/h)	549	3	60	444	3	76
Peak Hour Factor	0.81	0.81	0.85	0.85	0.50	0.50
Hourly flow rate (vph)	678	4	71	522	6	152
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLT		
Median storage (veh)				1		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			681	1341	678	
vC1, stage 1 conf vol				678		
vC2, stage 2 conf vol				664		
vCu, unblocked vol			681	1341	678	
tC, single (s)			4.1	6.4	6.2	
tC, 2 stage (s)				5.4		
tF (s)			2.2	3.5	3.3	
p0 queue free %			92	98	66	
cM capacity (veh/h)			911	293	452	

Direction Lane #	EB 1	EB 2	WB 1	WB 2	NB 1
Volume Total	678	4	71	522	158
Volume Left	0	0	71	0	6
Volume Right	0	4	0	0	152
cSH	1700	1700	911	1700	443
Volume to Capacity	0.40	0.00	0.08	0.31	0.36
Queue Length 95th (ft)	0	0	6	0	40
Control Delay (s)	0.0	0.0	9.3	0.0	17.6
Lane LOS			A		C
Approach Delay (s)	0.0		1.1		17.6
Approach LOS					C

Intersection Summary			
Average Delay		2.4	
Intersection Capacity Utilization		47.1%	ICU Level of Service A
Analysis Period (min)		15	

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

8: Woodland Road & Meadowside Road



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑	↗		↖	
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	14	14	10	10
Grade (%)		-4%	2%		-6%	
Storage Length (ft)	100			0	0	0
Storage Lanes	1			0	1	0
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.996		0.968	
Flt Protected	0.950				0.963	
Satd. Flow (prot)	1805	2027	1959	0	1669	0
Flt Permitted	0.950				0.963	
Satd. Flow (perm)	1805	2027	1959	0	1669	0
Headway Factor	0.97	0.89	0.93	0.93	1.05	1.05
Link Speed (mph)		40	40		35	
Link Distance (ft)		936	400		1342	
Travel Time (s)		16.0	6.8		26.1	
Volume (vph)	3	622	501	16	10	3
Peak Hour Factor	0.95	0.95	0.93	0.93	0.63	0.63
Adj. Flow (vph)	3	655	539	17	16	5
Lane Group Flow (vph)	3	655	556	0	21	0
Sign Control		Free	Free		Stop	

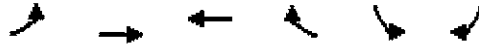
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	42.7%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

8: Woodland Road & Meadows Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑	↗		↘	
Sign Control		Free	Free		Stop	
Grade		-4%	2%		-6%	
Volume (veh/h)	3	622	501	16	10	3
Peak Hour Factor	0.95	0.95	0.93	0.93	0.63	0.63
Hourly flow rate (vph)	3	655	539	17	16	5
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TW	TL	
Median storage veh					1	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	556				1208	547
vC1, stage 1 conf vol					547	
vC2, stage 2 conf vol					661	
vCu, unblocked vol	556				1208	547
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)					5.4	
IF (s)	2.2				3.5	3.3
p0 queue free %	100				95	99
cM capacity (veh/h)	1015				340	537

Direction Lane #	EB 1	EB 2	WB 1	SB 1
Volume Total	3	655	556	21
Volume Left	3	0	0	16
Volume Right	0	0	17	5
cSH	1015	1700	1700	372
Volume to Capacity	0.00	0.39	0.33	0.06
Queue Length 95th (ft)	0	0	0	4
Control Delay (s)	8.6	0.0	0.0	15.3
Lane LOS	A			C
Approach Delay (s)	0.0		0.0	15.3
Approach LOS				C

Intersection Summary			
Average Delay		0.3	
Intersection Capacity Utilization		42.7%	ICU Level of Service A
Analysis Period (min)		15	

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

9: Woodland Road & Carlton Road



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			↑	↓	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	-3%			-5%	4%	
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.995				0.924	
Frt Protected	0.954			0.996		
Satd. Flow (prot)	1675	0	0	1838	1631	0
Frt Permitted	0.954			0.996		
Satd. Flow (perm)	1675	0	0	1838	1631	0
Headway Factor	1.07	1.07	1.01	1.01	1.07	1.07
Link Speed (mph)	40			45	45	
Link Distance (ft)	1794			1439	1446	
Travel Time (s)	30.6			21.8	21.9	
Volume (vph)	148	6	11	136	128	164
Peak Hour Factor	0.82	0.82	0.75	0.75	0.86	0.86
Adj. Flow (vph)	180	7	15	181	149	191
Lane Group Flow (vph)	187	0	0	196	340	0
Sign Control	Stop			Free	Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	32.0%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

9: Woodland Road & Carlton Road



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘ ↙			↑	↑	
Sign Control	Stop			Free	Free	
Grade	-3%			-5%	4%	
Volume (veh/h)	148	6	11	136	128	164
Peak Hour Factor	0.82	0.82	0.75	0.75	0.86	0.86
Hourly flow rate (vph)	180	7	15	181	149	191

Pedestrians

Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	
Median type	None
Median storage veh	
Upstream signal (ft)	
pX, platoon unblocked	
vC, conflicting volume	455 244 340
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	455 244 340
tC, single (s)	6.4 6.2 4.1
tC, 2 stage (s)	
tF (s)	3.5 3.3 2.2
p0 queue free %	68 99 99
cM capacity (veh/h)	557 795 1220

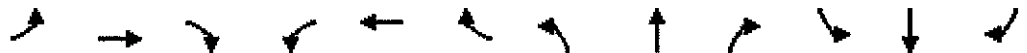
Direction Lane #	EB T	NB T	SB T
Volume Total	188	196	340
Volume Left	180	15	0
Volume Right	7	0	191
cSH	563	1220	1700
Volume to Capacity	0.33	0.01	0.20
Queue Length 95th (ft)	36	1	0
Control Delay (s)	14.6	0.7	0.0
Lane LOS	B	A	
Approach Delay (s)	14.6	0.7	0.0
Approach LOS	B		

Intersection Summary			
Average Delay		4.0	
Intersection Capacity Utilization		32.0%	ICU Level of Service A
Analysis Period (min)		15	

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

10: Route 940 & Carlton Road/Private Driveway



Movement	EB1	EB2	EB3	WB1	WB2	WB3	NB1	NB2	NB3	SB1	SB2	SB3
Lane Configurations		↕		↖	↗			↕			↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	11	10	11	11	11	11	11	8	8	8
Grade (%)		-4%			4%			1%				-1%
Total Lost time (s)		4.0		4.0	4.0			4.0			4.0	
Lane Util. Factor		1.00		1.00	1.00			1.00			1.00	
Fr _t		0.98		1.00	1.00			0.89			0.88	
Fl _t Protected		1.00		0.95	1.00			0.99			1.00	
Satd. Flow (prot)		1808		1619	1765			1580			1418	
Fl _t Permitted		1.00		0.28	1.00			0.93			0.93	
Satd. Flow (perm)		1801		485	1765			1489			1329	
Volume (vph)	4	459	59	233	469	0	51	1	232	1	0	8
Peak-hour factor, PHF	0.84	0.84	0.84	0.93	0.93	0.93	0.71	0.71	0.71	0.88	0.88	0.88
Adj. Flow (vph)	5	546	70	251	504	0	72	1	327	1	0	9
RTOR Reduction (vph)	0	8	0	0	0	0	0	265	0	0	7	0
Lane Group Flow (vph)	0	613	0	251	504	0	0	135	0	0	3	0
Turn Type		Perm		pm+pt			Perm			Perm		
Protected Phases		2		1	6		8			4		
Permitted Phases		2		6			8			4		
Actuated Green, G (s)		20.6		31.0	31.0		8.6			8.6		
Effective Green, g (s)		22.6		33.0	33.0		9.6			9.6		
Actuated g/C Ratio		0.45		0.65	0.65		0.19			0.19		
Clearance Time (s)		6.0		6.0	6.0		5.0			5.0		
Vehicle Extension (s)		6.0		3.0	6.0		3.0			3.0		
Lane Grp Cap (vph)		804		460	1151		282			252		
v/s Ratio Prot				0.07	0.29							
v/s Ratio Perm		0.34		0.29			0.09			0.00		
v/c Ratio		0.76		0.55	0.44		0.48			0.01		
Uniform Delay, d1		11.7		6.1	4.3		18.3			16.6		
Progression Factor		1.00		1.00	1.00		1.00			1.00		
Incremental Delay, d2		5.5		1.3	0.8		1.3			0.0		
Delay (s)		17.3		7.4	5.0		19.6			16.7		
Level of Service		B		A	A		B			B		
Approach Delay (s)		17.3			5.8		19.6			16.7		
Approach LOS		B			A		B			B		

Intersection Summary			
HCM Average Control Delay	12.9	HCM Level of Service	B
HCM Volume to Capacity ratio	0.66		
Actuated Cycle Length (s)	50.6	Sum of lost time (s)	12.0
Intersection Capacity Utilization	86.5%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

10: Route 940 & Carlton Road/Private Driveway



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		↕	↙	↘		↕		↕
Volume (vph)	4	459	233	469	51	1	1	0
Lane Group Flow (vph)	0	621	251	504	0	400	0	10
Turn Type	Perm		pm+pt		Perm		Perm	
Protected Phases		2	1	6		8		4
Permitted Phases	2		6		8		4	
Detector Phases	2	2	1	6	8	8	4	4
Minimum Initial (s)	10.0	10.0	4.0	10.0	6.0	6.0	6.0	6.0
Minimum Split (s)	16.0	16.0	10.0	16.0	11.0	11.0	11.0	11.0
Total Split (s)	33.0	33.0	12.0	45.0	15.0	15.0	15.0	15.0
Total Split (%)	55.0%	55.0%	20.0%	75.0%	25.0%	25.0%	25.0%	25.0%
Yellow Time (s)	4.5	4.5	4.0	4.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.5	1.5	2.0	1.5	1.5	1.5	1.5	1.5
Lead/Lag	Lag	Lag	Lead					
Lead-Lag Optimize?	Yes	Yes	Yes					
Recall Mode	Min	Min	None	Min	None	None	None	None
v/c Ratio		0.75	0.52	0.45		0.72		0.04
Control Delay		15.6	7.9	5.5		13.2		13.1
Queue Delay		0.0	0.0	0.0		0.0		0.0
Total Delay		15.6	7.9	5.5		13.2		13.1
Queue Length 50th (ft)		158	26	62		21		0
Queue Length 95th (ft)		238	49	108		45		11
Internal Link Dist (ft)		1322		1070		1366		73
Turn Bay Length (ft)			100					
Base Capacity (vph)		954	484	1241		591		311
Starvation Cap Reductn		0	0	0		0		0
Spillback Cap Reductn		0	0	0		0		0
Storage Cap Reductn		0	0	0		0		0
Reduced v/c Ratio		0.65	0.52	0.41		0.68		0.03

Intersection Summary

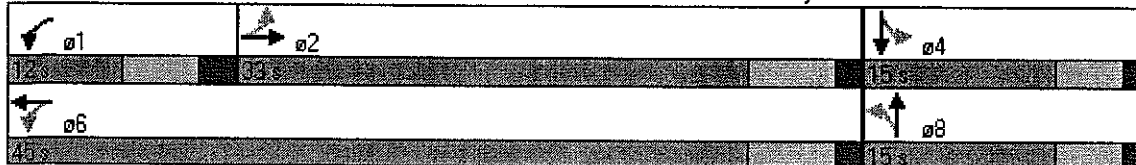
Cycle Length: 60

Actuated Cycle Length: 50

Natural Cycle: 55

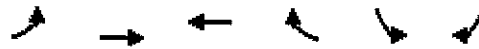
Control Type: Semi Act-Uncoord

Splits and Phases: 10: Route 940 & Carlton Road/Private Driveway



2017 Projected Conditions - With Site-Related Recommendations
 Saturday P.M. Peak Hour

11: Route 940 & Route 390



Movement	EBL	EBT	WBL	WBR	SBL	SBR
Lane Configurations		↕	↗		↘	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	11	11	11	11	14	14
Grade (%)		-2%	2%		4%	
Total Lost time (s)		4.0	4.0		4.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	0.99		0.88	
Flt Protected		0.97	1.00		0.99	
Satd. Flow (prot)		1755	1769		1773	
Flt Permitted		0.46	1.00		0.99	
Satd. Flow (perm)		820	1769		1773	
Volume (vph)	360	332	318	19	45	384
Peak-hour factor, PHF	0.79	0.79	0.84	0.84	0.81	0.81
Adj. Flow (vph)	456	420	379	23	56	474
RTOR Reduction (vph)	0	0	3	0	381	0
Lane Group Flow (vph)	0	876	399	0	149	0
Heavy Vehicles (%)	3%	3%	2%	2%	2%	2%
Turn Type	pm+pt					
Protected Phases	5	2	6		4	
Permitted Phases	2					
Actuated Green, G (s)		61.0	36.0		7.0	
Effective Green, g (s)		63.0	38.0		9.0	
Actuated g/C Ratio		0.79	0.48		0.11	
Clearance Time (s)		6.0	6.0		6.0	
Vehicle Extension (s)		6.0	6.0		3.0	
Lane Grp Cap (vph)		891	840		199	
v/s Ratio Prot		c0.26	0.23		c0.08	
v/s Ratio Perm		c0.52				
v/c Ratio		0.98	0.48		0.75	
Uniform Delay, d1		8.0	14.2		34.4	
Progression Factor		1.00	1.00		1.00	
Incremental Delay, d2		26.2	1.2		14.7	
Delay (s)		34.2	15.4		49.1	
Level of Service		C	B		D	
Approach Delay (s)		34.2	15.4		49.1	
Approach LOS		C	B		D	

Intersection Summary			
HCM Average Control Delay	34.4	HCM Level of Service	C
HCM Volume to Capacity ratio	0.94		
Actuated Cycle Length (s)	80.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	91.5%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

2017 Projected Conditions - With Site-Related Recommendations
 Saturday P.M. Peak Hour

11: Route 940 & Route 390



Lane Group	EBL	EBT	WBT	SBL
Lane Configurations		↕	↕	↕
Volume (vph)	360	332	318	45
Lane Group Flow (vph)	0	876	402	530
Turn Type	pm+pt			
Protected Phases	5	2	6	4
Permitted Phases	2			
Detector Phases	5	2 5	6	4
Minimum Initial (s)	4.0	10.0	10.0	7.0
Minimum Split (s)	10.0	16.0	16.0	13.0
Total Split (s)	25.0	67.0	42.0	13.0
Total Split (%)	31.3%	83.8%	52.5%	16.3%
Yellow Time (s)	4.5	4.5	4.5	4.5
All-Red Time (s)	1.5	1.5	1.5	1.5
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	Min	Min	None
v/c Ratio		0.98	0.48	0.91
Control Delay		35.4	16.4	30.9
Queue Delay		0.0	0.0	0.0
Total Delay		35.4	16.4	30.9
Queue Length 50th (ft)		167	128	48
Queue Length 95th (ft)		#206	184	#154
Internal Link Dist (ft)		491	1298	1509
Turn Bay Length (ft)				
Base Capacity (vph)		891	842	580
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.98	0.48	0.91

Intersection Summary

Cycle Length: 80

Actuated Cycle Length: 80

Natural Cycle: 80

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 11: Route 940 & Route 390

02	04
67 s	13 s
05	06
25 s	42 s

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

12: Route 940 & Route 191/Red Rock Road



Lane Group	EB1	EB2	EBR	WB1	WB2	WBR	NB1	NB2	NBR	SB1	SB2	SEB
Lane Configurations	↕			↕			↕			↕		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	8	8	8	11	11	11
Grade (%)		-2%			2%			-3%			-1%	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998			0.965			0.965			0.969	
Frt Protected		0.994			0.999			0.984			0.967	
Satd. Flow (prot)	0	1787	0	0	1719	0	0	1556	0	0	1696	0
Frt Permitted		0.994			0.999			0.984			0.967	
Satd. Flow (perm)	0	1787	0	0	1719	0	0	1556	0	0	1696	0
Headway Factor	1.03	1.03	1.03	1.06	1.06	1.06	1.18	1.18	1.18	1.04	1.04	1.04
Link Speed (mph)		45			45			35			45	
Link Distance (ft)		1662			865			282			1220	
Travel Time (s)		25.2			13.1			5.5			18.5	
Volume (vph)	45	327	5	5	298	106	4	5	3	103	14	35
Peak Hour Factor	0.89	0.89	0.89	0.81	0.81	0.81	0.45	0.45	0.45	0.88	0.88	0.88
Heavy Vehicles (%)	3%	3%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adj. Flow (vph)	51	367	6	6	368	131	9	11	7	117	16	40
Lane Group Flow (vph)	0	424	0	0	505	0	0	27	0	0	173	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 67.7%

ICU Level of Service C

Analysis Period (min) 15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

12: Route 940 & Route 191/Red Rock Road



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↕			↕		
Sign Control	Free			Free			Stop			Stop		
Grade	-2%			2%			-3%			-1%		
Volume (veh/h)	45	327	5	5	298	106	4	5	3	103	14	35
Peak Hour Factor	0.89	0.89	0.89	0.81	0.81	0.81	0.45	0.45	0.45	0.88	0.88	0.88
Hourly flow rate (vph)	51	367	6	6	368	131	9	11	7	117	16	40
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type							None			None		
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	499			373			965	982	370	929	920	433
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	499			373			965	982	370	929	920	433
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	95			99			96	95	99	48	94	94
cM capacity (veh/h)	1060			1185			201	236	676	227	257	623

Direction Lane #	EB 1	WB 1	NB 1	SB 1
Volume Total	424	505	27	173
Volume Left	51	6	9	117
Volume Right	6	131	7	40
cSH	1060	1185	264	270
Volume to Capacity	0.05	0.01	0.10	0.64
Queue Length 95th (ft)	4	0	8	100
Control Delay (s)	1.5	0.2	20.2	39.4
Lane LOS	A	A	C	E
Approach Delay (s)	1.5	0.2	20.2	39.4
Approach LOS			C	E

Intersection Summary			
Average Delay			7.1
Intersection Capacity Utilization	67.7%	ICU Level of Service	C
Analysis Period (min)			15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

13: Woodland Road & Western Site Driveway



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	14	12	14
Grade (%)	-3%			2%	0%	
Storage Length (ft)		250	100		0	0
Storage Lanes		1	1		1	1
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Fr _t		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1891	1714	1752	1967	1770	1689
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1891	1714	1752	1967	1770	1689
Headway Factor	0.98	0.90	1.01	0.93	1.00	0.92
Link Speed (mph)	40			40	20	
Link Distance (ft)	863			1227	233	
Travel Time (s)	14.7			20.9	7.9	
Volume (vph)	224	408	26	301	216	35
Peak Hour Factor	0.85	0.90	0.90	0.84	0.90	0.90
Adj. Flow (vph)	264	453	29	358	240	39
Lane Group Flow (vph)	264	453	29	358	240	39
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type	Other
Control Type	Unsignalized
Intersection Capacity Utilization	37.1%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

13: Woodland Road & Western Site Driveway



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Sign Control	Free			Free	Stop	
Grade	-3%			2%	0%	
Volume (veh/h)	224	408	26	301	216	35
Peak Hour Factor	0.85	0.90	0.90	0.84	0.90	0.90
Hourly flow rate (vph)	264	453	29	358	240	39
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None		
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			264		680	264
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			264		680	264
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
IF (s)			2.2		3.5	3.3
p0 queue free %			98		41	95
cM capacity (veh/h)			1301		408	775
Direction Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volume Total	264	453	29	358	240	39
Volume Left	0	0	29	0	240	0
Volume Right	0	453	0	0	0	39
cSH	1700	1700	1301	1700	408	775
Volume to Capacity	0.16	0.27	0.02	0.21	0.59	0.05
Queue Length 95th (ft)	0	0	2	0	91	4
Control Delay (s)	0.0	0.0	7.8	0.0	25.7	9.9
Lane LOS			A		D	A
Approach Delay (s)	0.0		0.6		23.5	
Approach LOS					C	
Intersection Summary						
Average Delay			4.9			
Intersection Capacity Utilization			37.1%		ICU Level of Service	A
Analysis Period (min)			15			

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

14: Woodland Road & Eastern Site Driveway



Lane Group	EET	EBP	WBI	WBT	NBI	NBR
Lane Configurations	↑	↗	↖	↑	↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	14	12	14
Grade (%)	-3%			2%	0%	
Storage Length (ft)		250	100		0	0
Storage Lanes		1	1		1	1
Turning Speed (mph)		9	15		15	9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Fr _t		0.850				0.850
Fr _t Protected			0.950		0.950	
Satd. Flow (prot)	1891	1714	1752	1967	1770	1689
Fr _t Permitted			0.950		0.950	
Satd. Flow (perm)	1891	1714	1752	1967	1770	1689
Headway Factor	0.98	0.90	1.01	0.93	1.00	0.92
Link Speed (mph)	40			40	20	
Link Distance (ft)	1227			2146	250	
Travel Time (s)	20.9			36.6	8.5	
Volume (vph)	119	140	64	111	216	35
Peak Hour Factor	0.85	0.90	0.90	0.84	0.90	0.90
Adj. Flow (vph)	140	156	71	132	240	39
Lane Group Flow (vph)	140	156	71	132	240	39
Sign Control	Free			Free	Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	28.8%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Site-Related Recommendations

Saturday P.M. Peak Hour

14: Woodland Road & Eastern Site Driveway



Movement	EB1	EB2	WB1	WB2	NB1	NB2
Lane Configurations	↑	↗	↖	↑	↖	↗
Sign Control	Free		Free		Stop	
Grade	-3%		2%		0%	
Volume (veh/h)	119	140	64	111	216	35
Peak Hour Factor	0.85	0.90	0.90	0.84	0.90	0.90
Hourly flow rate (vph)	140	156	71	132	240	39
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			140		414	140
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			140		414	140
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
IF (s)			2.2		3.5	3.3
p0 queue free %			95		58	96
cM capacity (veh/h)			1443		565	908

Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volume Total	140	156	71	132	240	39
Volume Left	0	0	71	0	240	0
Volume Right	0	156	0	0	0	39
cSH	1700	1700	1443	1700	565	908
Volume to Capacity	0.08	0.09	0.05	0.08	0.42	0.04
Queue Length 95th (ft)	0	0	4	0	53	3
Control Delay (s)	0.0	0.0	7.6	0.0	16.0	9.1
Lane LOS			A		C	A
Approach Delay (s)	0.0		2.7		15.0	
Approach LOS					C	

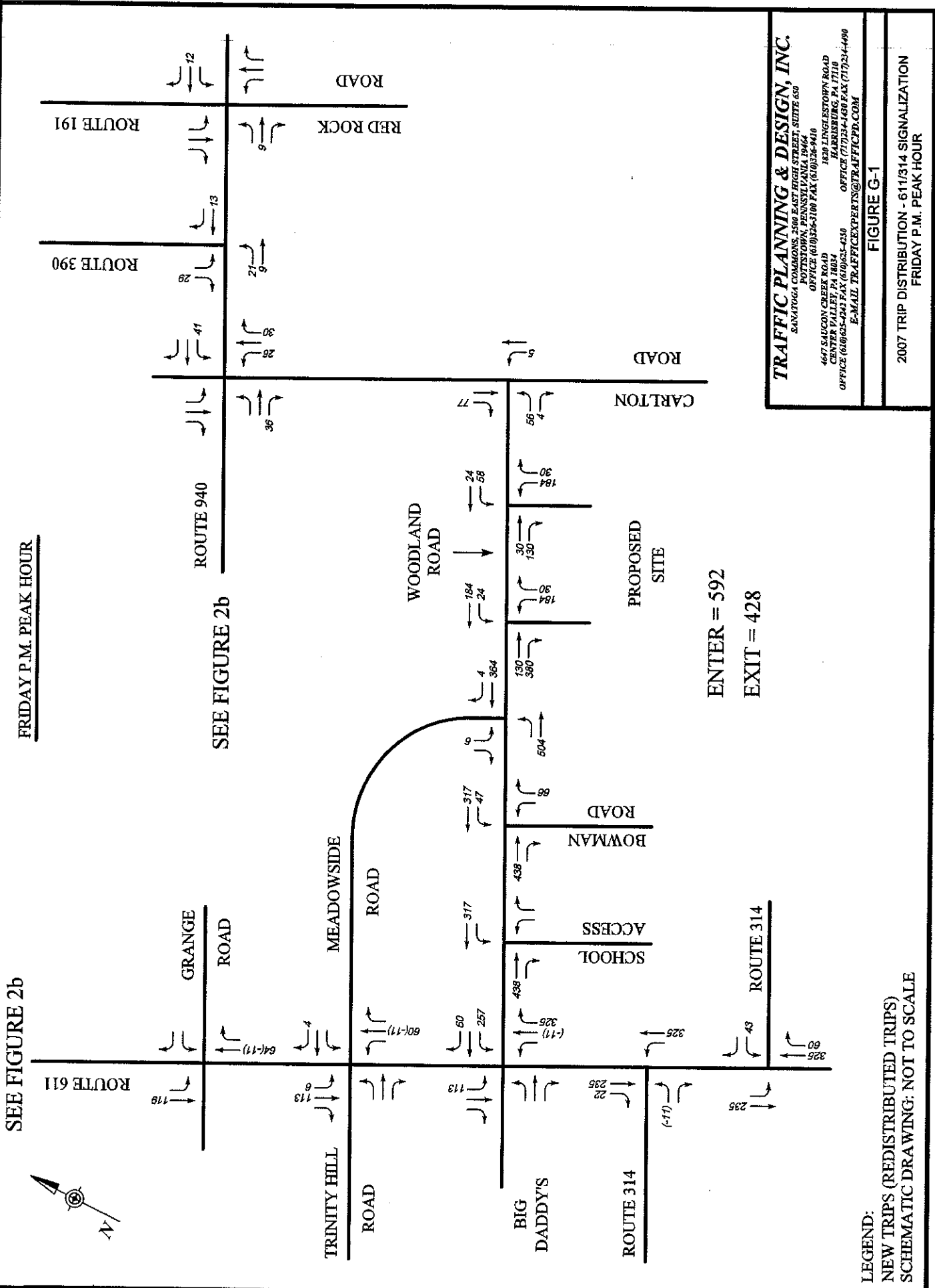
Intersection Summary		
Average Delay	6.1	
Intersection Capacity Utilization	28.8%	ICU Level of Service: A
Analysis Period (min)	15	

APPENDIX G
ROUTE 611/ROUTE 314 (WESTERN LEG)
SIGNALIZATION INFO

SEE FIGURE 2b

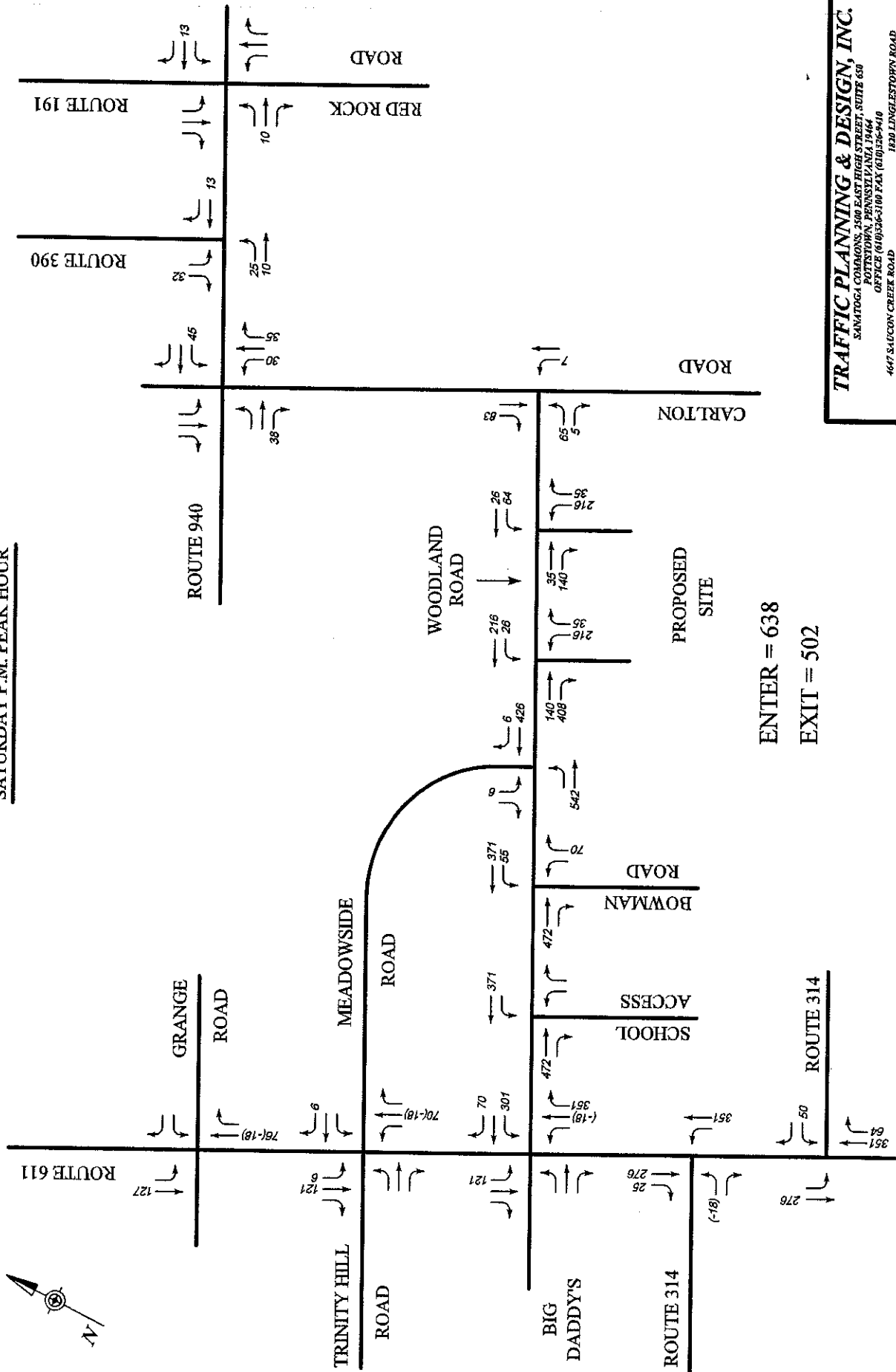
FRIDAY P.M. PEAK HOUR

SEE FIGURE 2b



SEE FIGURE 3b

SATURDAY P.M. PEAK HOUR



ENTER = 638
EXIT = 502

TRAFFIC PLANNING & DESIGN, INC.
 SANATOGA COMMONS, 2500 EAST HIGH STREET, SUITE 650
 POTTSTOWN, PENNSYLVANIA 19464
 OFFICE (610)356-3100 FAX (610)356-9410

1447 SAUCON CREEK ROAD
 CENTER VALLEY, PA 16824
 OFFICE (717)254-4200 FAX (717)254-4100
 E-MAIL TRAFFICEXPERTS@TRAFFICPD.COM

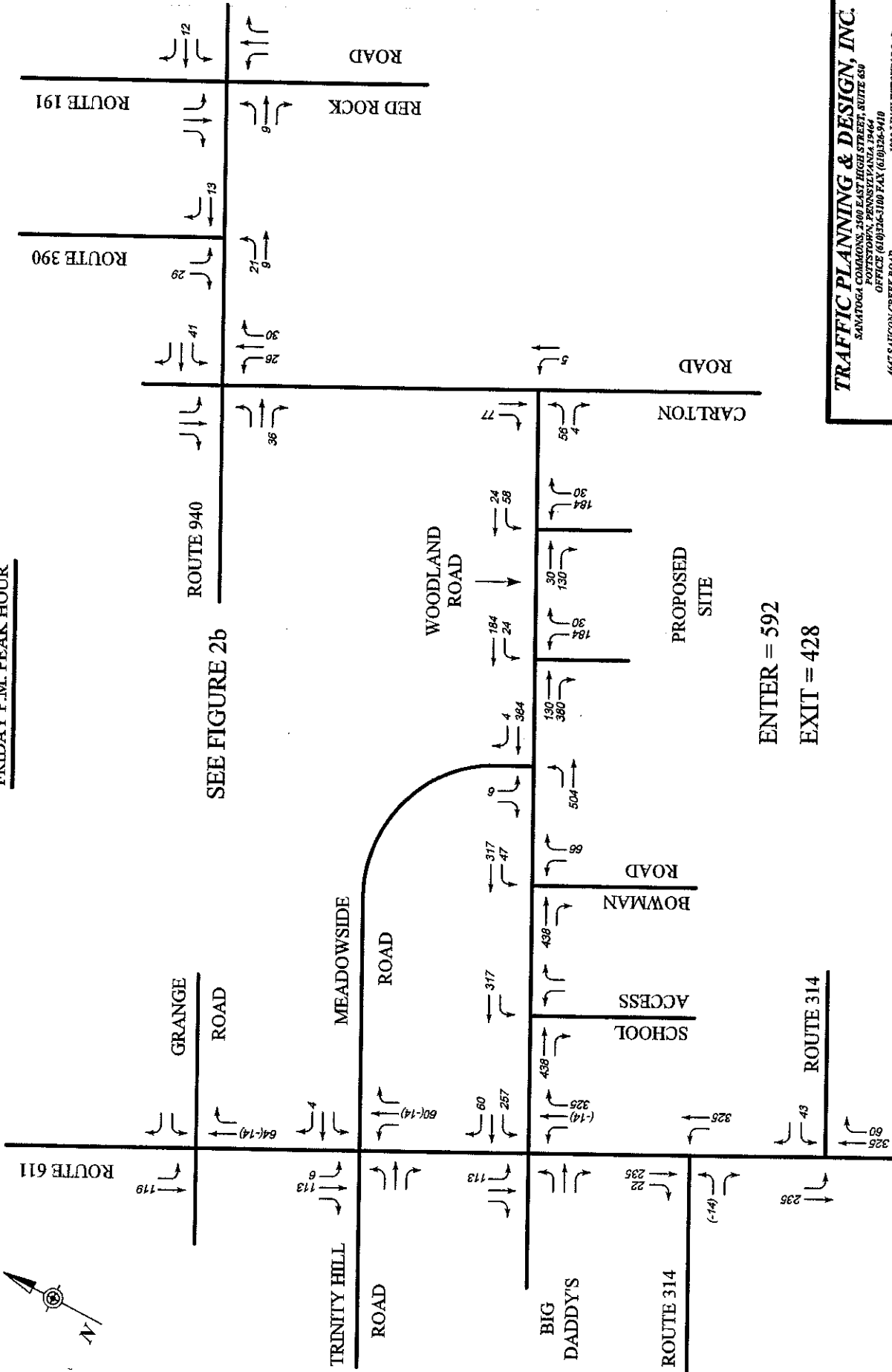
FIGURE G-2

2007 TRIP DISTRIBUTION - 611/314 SIGNALIZATION
 SATURDAY P.M. PEAK HOUR

LEGEND:
 NEW TRIPS (REDISTRIBUTED TRIPS)
 SCHEMATIC DRAWING: NOT TO SCALE

FRIDAY P.M. PEAK HOUR

SEE FIGURE 2b



SEE FIGURE 2b

ENTER = 592
EXIT = 428

LEGEND:
NEW TRIPS (REDISTRIBUTED TRIPS)
SCHEMATIC DRAWING: NOT TO SCALE

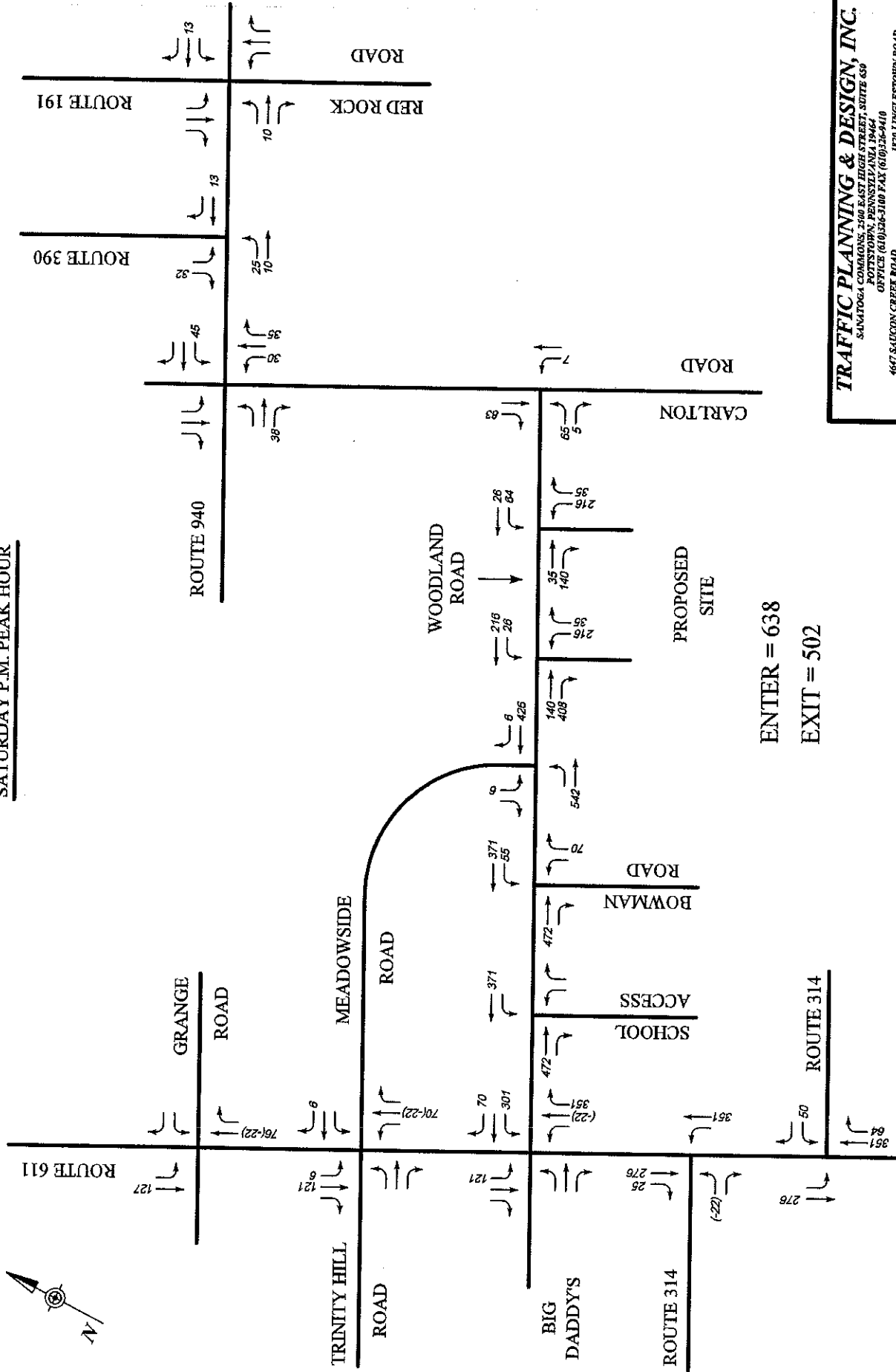
TRAFFIC PLANNING & DESIGN, INC.
 SANATOGA COMMONS, 3400 EAST HIGGINS ROAD, SUITE 850
 FORT STODOLN, PENNSYLVANIA 15464
 OFFICE (610)336-3100 FAX (610)336-9410
 1400 LINGENSTOWN ROAD
 1400 LINGENSTOWN ROAD
 HARRISBURG, PA 17110
 OFFICE (610)65-4242 FAX (610)65-4250 OFFICE (717)234-1480 FAX (717)334-4400
 E-MAIL: TRAFFICPEKINS@TRAFFICPD.COM

FIGURE G-3

2017 TRIP DISTRIBUTION - 611/314 SIGNALIZATION
 FRIDAY P.M. PEAK HOUR

SATURDAY P.M. PEAK HOUR

SEE FIGURE 3b



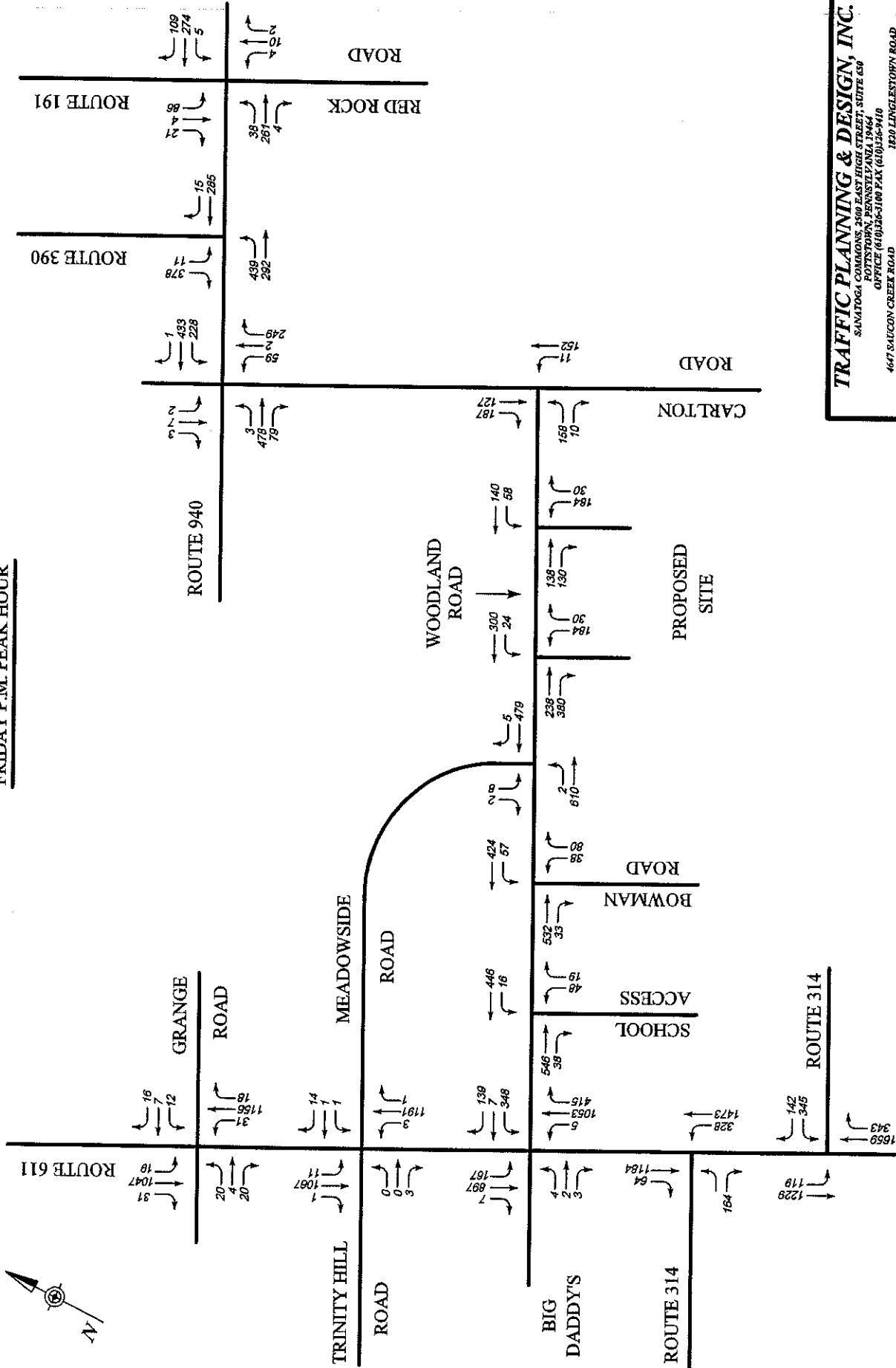
LEGEND:
 NEW TRIPS (REDISTRIBUTED TRIPS)
 SCHEMATIC DRAWING: NOT TO SCALE

TRAFFIC PLANNING & DESIGN, INC.
 SANATOGA COMMONS, 2500 EAST HIGH STREET, SUITE 630
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 OFFICE (610)326-3100 FAX (610)326-9410
 467 SAUCON CREEK ROAD
 CENTER VALLEY, PA 16834
 OFFICE (717)334-1430 FAX (717)334-4990
 E-MAIL TRAFFICEXPERTS@TRAFFICPD.COM

2017 TRIP DISTRIBUTION - 611/314 SIGNALIZATION
 SATURDAY P.M. PEAK HOUR

FIGURE G-4

FRIDAY P.M. PEAK HOUR



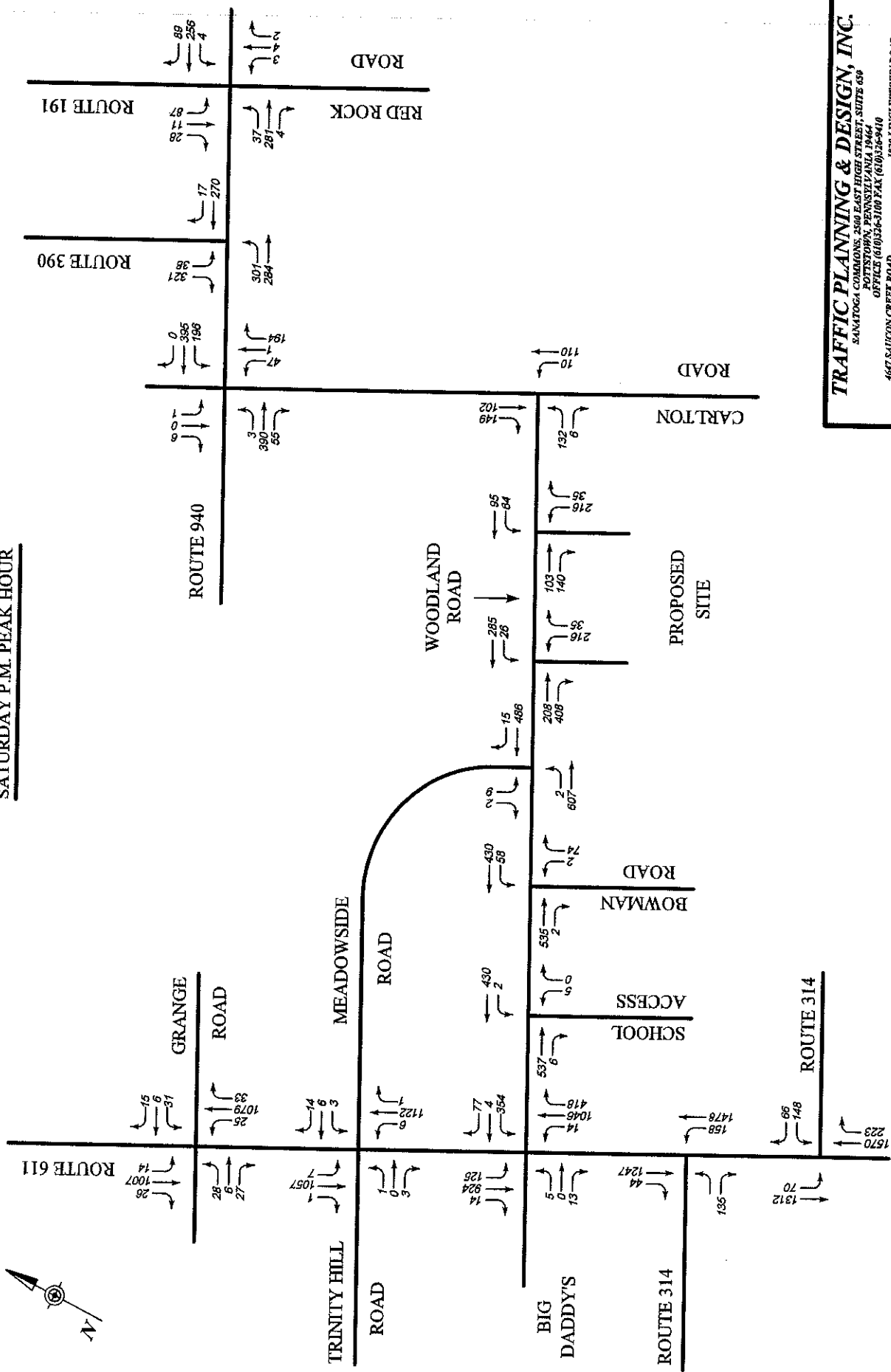
TRAFFIC PLANNING & DESIGN, INC.
 SANATOGA COMMONS 3500 EAST HIGHWAY STREET, SUITE 209
 POTTSTOWN, PENNSYLVANIA 19644
 OFFICE (610)326-3100 FAX (610)326-9410
 467 SAUCON CREEK ROAD
 CENTER VALLEY, PA 16834
 OFFICE (717)234-1459 FAX (717)234-1490
 E-MAIL TRAFFICEXPERTS@TRAFFICPD.COM

FIGURE G-5

2007 PROJECTED CONDITIONS - 611/314 SIGNALIZATION
 FRIDAY P.M. PEAK HOUR
 TRAFFIC VOLUMES

SCHEMATIC DRAWING: NOT TO SCALE

SATURDAY P.M. PEAK HOUR



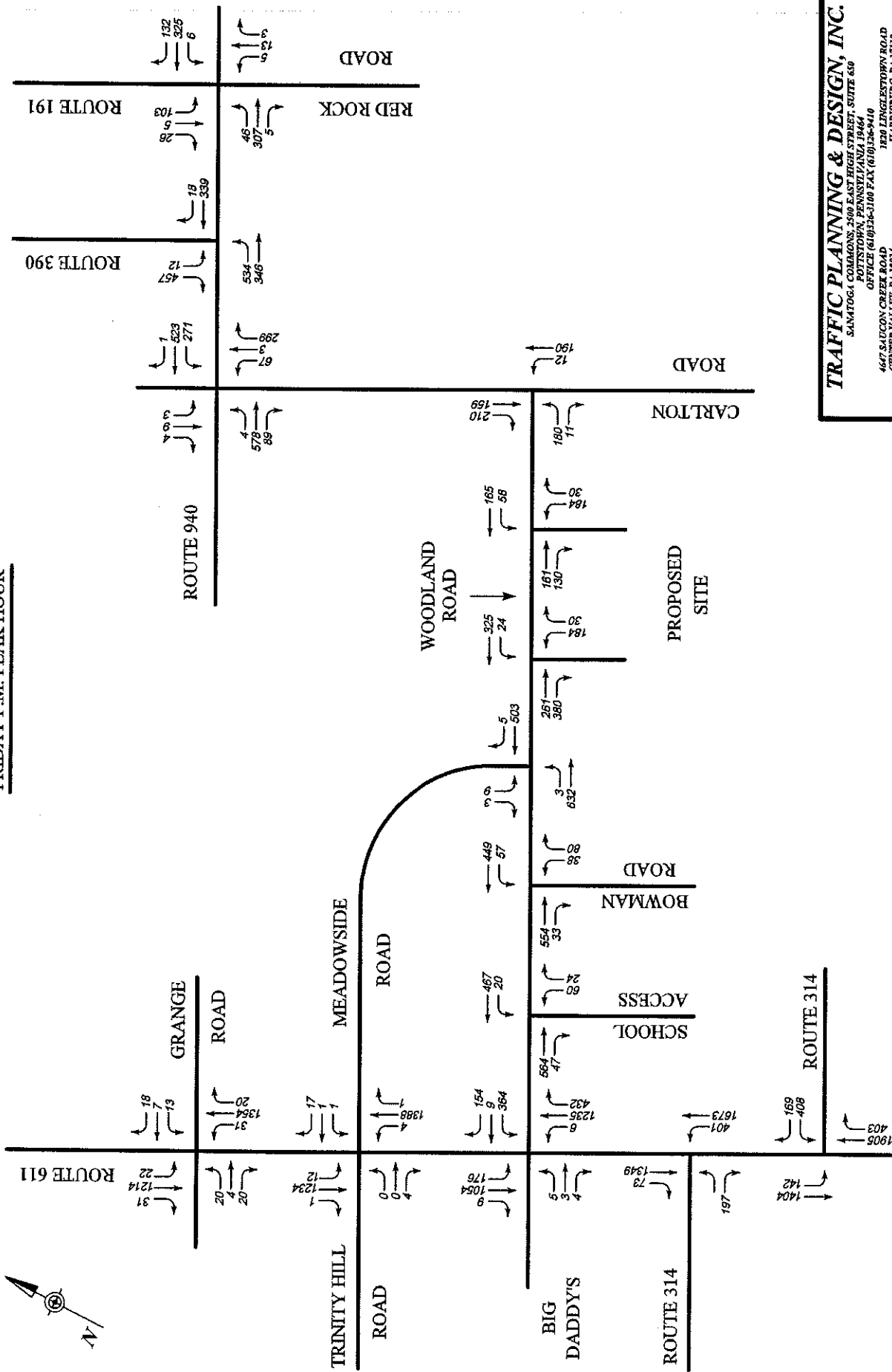
TRAFFIC PLANNING & DESIGN, INC.
 SANATOGA COMMONS, 286 EAST HIGH STREET, SUITE 608
 POTTSTOWN, PENNSYLVANIA 19464
 OFFICE (610)266-1100 FAX (610)266-9410
 467 SAUCON CREEK ROAD
 CENTER VALLEY, PA 18834
 OFFICE (717)234-4350 OFFICE (717)234-4350 FAX (717)234-4400
 E-MAIL TRAFFICPERTS@TRAFFICPD.COM

FIGURE G-6

2007 PROJECTED CONDITIONS - 611/314 SIGNALIZATION
 SATURDAY P.M. PEAK HOUR
 TRAFFIC VOLUMES

SCHEMATIC DRAWING: NOT TO SCALE

FRIDAY P.M. PEAK HOUR

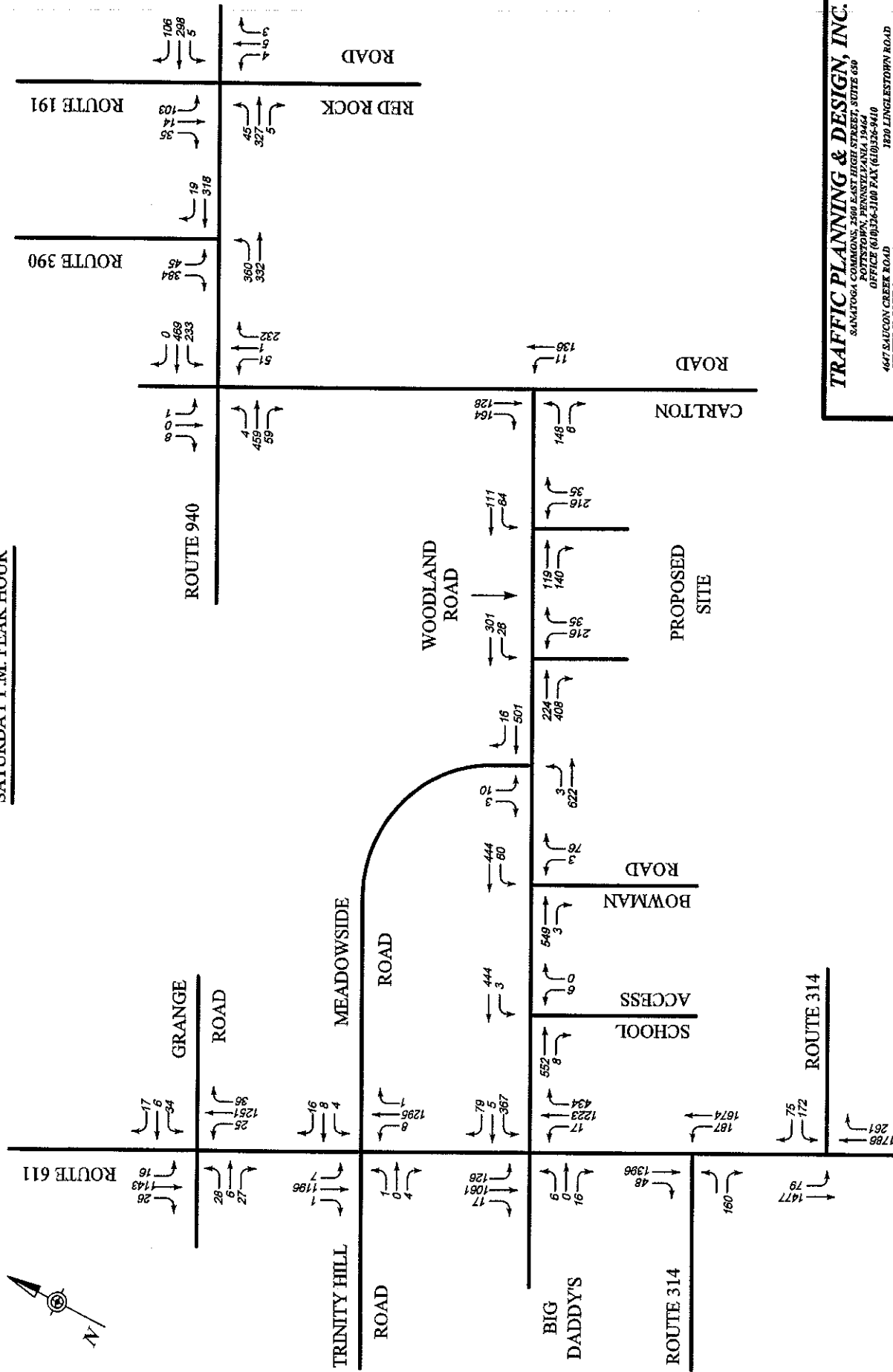


TRAFFIC PLANNING & DESIGN, INC.
 SAMATOGA COLLABORATIONS, 2500 EAST HIGH STREET, SUITE 650
 POTTSVILLE, PENNSYLVANIA 19464
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 4647 SAUCON CREEK ROAD
 CENTER VALLEY, PA 17014
 OFFICE (717)254-4480 FAX (717)254-4490
 E-MAIL TRAFFICEXPERTS@TRAFFICPD.COM

FIGURE G-7
 2017 PROJECTED CONDITIONS - 611/314 SIGNALIZATION
 FRIDAY P.M. PEAK HOUR
 TRAFFIC VOLUMES

SCHEMATIC DRAWING; NOT TO SCALE

SATURDAY P.M. PEAK HOUR



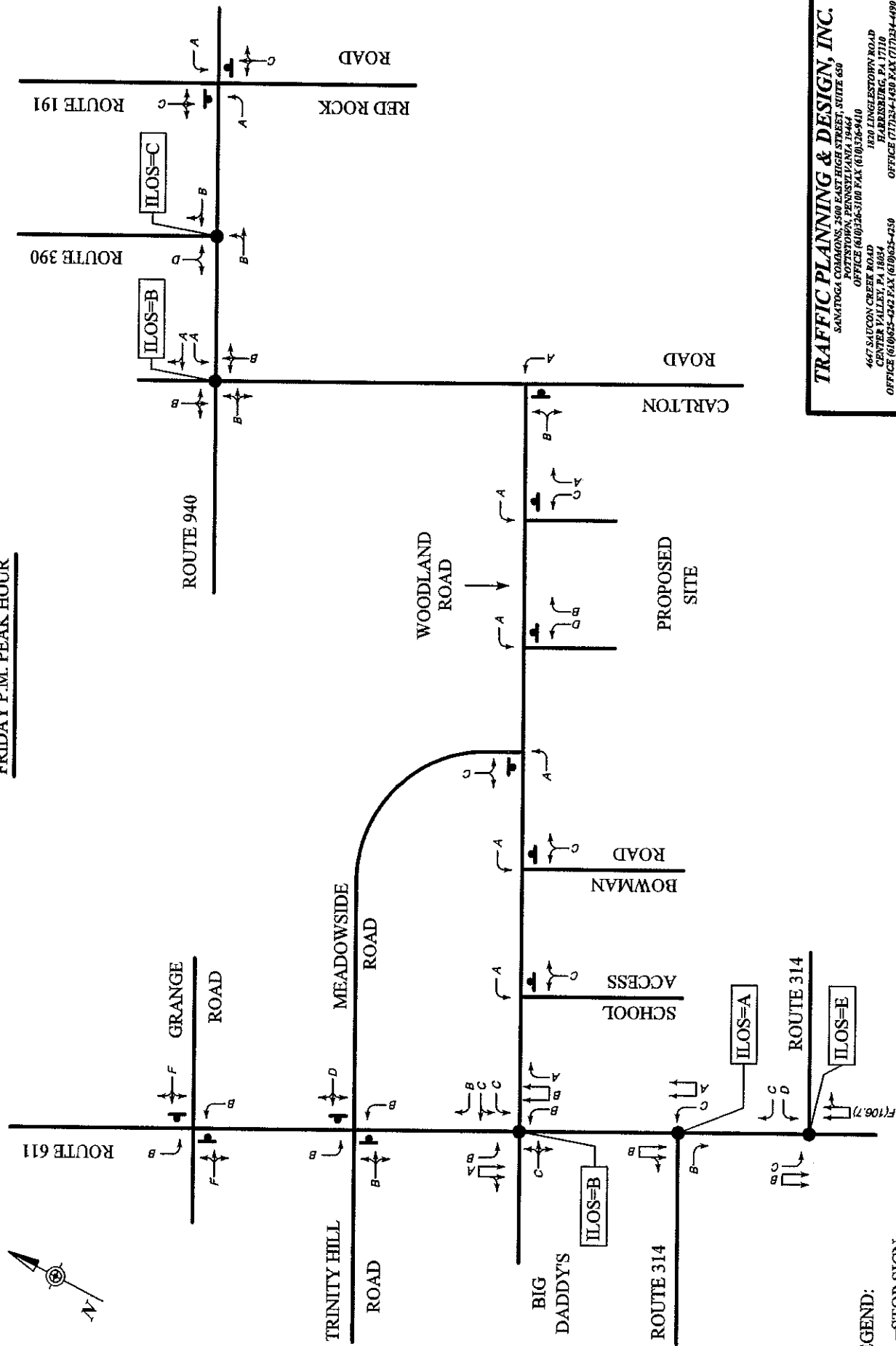
TRAFFIC PLANNING & DESIGN, INC.
 804 TOWSON COMMONS, 280 E. EAST HIGH STREET, SUITE 600
 POTTSTOWN, PENNSYLVANIA 19464
 OFFICE (610) 326-3100 FAX (610) 326-9410
 4647 SAUCON CREEK ROAD
 CENTER VALLEY, PA 18034
 OFFICE (717) 234-1450 FAX (717) 234-4400
 E-MAIL TRAFFICEXPERTS@TRAFFICPD.COM

FIGURE G-8

2017 PROJECTED CONDITIONS - 611/314 SIGNALIZATION
 SATURDAY P.M. PEAK HOUR
 TRAFFIC VOLUMES

SCHEMATIC DRAWING: NOT TO SCALE

FRIDAY P.M. PEAK HOUR



LEGEND:

◻ = STOP SIGN

● = SIGNALIZED INTERSECTION

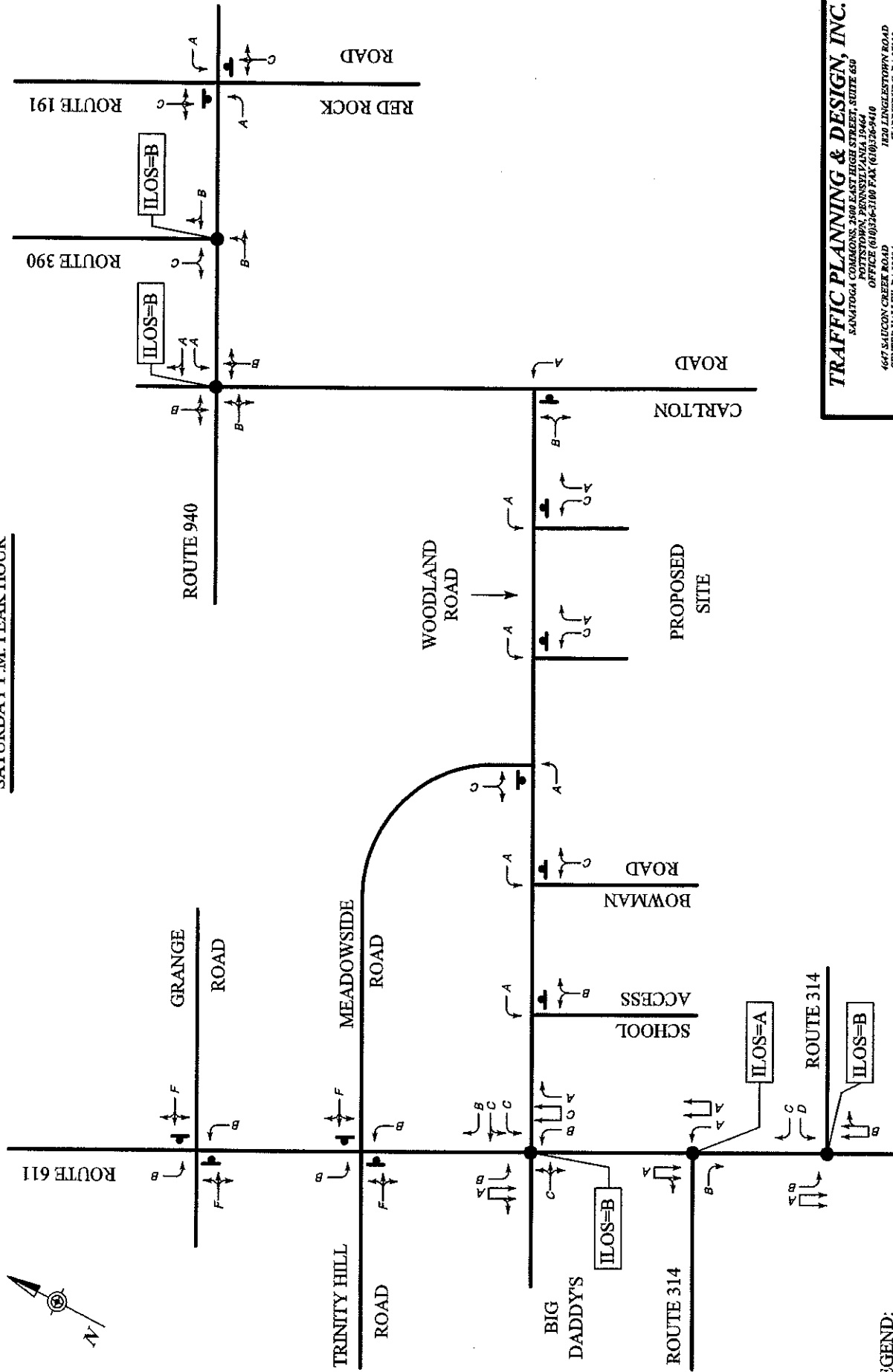
SCHEMATIC DRAWING: NOT TO SCALE

TRAFFIC PLANNING & DESIGN, INC.
 SANATOGA COLLABORATIONS, 2500 EAST HIGH STREET, SUITE 650
 POTTSTOWN, PENNSYLVANIA 19464
 OFFICE (610)326-3100 FAX (610)326-9410
 4647 SAUCON CREEK ROAD
 CENTER VALLEY, PA 16834
 OFFICE (717)234-1450 FAX (717)234-4490
 E-MAIL TRAFFICPEP@TRAFFICPD.COM

FIGURE G-9

2007 PROJECTED CONDITIONS - 611/314 SIGNALIZATION
 FRIDAY P.M. PEAK HOUR
 LEVELS OF SERVICE

SATURDAY P.M. PEAK HOUR



LEGEND:

● =STOP SIGN

● =SIGNALIZED INTERSECTION

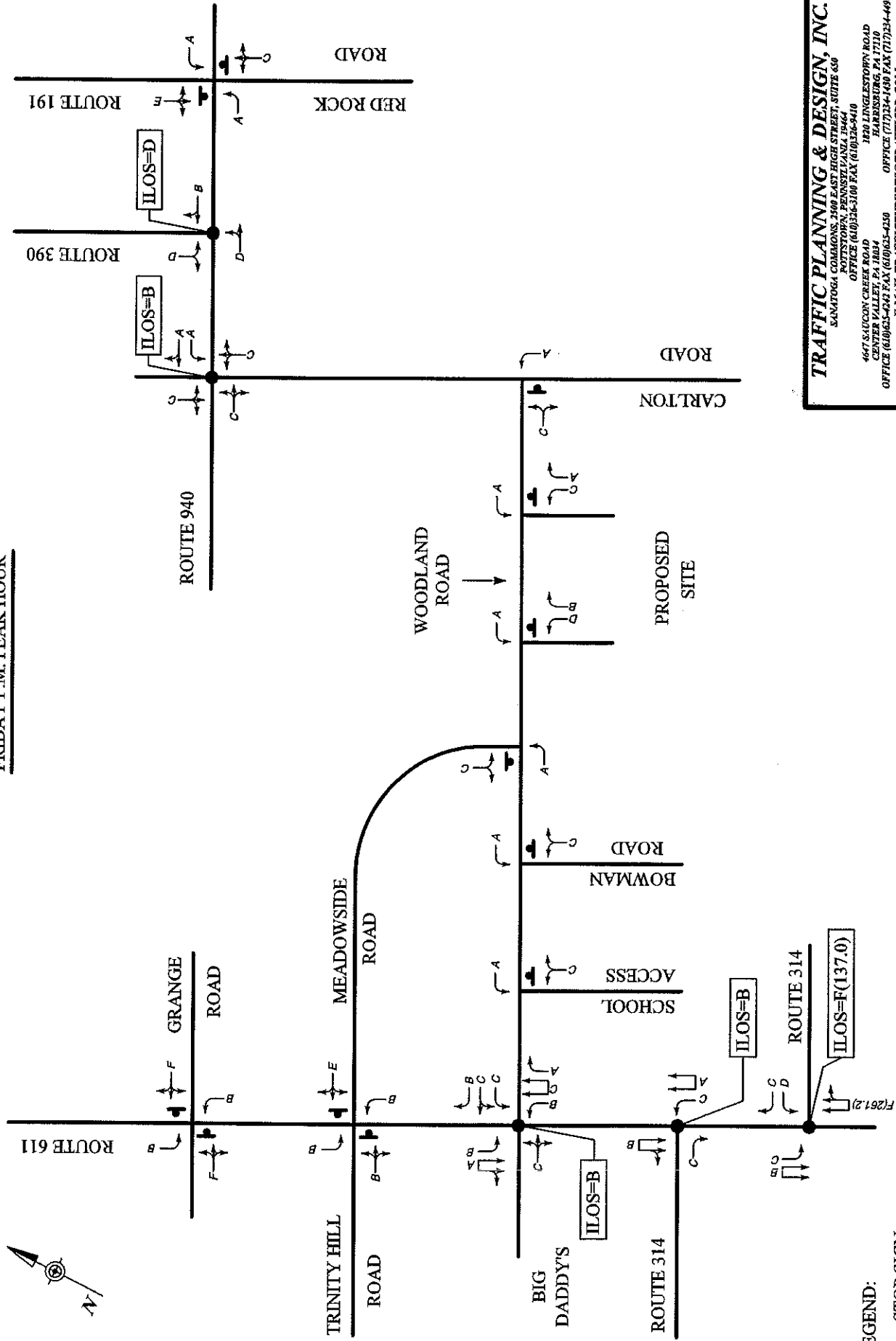
SCHEMATIC DRAWING: NOT TO SCALE

TRAFFIC PLANNING & DESIGN, INC.
 SANITOGA COMMONS, 280 EAST HIGH STREET, SUITE 630
 POTTSTOWN, PENNSYLVANIA 19464
 OFFICE (610)226-1100 FAX (610)226-9410
 467 SAUCON CREEK ROAD
 CENTER VALLEY, PA 18034
 OFFICE (717)234-1250
 120 LINGLESTOWN ROAD
 HARRISBURG, PA 17110
 OFFICE (717)234-1430 FAX (717)234-4400
 E-MAIL TRAFFICEXPERTS@TRAFFICPD.COM

FIGURE G-10

2007 PROJECTED CONDITIONS - 611/314 SIGNALIZATION
 SATURDAY P.M. PEAK HOUR
 LEVELS OF SERVICE

FRIDAY P.M. PEAK HOUR



LEGEND:

● = STOP SIGN

● = SIGNALIZED INTERSECTION

SCHEMATIC DRAWING: NOT TO SCALE

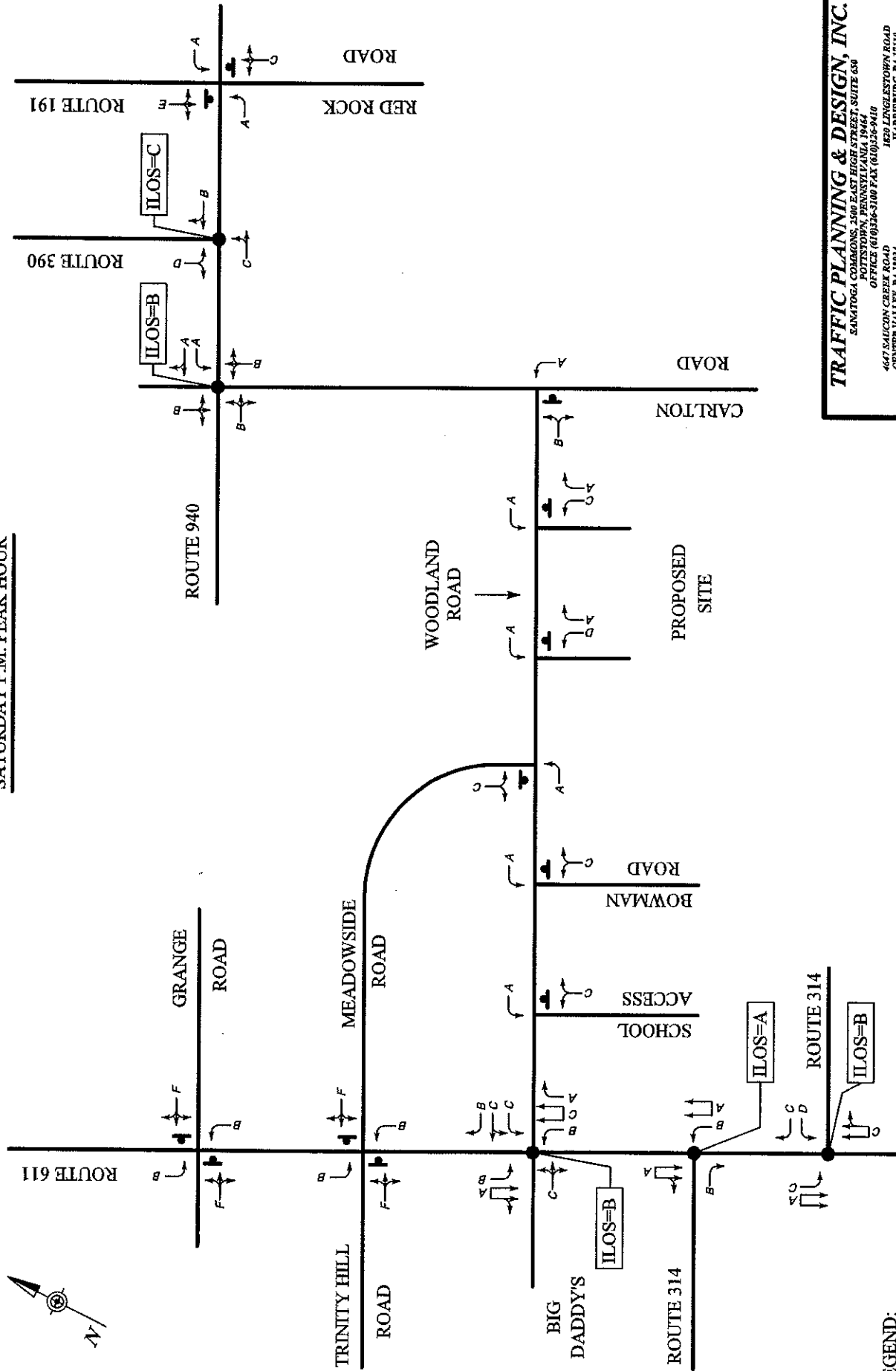
FIGURE G-11

TRAFFIC PLANNING & DESIGN, INC.
 SANATOGA COMMONS, 2500 EAST HIGH STREET, SUITE 600
 POTTSTOWN, PENNSYLVANIA 19464
 OFFICE (610)326-3100 FAX (610)326-9410

1820 LINGLESTOWN ROAD
 HARRISBURG, PA 17110
 OFFICE (717)234-1490 FAX (717)234-4490
 E-MAIL: TRAFFICEXPERTS@TRAFFICPD.COM

2017 PROJECTED CONDITIONS - 611/314 SIGNALIZATION
 FRIDAY P.M. PEAK HOUR
 LEVELS OF SERVICE

SATURDAY P.M. PEAK HOUR



LEGEND:

● = STOP SIGN

● = SIGNALIZED INTERSECTION

SCHEMATIC DRAWING: NOT TO SCALE

TRAFFIC PLANNING & DESIGN, INC.
 SAMMOTGA COMMONS, 2500 EAST HIGH STREET, SUITE 630
 POTTSVILLE, PENNSYLVANIA, 19467
 OFFICE (610) 266-3100 FAX (610) 266-9410
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 129 LINGLESTOWN ROAD
 HARRISBURG, PA 17110
 OFFICE (717) 254-4000 FAX (717) 254-4490
 E-MAIL: TRAFFICEXPERTS@TRAFFICPD.COM

FIGURE G-12

2017 PROJECTED CONDITIONS - 611/314 SIGNALIZATION
 SATURDAY P.M. PEAK HOUR
 LEVELS OF SERVICE

CAPACITY ANALYSIS WORKSHEETS

2007 PROJECTED CONDITIONS

2007 Projected Conditions - With Route 611/Route 314 Signalization
 Friday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗	↖	↑↑	↑↑	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	10	10	12	12	12	12
Grade (%)	4%			7%	-6%	
Total Lost time (s)		4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00	1.00	0.95	0.95	
Fr _t		0.86	1.00	1.00	0.99	
Flt Protected		1.00	0.95	1.00	1.00	
Satd. Flow (prot)		1445	1708	3415	3582	
Flt Permitted		1.00	0.95	1.00	1.00	
Satd. Flow (perm)		1445	1708	3415	3582	
Volume (vph)	0	164	328	1473	1184	64
Peak-hour factor, PHF	0.67	0.67	0.95	0.95	0.76	0.76
Adj. Flow (vph)	0	245	345	1551	1558	84
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	0	245	345	1551	1642	0
Heavy Vehicles (%)	4%	4%	2%	2%	3%	3%
Turn Type		Over	Prot			
Protected Phases		5	5	2	6	
Permitted Phases						
Actuated Green, G (s)		14.0	14.0	55.8	29.8	
Effective Green, g (s)		16.0	16.0	55.8	31.8	
Actuated g/C Ratio		0.29	0.29	1.00	0.57	
Clearance Time (s)		6.0	6.0	6.0	6.0	
Vehicle Extension (s)		3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)		414	490	3415	2041	
v/s Ratio Prot		0.17	c0.20	0.45	c0.46	
v/s Ratio Perm						
v/c Ratio		0.59	0.70	0.45	0.80	
Uniform Delay, d1		17.1	17.8	0.0	9.5	
Progression Factor		1.00	1.00	1.00	1.00	
Incremental Delay, d2		2.3	4.6	0.1	2.4	
Delay (s)		19.4	22.3	0.1	11.9	
Level of Service		B	C	A	B	
Approach Delay (s)	19.4			4.1	11.9	
Approach LOS	B			A	B	
Intersection Summary						
HCM Average Control Delay			8.5		HCM Level of Service	A
HCM Volume to Capacity ratio			0.77			
Actuated Cycle Length (s)			55.8		Sum of lost time (s)	8.0
Intersection Capacity Utilization			59.6%		ICU Level of Service	B
Analysis Period (min)			15			

c Critical Lane Group

2007 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Lane Group	EBR	NBL	NBT	SBT
Lane Configurations	↗	↖	↑↑	↑↑
Volume (vph)	164	328	1473	1184
Lane Group Flow (vph)	245	345	1551	1642
Turn Type	Over	Prot		
Protected Phases	5	5	2	6
Permitted Phases				
Detector Phases	5	5	2	6
Minimum Initial (s)	4.0	4.0	4.0	4.0
Minimum Split (s)	10.0	10.0	22.0	22.0
Total Split (s)	22.0	22.0	60.0	38.0
Total Split (%)	36.7%	36.7%	100.0%	63.3%
Yellow Time (s)	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead		Lag
Lead-Lag Optimize?	Yes	Yes		Yes
Recall Mode	None	None	Min	Min
v/c Ratio	0.59	0.71	0.45	0.81
Control Delay	22.9	25.1	0.4	13.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	22.9	25.1	0.4	13.4
Queue Length 50th (ft)	74	108	0	222
Queue Length 95th (ft)	94	#195	0	221
Internal Link Dist (ft)			1033	2203
Turn Bay Length (ft)		143		
Base Capacity (vph)	453	534	3415	2100
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.54	0.65	0.45	0.78

Intersection Summary

Cycle Length: 60

Actuated Cycle Length: 55.9

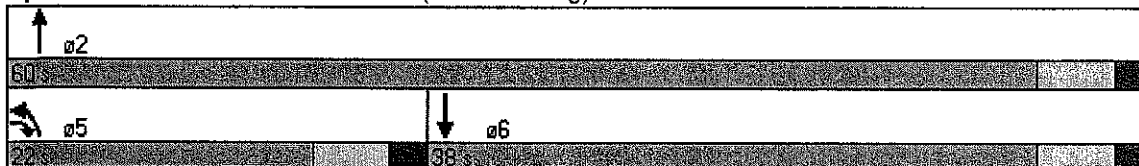
Natural Cycle: 50

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

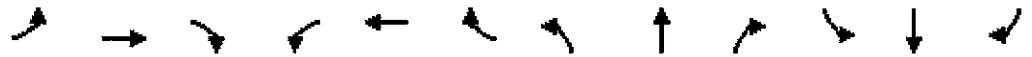
Splits and Phases: 2: Route 314 (Western Leg) & Route 611



2007 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↖	↗	↗	↖	↑↑	↗	↖	↕	↕
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	15	15	15	12	12	14	11	12	14	11	12	12
Grade (%)		6%			5%			3%				-7%
Total Lost time (s)		4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor		1.00		0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	
Fr _t		0.96		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	
Fl _t Protected		0.98		0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1863		1639	1646	1647	1685	3486	1664	1770	3659	
Fl _t Permitted		0.98		0.95	0.95	1.00	0.26	1.00	1.00	0.14	1.00	
Satd. Flow (perm)		1863		1639	1646	1647	459	3486	1664	254	3659	
Volume (vph)	4	2	3	348	7	139	5	1053	415	167	897	7
Peak-hour factor, PHF	0.56	0.56	0.56	0.89	0.89	0.89	0.99	0.99	0.99	0.82	0.82	0.82
Adj. Flow (vph)	7	4	5	391	8	156	5	1064	419	204	1094	9
RTOR Reduction (vph)	0	5	0	0	0	108	0	0	176	0	1	0
Lane Group Flow (vph)	0	11	0	196	203	48	5	1064	243	204	1102	0
Turn Type	Split		Split		pm+ov	Perm	pm+ov		pm+pt			
Protected Phases	4	4	8		8	1	2		8	1	6	
Permitted Phases					8		2	2		6		
Actuated Green, G (s)	0.7		9.5		9.5	16.0	21.9	21.9	31.4	33.9	33.9	
Effective Green, g (s)	2.7		11.5		11.5	19.5	25.4	25.4	36.9	37.4	37.4	
Actuated g/C Ratio	0.04		0.18		0.18	0.31	0.40	0.40	0.58	0.59	0.59	
Clearance Time (s)	6.0		6.0		6.0	5.5	7.5	7.5	6.0	5.5	7.5	
Vehicle Extension (s)	3.0		3.0		3.0	3.0	5.0	5.0	3.0	3.0	5.0	
Lane Grp Cap (vph)	79		296		298	609	183	1392	965	340	2152	
v/s Ratio Prot	c0.01		0.12		c0.12	0.01		c0.31	0.05	c0.08	0.30	
v/s Ratio Perm						0.02	0.01		0.10	0.28		
v/c Ratio	0.14		0.66		0.68	0.08	0.03	0.76	0.25	0.60	0.51	
Uniform Delay, d1	29.3		24.2		24.3	15.7	11.6	16.5	6.6	9.6	7.7	
Progression Factor	1.00		1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.8		5.5		6.3	0.1	0.1	3.0	0.1	3.0	0.4	
Delay (s)	30.2		29.7		30.6	15.7	11.7	19.5	6.7	12.6	8.1	
Level of Service	C		C		C	B	B	B	A	B	A	
Approach Delay (s)	30.2				26.1			15.9			8.8	
Approach LOS	C				C			B			A	
Intersection Summary												
HCM Average Control Delay			14.9		HCM Level of Service				B			
HCM Volume to Capacity ratio			0.68									
Actuated Cycle Length (s)			63.6		Sum of lost time (s)				16.0			
Intersection Capacity Utilization			64.9%		ICU Level of Service				C			
Analysis Period (min)			15									
c Critical Lane Group												

2007 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Lane Group	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↔	↖	↗	↗	↖	↑↑	↗	↖	↑↑
Volume (vph)	2	348	7	139	5	1053	415	167	897
Lane Group Flow (vph)	16	196	203	156	5	1064	419	204	1103
Turn Type		Split		pm+ov	Perm		pm+ov	pm+pt	
Protected Phases	4	8	8	1		2	8	1	6
Permitted Phases				8	2		2	6	
Detector Phases	4	8	8	1	2	2	8	1	6
Minimum Initial (s)	4.0	7.0	7.0	4.0	10.0	10.0	7.0	4.0	10.0
Minimum Split (s)	10.0	13.0	13.0	9.5	17.5	17.5	13.0	9.5	17.5
Total Split (s)	10.0	16.0	16.0	12.0	32.0	32.0	16.0	12.0	44.0
Total Split (%)	14.3%	22.9%	22.9%	17.1%	45.7%	45.7%	22.9%	17.1%	62.9%
Yellow Time (s)	4.0	4.0	4.0	5.5	5.5	5.5	4.0	5.5	5.5
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0
Lead/Lag				Lead	Lag	Lag		Lead	
Lead-Lag Optimize?				Yes	Yes	Yes		Yes	
Recall Mode	None	None	None	None	Min	Min	None	None	Min
v/c Ratio	0.09	0.61	0.63	0.21	0.03	0.71	0.33	0.57	0.47
Control Delay	26.0	32.4	33.1	3.9	11.6	16.7	1.1	15.3	6.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.0	32.4	33.1	3.9	11.6	16.7	1.1	15.3	6.8
Queue Length 50th (ft)	4	65	67	0	1	145	0	22	80
Queue Length 95th (ft)	12	#174	#182	34	8	274	14	81	161
Internal Link Dist (ft)	105		2012			2203			2327
Turn Bay Length (ft)		250		250	73		350	183	
Base Capacity (vph)	173	335	337	731	210	1598	1276	361	2394
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.59	0.60	0.21	0.02	0.67	0.33	0.57	0.46

Intersection Summary

Cycle Length: 70

Actuated Cycle Length: 58.7

Natural Cycle: 60

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles

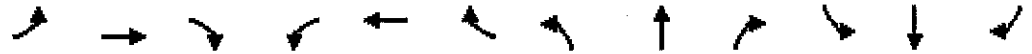
Splits and Phases: 3: Woodland Road/Private Driveway & Route 611

ø1	ø2	ø4	ø8
12 s	32 s	10 s	16 s
ø6			
44 s			

2007 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

4: Meadows Road/Trinity Hill Road & Route 611



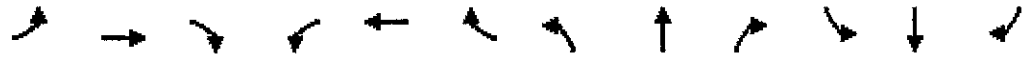
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↙	↕		↙	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	13	13	12	12	12	11	12	12	11	12	12
Grade (%)	2%			8%			1%			-1%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Fr _t	0.865			0.882								
Flt Protected					0.997		0.950			0.950		
Satd. Flow (prot)	0	1648	0	0	1572	0	1702	3522	0	1670	3455	0
Flt Permitted					0.997		0.950			0.950		
Satd. Flow (perm)	0	1648	0	0	1572	0	1702	3522	0	1670	3455	0
Headway Factor	0.97	0.97	0.97	1.05	1.05	1.05	1.05	1.01	1.01	1.04	0.99	0.99
Link Speed (mph)	35			35			45			45		
Link Distance (ft)	158			1027			2407			3261		
Travel Time (s)	3.1			20.0			36.5			49.4		
Volume (vph)	0	0	3	1	1	14	3	1191	1	11	1067	1
Peak Hour Factor	0.75	0.75	0.75	0.50	0.50	0.50	0.97	0.97	0.97	0.83	0.83	0.83
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	5%	5%	5%
Adj. Flow (vph)	0	0	4	2	2	28	3	1228	1	13	1286	1
Lane Group Flow (vph)	0	4	0	0	32	0	3	1229	0	13	1287	0
Sign Control	Stop			Stop			Free			Free		

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	43.0%
	ICU Level of Service A
Analysis Period (min)	15

2007 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

4: Meadows Road/Trinity Hill Road & Route 611

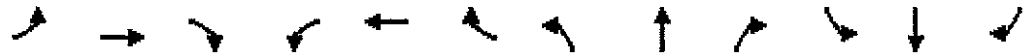


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Sign Control		Stop			Stop			Free			Free	
Grade		2%			8%			1%			-1%	
Volume (veh/h)	0	0	3	1	1	14	3	1191	1	11	1067	1
Peak Hour Factor	0.75	0.75	0.75	0.50	0.50	0.50	0.97	0.97	0.97	0.83	0.83	0.83
Hourly flow rate (vph)	0	0	4	2	2	28	3	1228	1	13	1286	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1962	2548	643	1908	2548	614	1287			1229		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1962	2548	643	1908	2548	614	1287			1229		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.2		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	95	92	94	99			98		
cM capacity (veh/h)	32	26	416	40	25	434	535			547		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	4	32	3	819	410	13	857	430				
Volume Left	0	2	3	0	0	13	0	0				
Volume Right	4	28	0	0	1	0	0	1				
cSH	416	165	535	1700	1700	547	1700	1700				
Volume to Capacity	0.01	0.19	0.01	0.48	0.24	0.02	0.50	0.25				
Queue Length 95th (ft)	1	17	0	0	0	2	0	0				
Control Delay (s)	13.7	31.9	11.8	0.0	0.0	11.8	0.0	0.0				
Lane LOS	B	D	B			B						
Approach Delay (s)	13.7	31.9	0.0			0.1						
Approach LOS	B	D										
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			43.0%			ICU Level of Service				A		
Analysis Period (min)			15									

2007 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↖	↕		↖	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	10	10	10	11	12	12	11	12	12
Grade (%)	0%			9%			2%			5%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	0.938			0.937			0.998			0.996		
Flt Protected	0.978			0.983			0.950			0.950		
Satd. Flow (prot)	0	1709	0	0	1529	0	1694	3497	0	1753	3613	0
Flt Permitted	0.978			0.983			0.950			0.950		
Satd. Flow (perm)	0	1709	0	0	1529	0	1694	3497	0	1753	3613	0
Headway Factor	1.00	1.00	1.00	1.16	1.16	1.16	1.06	1.01	1.01	1.01	0.97	0.97
Link Speed (mph)	30			30			45			45		
Link Distance (ft)	294			1492			3261			2754		
Travel Time (s)	6.7			33.9			49.4			41.7		
Volume (vph)	20	4	20	12	7	16	31	1156	18	19	1047	31
Peak Hour Factor	0.90	0.90	0.90	0.75	0.90	0.75	0.90	0.97	0.97	0.77	0.77	0.90
Adj. Flow (vph)	22	4	22	16	8	21	34	1192	19	25	1360	34
Lane Group Flow (vph)	0	48	0	0	45	0	34	1211	0	25	1394	0
Sign Control	Stop			Stop			Free			Free		

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 42.9% ICU Level of Service A

Analysis Period (min) 15

2007 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↖	↕↔		↖	↕↔	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			9%			2%			-5%	
Volume (veh/h)	20	4	20	12	7	16	31	1156	18	19	1047	31
Peak Hour Factor	0.90	0.90	0.90	0.75	0.90	0.75	0.90	0.97	0.97	0.77	0.77	0.90
Hourly flow rate (vph)	22	4	22	16	8	21	34	1192	19	25	1360	34
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2116	2706	697	2024	2713	605	1394			1210		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2116	2706	697	2024	2713	605	1394			1210		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	76	94	32	57	95	93			96		
cM capacity (veh/h)	17	19	383	24	18	440	486			572		

Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	49	45	34	795	416	25	906	488
Volume Left	22	16	34	0	0	25	0	0
Volume Right	22	21	0	0	19	0	0	34
cSH	30	39	486	1700	1700	572	1700	1700
Volume to Capacity	1.62	1.15	0.07	0.47	0.24	0.04	0.53	0.29
Queue Length 95th (ft)	140	113	6	0	0	3	0	0
Control Delay (s)	591.8	353.5	13.0	0.0	0.0	11.6	0.0	0.0
Lane LOS	F	F	B			B		
Approach Delay (s)	591.8	353.5	0.4			0.2		
Approach LOS	F	F						

Intersection Summary		
Average Delay		16.5
Intersection Capacity Utilization	42.9%	ICU Level of Service
Analysis Period (min)		15
		A

2007 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗	↖	↕	↕	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	10	10	12	12	12	12
Grade (%)	4%			7%	-6%	
Total Lost time (s)		4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00	1.00	0.95	0.95	
Fr _t		0.86	1.00	1.00	0.99	
Fr _l Protected		1.00	0.95	1.00	1.00	
Satd. Flow (prot)		1445	1708	3415	3627	
Fr _l Permitted		1.00	0.13	1.00	1.00	
Satd. Flow (perm)		1445	242	3415	3627	
Volume (vph)	0	135	158	1478	1247	44
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.97	0.97
Adj. Flow (vph)	0	141	165	1540	1286	45
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	0	141	165	1540	1331	0
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Turn Type		Over pm+pt				
Protected Phases		5	5	2	6	
Permitted Phases		2				
Actuated Green, G (s)		10.6	47.5	54.5	30.9	
Effective Green, g (s)		12.6	50.5	54.5	33.9	
Actuated g/C Ratio		0.23	0.93	1.00	0.62	
Clearance Time (s)		6.0	6.0	7.0	7.0	
Vehicle Extension (s)		3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)		334	563	3415	2256	
v/s Ratio Prot		0.10	0.07	0.45	0.37	
v/s Ratio Perm		0.20				
v/c Ratio		0.42	0.29	0.45	0.59	
Uniform Delay, d ₁		17.8	3.0	0.0	6.2	
Progression Factor		1.00	1.00	1.00	1.00	
Incremental Delay, d ₂		0.9	0.3	0.1	0.4	
Delay (s)		18.7	3.3	0.1	6.5	
Level of Service		B	A	A	A	
Approach Delay (s)	18.7				0.4	6.5
Approach LOS	B				A	A
Intersection Summary						
HCM Average Control Delay			3.8	HCM Level of Service		A
HCM Volume to Capacity ratio			0.54			
Actuated Cycle Length (s)			54.5	Sum of lost time (s)		4.0
Intersection Capacity Utilization			51.3%	ICU Level of Service		A
Analysis Period (min)			15			

c Critical Lane Group

2007 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Lane Group	EBR	NBI	NBT	SBT
Lane Configurations	↗	↖	↑↑	↑↓
Volume (vph)	135	158	1478	1247
Lane Group Flow (vph)	141	165	1540	1331
Turn Type	Over pm+pt			
Protected Phases	5	5	2	6
Permitted Phases	2			
Detector Phases	5	5	2	6
Minimum Initial (s)	4.0	4.0	4.0	4.0
Minimum Split (s)	10.0	10.0	23.0	23.0
Total Split (s)	22.0	22.0	60.0	38.0
Total Split (%)	36.7%	36.7%	100.0%	63.3%
Yellow Time (s)	4.0	4.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead		Lag
Lead-Lag Optimize?	Yes	Yes		Yes
Recall Mode	None	None	Min	Min
v/c Ratio	0.42	0.31	0.45	0.59
Control Delay	18.1	3.0	0.4	8.0
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	18.1	3.0	0.4	8.0
Queue Length 50th (ft)	32	0	0	106
Queue Length 95th (ft)	81	23	0	209
Internal Link Dist (ft)			1031	2203
Turn Bay Length (ft)	143			
Base Capacity (vph)	442	633	3415	2339
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.32	0.26	0.45	0.57

Intersection Summary

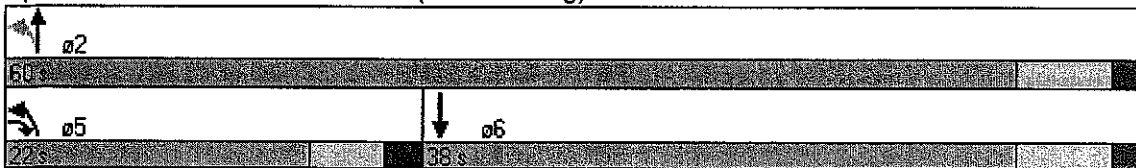
Cycle Length: 60

Actuated Cycle Length: 54.7

Natural Cycle: 40

Control Type: Semi Act-Uncoord

Splits and Phases: 2: Route 314 (Western Leg) & Route 611



2007 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↘	↙	↗	↘	↗	↗	↘	↗	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	15	15	15	12	12	14	11	12	14	11	12	12
Grade (%)		6%			5%			3%				-7%
Total Lost time (s)		4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00		0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	
Frnt		0.91		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	
Flt Protected		0.99		0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1783		1639	1645	1647	1685	3486	1664	1770	3655	
Flt Permitted		0.99		0.95	0.95	1.00	0.30	1.00	1.00	0.14	1.00	
Satd. Flow (perm)		1783		1639	1645	1647	531	3486	1664	264	3655	
Volume (vph)	5	1	13	354	4	77	14	1046	418	125	924	14
Peak-hour factor, PHF	0.61	0.61	0.61	0.92	0.92	0.92	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	8	2	21	385	4	84	14	1067	427	128	943	14
RTOR Reduction (vph)	0	20	0	0	0	59	0	0	175	0	1	0
Lane Group Flow (vph)	0	11	0	193	196	25	14	1067	252	128	956	0
Turn Type	Split			Split		pm+ov	Perm		pm+ov	pm+pt		
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases						8	2		2		6	
Actuated Green, G (s)		1.3		10.4	10.4	15.1	20.7	20.7	31.1	30.9	30.9	
Effective Green, g (s)		3.3		12.4	12.4	18.6	24.2	24.2	36.6	34.4	34.4	
Actuated g/C Ratio		0.05		0.20	0.20	0.30	0.39	0.39	0.59	0.55	0.55	
Clearance Time (s)		6.0		6.0	6.0	5.5	7.5	7.5	6.0	5.5	7.5	
Vehicle Extension (s)		3.0		3.0	3.0	3.0	5.0	5.0	3.0	3.0	5.0	
Lane Grp Cap (vph)		95		327	328	599	207	1358	981	297	2025	
v/s Ratio Prot		c0.01		0.12	c0.12	0.00		c0.31	0.05	0.04	c0.26	
v/s Ratio Perm						0.01	0.03		0.10	0.20		
v/c Ratio		0.12		0.59	0.60	0.04	0.07	0.79	0.26	0.43	0.47	
Uniform Delay, d1		28.0		22.5	22.6	15.4	11.9	16.7	6.2	9.6	8.4	
Progression Factor		1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2		0.6		2.8	2.9	0.0	0.3	3.6	0.1	1.0	0.4	
Delay (s)		28.6		25.4	25.5	15.5	12.2	20.2	6.3	10.6	8.7	
Level of Service		C		C	C	B	B	C	A	B	A	
Approach Delay (s)		28.6			23.7			16.2			8.9	
Approach LOS		C			C			B			A	

Intersection Summary			
HCM Average Control Delay	14.9	HCM Level of Service	B
HCM Volume to Capacity ratio	0.66		
Actuated Cycle Length (s)	62.1	Sum of lost time (s)	16.0
Intersection Capacity Utilization	62.4%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

2007 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Lane Group	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↕	↖	↗	↗	↖	↕	↗	↖	↕
Volume (vph)	1	354	4	77	14	1046	418	125	924
Lane Group Flow (vph)	31	193	196	84	14	1067	427	128	957
Turn Type		Split		pm+ov	Perm		pm+ov	pm+pt	
Protected Phases	4	8	8	1		2	8	1	6
Permitted Phases				8	2		2	6	
Detector Phases	4	8	8	1	2	2	8	1	6
Minimum Initial (s)	4.0	7.0	7.0	4.0	10.0	10.0	7.0	4.0	10.0
Minimum Split (s)	10.0	13.0	13.0	9.5	17.5	17.5	13.0	9.5	17.5
Total Split (s)	10.0	17.0	17.0	12.0	31.0	31.0	17.0	12.0	43.0
Total Split (%)	14.3%	24.3%	24.3%	17.1%	44.3%	44.3%	24.3%	17.1%	61.4%
Yellow Time (s)	4.0	4.0	4.0	5.5	5.5	5.5	4.0	5.5	5.5
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0
Lead/Lag				Lead	Lag	Lag		Lead	
Lead-Lag Optimize?				Yes	Yes	Yes		Yes	
Recall Mode	None	None	None	None	Min	Min	None	None	Min
v/c Ratio	0.16	0.54	0.55	0.12	0.06	0.72	0.32	0.38	0.46
Control Delay	19.3	29.8	30.0	4.7	14.0	18.2	1.1	9.6	8.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.3	29.8	30.0	4.7	14.0	18.2	1.1	9.6	8.0
Queue Length 50th (ft)	3	65	66	0	3	152	0	14	71
Queue Length 95th (ft)	16	#163	#166	26	15	283	14	47	161
Internal Link Dist (ft)	105		2012			2203			2327
Turn Bay Length (ft)		250		250	73		350	183	
Base Capacity (vph)	194	385	387	670	243	1598	1318	342	2282
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.50	0.51	0.13	0.06	0.67	0.32	0.37	0.42

Intersection Summary

Cycle Length: 70

Actuated Cycle Length: 57.7

Natural Cycle: 60

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Woodland Road/Private Driveway & Route 611

↖ ø1	↕ ø2	↖ ø4	↕ ø8
12s	31s	10s	17s
↘ ø6			
43s			

2007 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

4: Meadows Road/Trinity Hill Road & Route 611



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↙	↕		↙	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	13	13	12	12	12	11	12	12	11	12	12
Grade (%)		2%			8%			1%			-1%	
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.899			0.918							
Frt Protected		0.988			0.993			0.950		0.950		
Satd. Flow (prot)	0	1693	0	0	1630	0	1702	3522	0	1719	3557	0
Frt Permitted		0.988			0.993			0.950		0.950		
Satd. Flow (perm)	0	1693	0	0	1630	0	1702	3522	0	1719	3557	0
Headway Factor	0.97	0.97	0.97	1.05	1.05	1.05	1.05	1.01	1.01	1.04	0.99	0.99
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		158			1027			2407			3261	
Travel Time (s)		3.1			20.0			36.5			49.4	
Volume (vph)	1	0	3	3	6	14	6	1122	1	7	1057	1
Peak Hour Factor	0.50	0.50	0.50	0.39	0.39	0.39	0.92	0.92	0.92	0.89	0.89	0.89
Adj. Flow (vph)	2	0	6	8	15	36	7	1220	1	8	1188	1
Lane Group Flow (vph)	0	8	0	0	59	0	7	1221	0	8	1189	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary
 Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization 41.0% ICU Level of Service A
 Analysis Period (min) 15

2007 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

4: Meadows Road/Trinity Hill Road & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↖ ↗		↖ ↗		↖ ↗	
Sign Control	Stop			Stop			Free		Free		Free	
Grade	2%			8%			1%		1%		-1%	
Volume (veh/h)	1	0	3	3	6	14	6	1122	1	7	1057	1
Peak Hour Factor	0.50	0.50	0.50	0.39	0.39	0.39	0.92	0.92	0.92	0.89	0.89	0.89
Hourly flow rate (vph)	2	0	6	8	15	36	7	1220	1	8	1188	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1870	2438	594	1849	2438	610	1189			1221		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1870	2438	594	1849	2438	610	1189			1221		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	92	100	99	83	49	92	99			99		
cM capacity (veh/h)	24	30	448	44	30	437	583			567		

Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	8	59	7	813	408	8	792	397
Volume Left	2	8	7	0	0	8	0	0
Volume Right	6	36	0	0	1	0	0	1
cSH	82	77	583	1700	1700	567	1700	1700
Volume to Capacity	0.10	0.77	0.01	0.48	0.24	0.01	0.47	0.23
Queue Length 95th (ft)	8	93	1	0	0	1	0	0
Control Delay (s)	53.5	137.4	11.2	0.0	0.0	11.4	0.0	0.0
Lane LOS	F	F	B			B		
Approach Delay (s)	53.5	137.4	0.1			0.1		
Approach LOS	F	F						

Intersection Summary		
Average Delay	3.5	
Intersection Capacity Utilization	41.0%	ICU Level of Service A
Analysis Period (min)	15	

2007 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



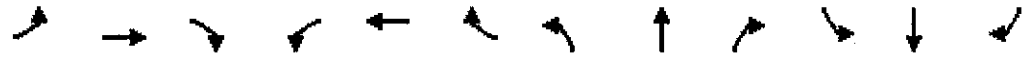
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	10	10	10	11	12	12	11	12	12
Grade (%)		0%			9%			2%			-5%	
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Fr _t		0.940			0.960			0.996			0.996	
F _{lt} Protected		0.978			0.971		0.950			0.950		
Satd. Flow (prot)	0	1712	0	0	1548	0	1694	3490	0	1753	3613	0
F _{lt} Permitted		0.978			0.971		0.950			0.950		
Satd. Flow (perm)	0	1712	0	0	1548	0	1694	3490	0	1753	3613	0
Headway Factor	1.00	1.00	1.00	1.16	1.16	1.16	1.06	1.01	1.01	1.01	0.97	0.97
Link Speed (mph)		30			30			45			45	
Link Distance (ft)		294			1492			3261			2754	
Travel Time (s)		6.7			33.9			49.4			41.7	
Volume (vph)	28	6	27	31	6	15	25	1079	33	14	1007	26
Peak Hour Factor	0.90	0.90	0.90	0.72	0.90	0.72	0.90	0.94	0.94	0.95	0.95	0.90
Adj. Flow (vph)	31	7	30	43	7	21	28	1148	35	15	1060	29
Lane Group Flow (vph)	0	68	0	0	71	0	28	1183	0	15	1089	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary
 Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization 41.7% ICU Level of Service A
 Analysis Period (min) 15

2007 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			9%			2%			-5%	
Volume (veh/h)	28	6	27	31	6	15	25	1079	33	14	1007	26
Peak Hour Factor	0.90	0.90	0.90	0.72	0.90	0.72	0.90	0.94	0.94	0.95	0.95	0.90
Hourly flow rate (vph)	31	7	30	43	7	21	28	1148	35	15	1060	29
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1758	2342	544	1814	2339	591	1089			1183		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1758	2342	544	1814	2339	591	1089			1183		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	25	80	94	0	80	95	96			97		
cM capacity (veh/h)	41	33	483	37	33	449	636			586		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	68	71	28	765	418	15	707	382				
Volume Left	31	43	28	0	0	15	0	0				
Volume Right	30	21	0	0	35	0	0	29				
cSH	67	50	636	1700	1700	586	1700	1700				
Volume to Capacity	1.01	1.42	0.04	0.45	0.25	0.03	0.42	0.22				
Queue Length 95th (ft)	128	166	3	0	0	2	0	0				
Control Delay (s)	219.0	409.3	10.9	0.0	0.0	11.3	0.0	0.0				
Lane LOS	F	F	B				B					
Approach Delay (s)	219.0	409.3	0.3			0.2						
Approach LOS	F	F										
Intersection Summary												
Average Delay			18.0									
Intersection Capacity Utilization			41.7%			ICU Level of Service				A		
Analysis Period (min)			15									

2017 PROJECTED CONDITIONS

2017 Projected Conditions - With Route 611/Route 314 Signalization
 Friday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗	↘	↑↑	↑↑	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	10	10	12	12	12	12
Grade (%)	4%			7%	-6%	
Total Lost time (s)		4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00	1.00	0.95	0.95	
Frt		0.86	1.00	1.00	0.99	
Flt Protected		1.00	0.95	1.00	1.00	
Satd. Flow (prot)		1445	1708	3415	3582	
Flt Permitted		1.00	0.95	1.00	1.00	
Satd. Flow (perm)		1445	1708	3415	3582	
Volume (vph)	0	197	401	1673	1349	73
Peak-hour factor, PHF	0.67	0.67	0.95	0.95	0.76	0.76
Adj. Flow (vph)	0	294	422	1761	1775	96
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	0	294	422	1761	1871	0
Heavy Vehicles (%)	4%	4%	2%	2%	3%	3%
Turn Type		Over	Prot			
Protected Phases		5	5	2	6	
Permitted Phases						
Actuated Green, G (s)		15.4	15.4	59.0	32.1	
Effective Green, g (s)		17.4	17.4	59.0	33.6	
Actuated g/C Ratio		0.29	0.29	1.00	0.57	
Clearance Time (s)		6.0	6.0	7.0	5.5	
Vehicle Extension (s)		3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)		426	504	3415	2040	
v/s Ratio Prot		0.20	c0.25	0.52	c0.52	
v/s Ratio Perm						
v/c Ratio		0.69	0.84	0.52	0.92	
Uniform Delay, d1		18.4	19.5	0.0	11.4	
Progression Factor		1.00	1.00	1.00	1.00	
Incremental Delay, d2		4.8	11.6	0.1	7.0	
Delay (s)		23.2	31.0	0.1	18.5	
Level of Service		C	C	A	B	
Approach Delay (s)	23.2			6.1	18.5	
Approach LOS	C			A	B	
Intersection Summary						
HCM Average Control Delay			12.6		HCM Level of Service	B
HCM Volume to Capacity ratio			0.89			
Actuated Cycle Length (s)			59.0		Sum of lost time (s)	8.0
Intersection Capacity Utilization			68.5%		ICU Level of Service	C
Analysis Period (min)			15			

c Critical Lane Group

2017 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Lane Group	EBR	NBL	NBT	SBT
Lane Configurations	↗	↖	↑↑	↑↑
Volume (vph)	197	401	1673	1349
Lane Group Flow (vph)	294	422	1761	1871
Turn Type	Over	Prot		
Protected Phases	5	5	2	6
Permitted Phases				
Detector Phases	5	5	2	6
Minimum Initial (s)	4.0	4.0	4.0	4.0
Minimum Split (s)	11.0	11.0	23.0	21.5
Total Split (s)	22.0	22.0	60.0	38.0
Total Split (%)	36.7%	36.7%	100.0%	63.3%
Yellow Time (s)	4.0	4.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	0.5
Lead/Lag	Lead	Lead		Lag
Lead-Lag Optimize?	Yes	Yes		Yes
Recall Mode	None	None	Min	Min
v/c Ratio	0.69	0.84	0.52	0.92
Control Delay	28.0	35.4	0.6	20.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	28.0	35.4	0.6	20.7
Queue Length 50th (ft)	92	140	0	286
Queue Length 95th (ft)	113	#280	0	277
Internal Link Dist (ft)			1029	2209
Turn Bay Length (ft)		143		
Base Capacity (vph)	437	516	3415	2050
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.67	0.82	0.52	0.91

Intersection Summary

Cycle Length: 60

Actuated Cycle Length: 59

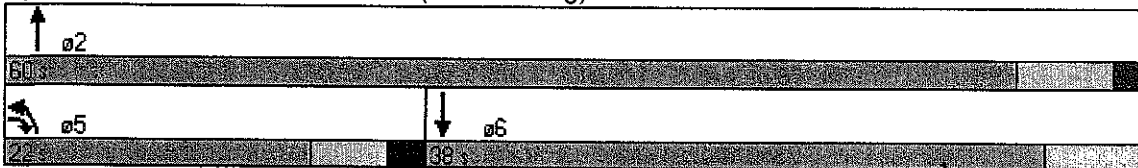
Natural Cycle: 65

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Route 314 (Western Leg) & Route 611



2017 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↖	↗	↗	↖	↕	↗	↖	↕	↖
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	15	15	15	12	12	14	11	12	14	11	12	12
Grade (%)		6%			5%			3%				-7%
Total Lost time (s)		4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00		0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	
Fr _t		0.96		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	
Fl _t Protected		0.98		0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1858		1639	1647	1647	1685	3486	1664	1770	3658	
Fl _t Permitted		0.98		0.95	0.95	1.00	0.21	1.00	1.00	0.13	1.00	
Satd. Flow (perm)		1858		1639	1647	1647	367	3486	1664	244	3658	
Volume (vph)	5	3	4	364	9	154	6	1235	432	176	1054	9
Peak-hour factor, PHF	0.56	0.56	0.56	0.89	0.89	0.89	0.99	0.99	0.99	0.82	0.82	0.82
Adj. Flow (vph)	9	5	7	409	10	173	6	1247	436	215	1285	11
RTOR Reduction (vph)	0	7	0	0	0	104	0	0	181	0	1	0
Lane Group Flow (vph)	0	14	0	205	214	69	6	1247	255	215	1295	0
Turn Type	Split		Split		pm+ov Perm		pm+ov		pm+pt			
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases						8	2		2		6	
Actuated Green, G (s)		1.4		10.3	10.3	16.9	23.0	23.0	33.3	35.1	35.1	
Effective Green, g (s)		3.4		12.3	12.3	20.4	26.5	26.5	38.8	38.6	38.6	
Actuated g/C Ratio		0.05		0.19	0.19	0.31	0.40	0.40	0.59	0.58	0.58	
Clearance Time (s)		6.0		6.0	6.0	5.5	7.5	7.5	6.0	5.5	7.5	
Vehicle Extension (s)		3.0		3.0	3.0	3.0	5.0	5.0	3.0	3.0	5.0	
Lane Grp Cap (vph)		95		304	306	606	147	1393	974	328	2130	
v/s Ratio Prot		c0.01		0.13	c0.13	0.01		c0.36	0.05	0.08	c0.35	
v/s Ratio Perm						0.03	0.02		0.10	0.30		
v/c Ratio		0.15		0.67	0.70	0.11	0.04	0.90	0.26	0.66	0.61	
Uniform Delay, d ₁		30.1		25.1	25.3	16.5	12.1	18.6	6.7	11.6	9.0	
Progression Factor		1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d ₂		0.7		5.8	6.8	0.1	0.2	8.3	0.1	4.7	0.7	
Delay (s)		30.8		30.9	32.1	16.6	12.4	26.9	6.9	16.3	9.7	
Level of Service		C		C	C	B	B	C	A	B	A	
Approach Delay (s)		30.8			27.2			21.7			10.6	
Approach LOS		C			C			C			B	

Intersection Summary			
HCM Average Control Delay	18.2	HCM Level of Service	B
HCM Volume to Capacity ratio	0.77		
Actuated Cycle Length (s)	66.3	Sum of lost time (s)	16.0
Intersection Capacity Utilization	70.9%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

2017 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Lane Group	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↕	↙	↕	↗	↙	↕	↗	↙	↕
Volume (vph)	3	364	9	154	6	1235	432	176	1054
Lane Group Flow (vph)	21	205	214	173	6	1247	436	215	1296
Turn Type		Split		pm+ov	Perm		pm+ov	pm+pt	
Protected Phases	4	8	8	1		2	8	1	6
Permitted Phases				8	2		2	6	
Detector Phases	4	8	8	1	2	2	8	1	6
Minimum Initial (s)	4.0	7.0	7.0	4.0	10.0	10.0	7.0	4.0	10.0
Minimum Split (s)	10.0	13.0	13.0	9.5	17.5	17.5	13.0	9.5	17.5
Total Split (s)	10.0	17.0	17.0	12.0	31.0	31.0	17.0	12.0	43.0
Total Split (%)	14.3%	24.3%	24.3%	17.1%	44.3%	44.3%	24.3%	17.1%	61.4%
Yellow Time (s)	4.0	4.0	4.0	5.5	5.5	5.5	4.0	5.5	5.5
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0
Lead/Lag				Lead	Lag	Lag		Lead	
Lead-Lag Optimize?				Yes	Yes	Yes		Yes	
Recall Mode	None	None	None	None	Min	Min	None	None	Min
v/c Ratio	0.12	0.63	0.66	0.24	0.04	0.85	0.35	0.62	0.57
Control Delay	25.5	33.8	34.7	5.1	14.2	24.3	1.2	18.9	9.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.5	33.8	34.7	5.1	14.2	24.3	1.2	18.9	9.3
Queue Length 50th (ft)	5	70	73	5	1	191	0	26	110
Queue Length 95th (ft)	14	#174	#184	42	9	#398	14	#94	210
Internal Link Dist (ft)	105		2012			2209			2327
Turn Bay Length (ft)		250		250	73		350	183	
Base Capacity (vph)	169	339	341	735	145	1498	1253	346	2271
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.60	0.63	0.24	0.04	0.83	0.35	0.62	0.57

Intersection Summary

Cycle Length: 70

Actuated Cycle Length: 62.7

Natural Cycle: 70

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Woodland Road/Private Driveway & Route 611

↙ #1	↕ #2	↗ #4	↙ #8
12s	31s	10s	17s
↙ #6			
43s			

2017 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

4: Meadows Road/Trinity Hill Road & Route 611



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↖	↕		↖	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	13	13	12	12	12	11	12	12	11	12	12
Grade (%)	2%			8%			1%			-1%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	0.865			0.879								
Flt Protected					0.997		0.950			0.950		
Satd. Flow (prot)	0	1648	0	0	1567	0	1702	3522	0	1670	3455	0
Flt Permitted					0.997		0.950			0.950		
Satd. Flow (perm)	0	1648	0	0	1567	0	1702	3522	0	1670	3455	0
Headway Factor	0.97	0.97	0.97	1.05	1.05	1.05	1.05	1.01	1.01	1.04	0.99	0.99
Link Speed (mph)	35			35			45			45		
Link Distance (ft)	158			1027			2407			3261		
Travel Time (s)	3.1			20.0			36.5			49.4		
Volume (vph)	0	0	4	1	1	17	4	1388	1	12	1234	1
Peak Hour Factor	0.75	0.75	0.75	0.50	0.50	0.50	0.97	0.97	0.97	0.83	0.83	0.83
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	5%	5%	5%
Adj. Flow (vph)	0	0	5	2	2	34	4	1431	1	14	1487	1
Lane Group Flow (vph)	0	5	0	0	38	0	4	1432	0	14	1488	0
Sign Control	Stop			Stop			Free			Free		

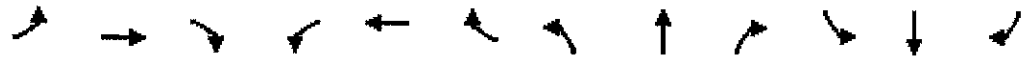
Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	48.4%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

4: Meadows Road/Trinity Hill Road & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↙	↕		↙	↕	
Sign Control		Stop			Stop			Free			Free	
Grade		2%			8%			1%			-1%	
Volume (veh/h)	0	0	4	1	1	17	4	1388	1	12	1234	1
Peak Hour Factor	0.75	0.75	0.75	0.50	0.50	0.50	0.97	0.97	0.97	0.83	0.83	0.83
Hourly flow rate (vph)	0	0	5	2	2	34	4	1431	1	14	1487	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2275	2956	744	2217	2957	716	1488			1432		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2275	2956	744	2217	2957	716	1488			1432		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.2		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	99	91	85	91	99			97		
cM/capacity (veh/h)	17	14	357	23	14	372	448			456		
Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	5	38	4	954	478	14	991	497				
Volume Left	0	2	4	0	0	14	0	0				
Volume Right	5	34	0	0	1	0	0	1				
cSH	357	116	448	1700	1700	456	1700	1700				
Volume to Capacity	0.01	0.33	0.01	0.56	0.28	0.03	0.58	0.29				
Queue Length 95th (ft)	1	32	1	0	0	2	0	0				
Control Delay (s)	15.2	50.4	13.1	0.0	0.0	13.2	0.0	0.0				
Lane LOS	C	F	B			B						
Approach Delay (s)	15.2	50.4	0.0			0.1						
Approach LOS	C	F										
Intersection Summary												
Average Delay			0.8									
Intersection Capacity Utilization			48.4%			ICU Level of Service				A		
Analysis Period (min)			15									

2017 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↕			↕			↖	↗		↖	↗	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	10	10	10	11	12	12	11	12	12
Grade (%)	0%			9%			2%			-5%		
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Fr _t	0.938			0.934			0.998			0.997		
Fl _t Protected	0.978			0.983			0.950			0.950		
Satd. Flow (prot)	0	1709	0	0	1524	0	1694	3497	0	1753	3617	0
Fl _t Permitted	0.978			0.983			0.950			0.950		
Satd. Flow (perm)	0	1709	0	0	1524	0	1694	3497	0	1753	3617	0
Headway Factor	1.00	1.00	1.00	1.16	1.16	1.16	1.06	1.01	1.01	1.01	0.97	0.97
Link Speed (mph)	30			30			45			45		
Link Distance (ft)	294			1492			3261			2754		
Travel Time (s)	6.7			33.9			49.4			41.7		
Volume (vph)	20	4	20	13	7	18	31	1354	20	22	1214	31
Peak Hour Factor	0.90	0.90	0.90	0.75	0.90	0.75	0.90	0.97	0.97	0.77	0.77	0.90
Adj. Flow (vph)	22	4	22	17	8	24	34	1396	21	29	1577	34
Lane Group Flow (vph)	0	48	0	0	49	0	34	1417	0	29	1611	0
Sign Control	Stop			Stop			Free			Free		

Intersection Summary
 Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization 48.5% ICU Level of Service A
 Analysis Period (min) 15

2017 Projected Conditions - With Route 611/Route 314 Signalization

Friday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕↕		↗	↕↕	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			9%			2%			-5%	
Volume (veh/h)	20	4	20	13	7	18	31	1354	20	22	1214	31
Peak Hour Factor	0.90	0.90	0.90	0.75	0.90	0.75	0.90	0.97	0.97	0.77	0.77	0.90
Hourly flow rate (vph)	22	4	22	17	8	24	34	1396	21	29	1577	34
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2446	3136	806	2345	3143	708	1611			1416		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2446	3136	806	2345	3143	708	1611			1416		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	52	93	0	14	94	91			94		
cM capacity (veh/h)	4	9	325	10	9	376	401			477		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	49	49	34	931	486	29	1051	560				
Volume Left	22	17	34	0	0	29	0	0				
Volume Right	22	24	0	0	21	0	0	34				
cSH	8	18	401	1700	1700	477	1700	1700				
Volume to Capacity	6.44	2.66	0.09	0.55	0.29	0.06	0.62	0.33				
Queue Length 95th (ft)	Err	165	7	0	0	5	0	0				
Control Delay (s)	Err	1183.1	14.8	0.0	0.0	13.0	0.0	0.0				
Lane LOS	F	F	B				B					
Approach Delay (s)	Err	1183.1	0.4			0.2						
Approach LOS	F	F										
Intersection Summary												
Average Delay	171.8											
Intersection Capacity Utilization	48.5%			ICU Level of Service				A				
Analysis Period (min)	15											

2017 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗	↖	↕	↕	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	10	10	12	12	12	12
Grade (%)	4%			7%	-6%	
Total Lost time (s)		4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00	1.00	0.95	0.95	
Frt		0.86	1.00	1.00	1.00	
Flt Protected		1.00	0.95	1.00	1.00	
Satd. Flow (prot)		1445	1708	3415	3627	
Flt Permitted		1.00	0.95	1.00	1.00	
Satd. Flow (perm)		1445	1708	3415	3627	
Volume (vph)	0	160	187	1674	1396	48
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.97	0.97
Adj. Flow (vph)	0	167	195	1744	1439	49
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	0	167	195	1744	1488	0
Heavy Vehicles (%)	4%	4%	2%	2%	2%	2%
Turn Type		Over	Prot			
Protected Phases		5	5	2	6	
Permitted Phases						
Actuated Green, G (s)		11.3	11.3	54.4	30.1	
Effective Green, g (s)		13.3	13.3	54.4	33.1	
Actuated g/C Ratio		0.24	0.24	1.00	0.61	
Clearance Time (s)		6.0	6.0	7.0	7.0	
Vehicle Extension (s)		3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)		353	418	3415	2207	
v/s Ratio Prot		0.12	0.11	c0.51	c0.41	
v/s Ratio Perm						
v/c Ratio		0.47	0.47	0.51	0.67	
Uniform Delay, d1		17.6	17.5	0.0	7.1	
Progression Factor		1.00	1.00	1.00	1.00	
Incremental Delay, d2		1.0	0.8	0.1	0.8	
Delay (s)		18.6	18.4	0.1	7.9	
Level of Service		B	B	A	A	
Approach Delay (s)	18.6			2.0	7.9	
Approach LOS	B			A	A	
Intersection Summary						
HCM Average Control Delay		5.2		HCM Level of Service		A
HCM Volume to Capacity ratio		0.62				
Actuated Cycle Length (s)		54.4		Sum of lost time (s)		4.0
Intersection Capacity Utilization		57.1%		ICU Level of Service		B
Analysis Period (min)		15				
c Critical Lane Group						

2017 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

2: Route 314 (Western Leg) & Route 611



Lane Group	EBR	NBL	NBT	SBT
Lane Configurations	↗	↖	↑↑	↑↗
Volume (vph)	160	187	1674	1396
Lane Group Flow (vph)	167	195	1744	1488
Turn Type	Over	Prot		
Protected Phases	5	5	2	6
Permitted Phases				
Detector Phases	5	5	2	6
Minimum Initial (s)	4.0	4.0	4.0	4.0
Minimum Split (s)	10.0	10.0	23.0	23.0
Total Split (s)	22.0	22.0	60.0	38.0
Total Split (%)	36.7%	36.7%	100.0%	63.3%
Yellow Time (s)	4.0	4.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0
Lead/Lag	Lead	Lead		Lag
Lead-Lag Optimize?	Yes	Yes		Yes
Recall Mode	None	None	Min	Min
v/c Ratio	0.47	0.47	0.51	0.68
Control Delay	19.6	19.0	0.5	9.5
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	19.6	19.0	0.5	9.5
Queue Length 50th (ft)	45	52	0	141
Queue Length 95th (ft)	94	104	0	256
Internal Link Dist (ft)			1033	2203
Turn Bay Length (ft)		143		
Base Capacity (vph)	442	522	3415	2257
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.38	0.37	0.51	0.66

Intersection Summary

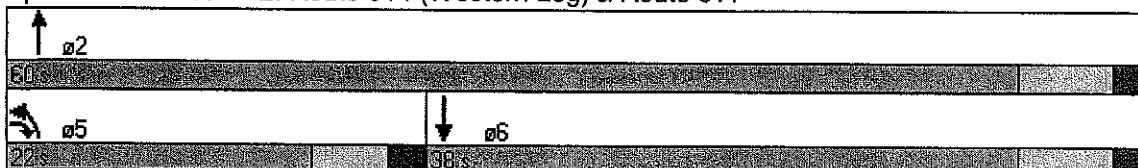
Cycle Length: 60

Actuated Cycle Length: 54.5

Natural Cycle: 40

Control Type: Semi Act-Uncoord

Splits and Phases: 2: Route 314 (Western Leg) & Route 611



2017 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕		↙	↘	↗	↖	↑↑	↗	↖	↑↑	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	15	15	15	12	12	14	11	12	14	11	12	12
Grade (%)		6%			5%			3%				-7%
Total Lost time (s)		4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00		0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	
Fr _t		0.91		1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	
Flt Protected		0.99		0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1781		1639	1645	1647	1685	3486	1664	1770	3655	
Flt Permitted		0.99		0.95	0.95	1.00	0.26	1.00	1.00	0.13	1.00	
Satd. Flow (perm)		1781		1639	1645	1647	460	3486	1664	246	3655	
Volume (vph)	6	1	16	367	5	79	17	1223	434	126	1061	17
Peak-hour factor, PHF	0.61	0.61	0.61	0.92	0.92	0.92	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	10	2	26	399	5	86	17	1248	443	129	1083	17
RTOR Reduction (vph)	0	24	0	0	0	61	0	0	180	0	2	0
Lane Group Flow (vph)	0	14	0	200	204	25	17	1248	263	129	1098	0
Turn Type	Split			Split		pm+ov	Perm		pm+ov	pm+pt		
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases						8	2		2	6		
Actuated Green, G (s)		2.1		10.3	10.3	15.1	22.8	22.8	33.1	33.1	33.1	
Effective Green, g (s)		4.1		12.3	12.3	18.6	26.3	26.3	38.6	36.6	36.6	
Actuated g/C Ratio		0.06		0.19	0.19	0.29	0.40	0.40	0.59	0.56	0.56	
Clearance Time (s)		6.0		6.0	6.0	5.5	7.5	7.5	6.0	5.5	7.5	
Vehicle Extension (s)		3.0		3.0	3.0	3.0	5.0	5.0	3.0	3.0	5.0	
Lane Grp Cap (vph)		112		310	311	573	186	1410	988	286	2058	
v/s Ratio Prot		c0.01		0.12	c0.12	0.00		c0.36	0.05	0.04	c0.30	
v/s Ratio Perm						0.01	0.04		0.11	0.21		
v/c Ratio		0.12		0.65	0.66	0.04	0.09	0.89	0.27	0.45	0.53	
Uniform Delay, d1		28.8		24.3	24.4	16.8	12.0	17.9	6.4	11.1	8.9	
Progression Factor		1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2		0.5		4.6	4.9	0.0	0.4	7.5	0.1	1.1	0.5	
Delay (s)		29.2		28.9	29.3	16.8	12.4	25.5	6.5	12.2	9.4	
Level of Service		C		C	C	B	B	C	A	B	A	
Approach Delay (s)		29.2		26.9				20.4			9.7	
Approach LOS		C		C				C			A	

Intersection Summary		
HCM Average Control Delay	17.6	HCM Level of Service
HCM Volume to Capacity ratio	0.73	B
Actuated Cycle Length (s)	65.0	Sum of lost time (s)
Intersection Capacity Utilization	67.8%	16.0
Analysis Period (min)	15	ICU Level of Service
		C
c Critical Lane Group		

2017 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

3: Woodland Road/Private Driveway & Route 611



Lane Group	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↕	↖	↗	↖	↖	↕	↗	↖	↕
Volume (vph)	1	367	5	79	17	1223	434	126	1061
Lane Group Flow (vph)	38	200	204	86	17	1248	443	129	1100
Turn Type		Split		pm+ov	Perm		pm+ov	pm+pt	
Protected Phases	4	8	8	1		2	8	1	6
Permitted Phases				8	2		2	6	
Detector Phases	4	8	8	1	2	2	8	1	6
Minimum Initial (s)	4.0	7.0	7.0	4.0	10.0	10.0	7.0	4.0	10.0
Minimum Split (s)	10.0	13.0	13.0	9.5	17.5	17.5	13.0	9.5	17.5
Total Split (s)	10.0	17.0	17.0	12.0	31.0	31.0	17.0	12.0	43.0
Total Split (%)	14.3%	24.3%	24.3%	17.1%	44.3%	44.3%	24.3%	17.1%	61.4%
Yellow Time (s)	4.0	4.0	4.0	5.5	5.5	5.5	4.0	5.5	5.5
All-Red Time (s)	2.0	2.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0
Lead/Lag				Lead	Lag	Lag		Lead	
Lead-Lag Optimize?				Yes	Yes	Yes		Yes	
Recall Mode	None	None	None	None	Min	Min	None	None	Min
v/c Ratio	0.20	0.60	0.61	0.13	0.09	0.83	0.34	0.40	0.52
Control Delay	19.1	33.2	33.5	4.7	15.4	23.9	1.2	10.7	9.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	19.1	33.2	33.5	4.7	15.4	23.9	1.2	10.7	9.5
Queue Length 50th (ft)	5	85	87	0	5	263	0	24	143
Queue Length 95th (ft)	17	#171	#175	26	17	#398	14	48	193
Internal Link Dist (ft)	105		2012			2203			2327
Turn Bay Length (ft)		250		250	73		350	183	
Base Capacity (vph)	190	356	357	658	204	1549	1285	327	2208
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.56	0.57	0.13	0.08	0.81	0.34	0.39	0.50

Intersection Summary

Cycle Length: 70

Actuated Cycle Length: 61.5

Natural Cycle: 65

Control Type: Semi Act-Uncoord

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Woodland Road/Private Driveway & Route 611

↖ ø1	↕ ø2	↗ ø4	↖ ø8
12 s	31 s	10 s	17 s
↘ ø5			
43 s			

2017 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

4: Meadowside Road/Trinity Hill Road & Route 611



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↖	↕		↖	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	13	13	12	12	12	11	12	12	11	12	12
Grade (%)		2%			8%			1%			-1%	
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.892			0.923							
Flt Protected		0.990			0.993		0.950			0.950		
Satd. Flow (prot)	0	1683	0	0	1639	0	1702	3522	0	1719	3557	0
Flt Permitted		0.990			0.993		0.950			0.950		
Satd. Flow (perm)	0	1683	0	0	1639	0	1702	3522	0	1719	3557	0
Headway Factor	0.97	0.97	0.97	1.05	1.05	1.05	1.05	1.01	1.01	1.04	0.99	0.99
Link Speed (mph)		35			35			45			45	
Link Distance (ft)		158			1027			2407			3261	
Travel Time (s)		3.1			20.0			36.5			49.4	
Volume (vph)	1	0	4	4	8	16	8	1295	1	7	1196	1
Peak Hour Factor	0.50	0.50	0.50	0.39	0.39	0.39	0.92	0.92	0.92	0.89	0.89	0.89
Adj. Flow (vph)	2	0	8	10	21	41	9	1408	1	8	1344	1
Lane Group Flow (vph)	0	10	0	0	72	0	9	1409	0	8	1345	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type	Other
Control Type	Unsignalized
Intersection Capacity Utilization	45.8%
ICU Level of Service	A
Analysis Period (min)	15

2017 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

4: Meadows Road/Trinity Hill Road & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Sign Control		Stop			Stop			Free			Free	
Grade		2%			8%			1%			-1%	
Volume (veh/h)	1	0	4	4	8	16	8	1295	1	7	1196	1
Peak Hour Factor	0.50	0.50	0.50	0.39	0.39	0.39	0.92	0.92	0.92	0.89	0.89	0.89
Hourly flow rate (vph)	2	0	8	10	21	41	9	1408	1	8	1344	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2133	2786	672	2121	2786	704	1345			1409		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2133	2786	672	2121	2786	704	1345			1409		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	100	98	62	0	89	98			98		
cM capacity (veh/h)	0	18	398	27	18	379	508			480		
Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	10	72	9	938	470	8	896	449				
Volume Left	2	10	9	0	0	8	0	0				
Volume Right	8	41	0	0	1	0	0	1				
cSH	0	43	508	1700	1700	480	1700	1700				
Volume to Capacity	Err	1.65	0.02	0.55	0.28	0.02	0.53	0.26				
Queue Length 95th (ft)	Err	181	1	0	0	1	0	0				
Control Delay (s)	Err	522.3	12.2	0.0	0.0	12.6	0.0	0.0				
Lane LOS	F	F	B			B						
Approach Delay (s)	Err	522.3	0.1			0.1						
Approach LOS	F	F										
Intersection Summary												
Average Delay												Err
Intersection Capacity Utilization			45.8%									ICU Level of Service
Analysis Period (min)			15									A

2017 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



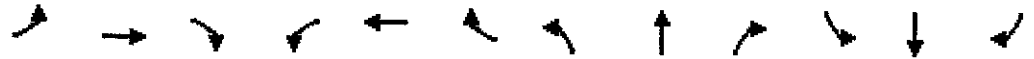
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	10	10	10	11	12	12	11	12	12
Grade (%)		0%			9%			2%			-5%	
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.940			0.958			0.996			0.996	
Frt Protected		0.978			0.971		0.950			0.950		
Satd. Flow (prot)	0	1712	0	0	1544	0	1694	3490	0	1753	3613	0
Frt Permitted		0.978			0.971		0.950			0.950		
Satd. Flow (perm)	0	1712	0	0	1544	0	1694	3490	0	1753	3613	0
Headway Factor	1.00	1.00	1.00	1.16	1.16	1.16	1.06	1.01	1.01	1.01	0.97	0.97
Link Speed (mph)		30			30			45			45	
Link Distance (ft)		294			1492			3261			2754	
Travel Time (s)		6.7			33.9			49.4			41.7	
Volume (vph)	28	6	27	34	6	17	25	1251	36	16	1143	26
Peak Hour Factor	0.90	0.90	0.90	0.72	0.90	0.72	0.90	0.94	0.94	0.95	0.95	0.90
Adj. Flow (vph)	31	7	30	47	7	24	28	1331	38	17	1203	29
Lane Group Flow (vph)	0	68	0	0	78	0	28	1369	0	17	1232	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary
 Area Type: Other
 Control Type: Unsignalized
 Intersection Capacity Utilization 47.0% ICU Level of Service A
 Analysis Period (min) 15

2017 Projected Conditions - With Route 611/Route 314 Signalization

Saturday P.M. Peak Hour

5: Grange Road/Green Springs Driveway & Route 611



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SB	SBT	SBR
Lane Configurations		↕			↕		↗	↕		↗	↕	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			9%			2%			-5%	
Volume (veh/h)	28	6	27	34	6	17	25	1251	36	16	1143	26
Peak Hour Factor	0.90	0.90	0.90	0.72	0.90	0.72	0.90	0.94	0.94	0.95	0.95	0.90
Hourly flow rate (vph)	31	7	30	47	7	24	28	1331	38	17	1203	29
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1999	2676	616	2074	2671	685	1232			1369		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1999	2676	616	2074	2671	685	1232			1369		
tC, single (s)	7.5	6.5	6.9	7.6	6.6	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	67	93	0	67	94	95			97		
cM capacity (veh/h)	23	20	433	20	20	390	561			497		
Direction Lane #	EB 1	WB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3				
Volume Total	68	78	28	887	482	17	802	430				
Volume Left	31	47	28	0	0	17	0	0				
Volume Right	30	24	0	0	38	0	0	29				
cSH	39	28	561	1700	1700	497	1700	1700				
Volume to Capacity	1.75	2.76	0.05	0.52	0.28	0.03	0.47	0.25				
Queue Length 95th (ft)	179	233	4	0	0	3	0	0				
Control Delay (s)	583.3	1093.0	11.7	0.0	0.0	12.5	0.0	0.0				
Lane LOS	F	F	B			B						
Approach Delay (s)	583.3	1093.0	0.2			0.2						
Approach LOS	F	F										
Intersection Summary												
Average Delay			44.7									
Intersection Capacity Utilization			47.0%			ICU Level of Service				A		
Analysis Period (min)			15									

APPENDIX H
SIGNAL WARRANT ANALYSIS WORKSHEETS

Traffic Signal Warrant Summary PennDOT Warrant (xi) - Peak Hour Volume

Municipality:	Paradise Township	Analyst:	EMM
County:	Monroe	TPD Project #:	CECO.A.00008

	Street Name	Lanes	Speed	Direction
Major Street:	Route 611	2	45	North-South
Minor Street:	Route 314	2	X	East-West

Volume Level Criteria

1. Is the critical speed of major street > 40 mph? Yes
2. Is the intersection in a built-up area of isolated community of <10,000 population? No

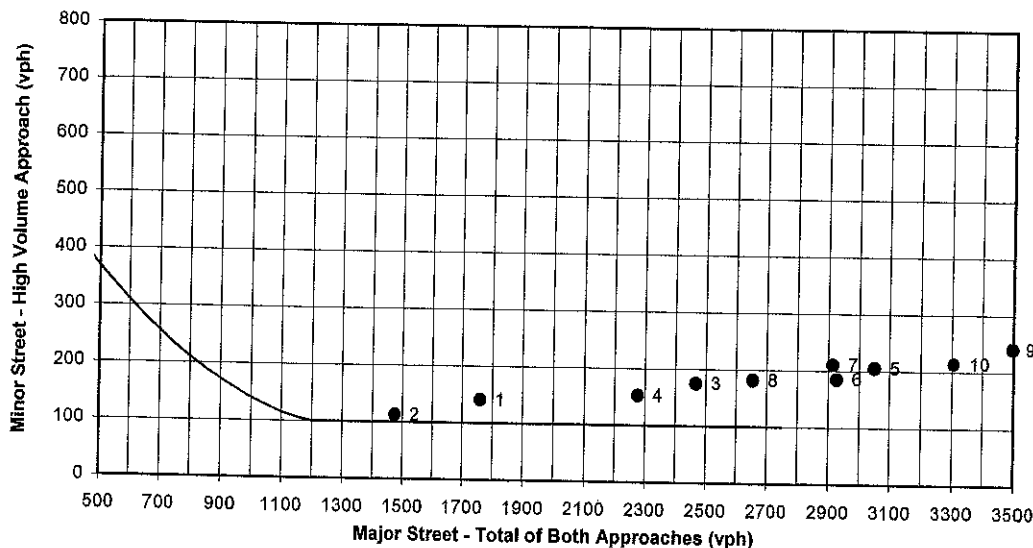
70% factor applied to volume thresholds

PennDOT Warrant xi - Peak Hour

Condition	Eastbound			Westbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2005 Existing FRI	9	0	132	0	0	0	287	791	0	0	643	38
2005 Existing SAT	17	0	96	0	0	0	115	757	0	0	583	18
2007 Base FRI	11	0	164	0	0	0	328	1148	0	0	949	42
2007 Base SAT	18	0	135	0	0	0	158	1127	0	0	971	19
2007 Projected FRI	41	0	164	0	0	0	328	1473	0	0	1184	64
2007 Projected SAT	50	0	135	0	0	0	158	1478	0	0	1247	44
2017 Base FRI	14	0	197	0	0	0	401	1348	0	0	1114	51
2017 Base SAT	22	0	160	0	0	0	187	1323	0	0	1120	23
2017 Projected FRI	44	0	197	0	0	0	401	1673	0	0	1349	73
2017 Projected SAT	54	0	160	0	0	0	187	1674	0	0	1396	48

Results

Condition	Major Street Volume	Minor Street Volume	Minor Street Warrant	Meets Warrant?
1 2005 Existing FRI	1759	141	100	yes
2 2005 Existing SAT	1473	113	100	yes
3 2007 Base FRI	2467	175	100	yes
4 2007 Base SAT	2275	153	100	yes
5 2007 Projected FRI	3049	205	100	yes
6 2007 Projected SAT	2927	185	100	yes
7 2017 Base FRI	2914	211	100	yes
8 2017 Base SAT	2653	182	100	yes
9 2017 Projected FRI	3496	241	100	yes
10 2017 Projected SAT	3305	214	100	yes



Traffic Signal Warrant Summary PennDOT Warrant (xi) - Peak Hour Volume

Municipality:	Paradise Township	Analyst:	EMM
County:	Monroe	TPD Project #:	CECO.A.00008

	Street Name	Lanes	Speed	Direction
Major Street:	Route 611	2	45	North-South
Minor Street:	Meadowside Road/Trinity Hill Road	1	X	East-West

Volume Level Criteria

1. Is the critical speed of major street > 40 mph? Yes _____
2. Is the intersection in a built-up area of isolated community of <10,000 population? No _____

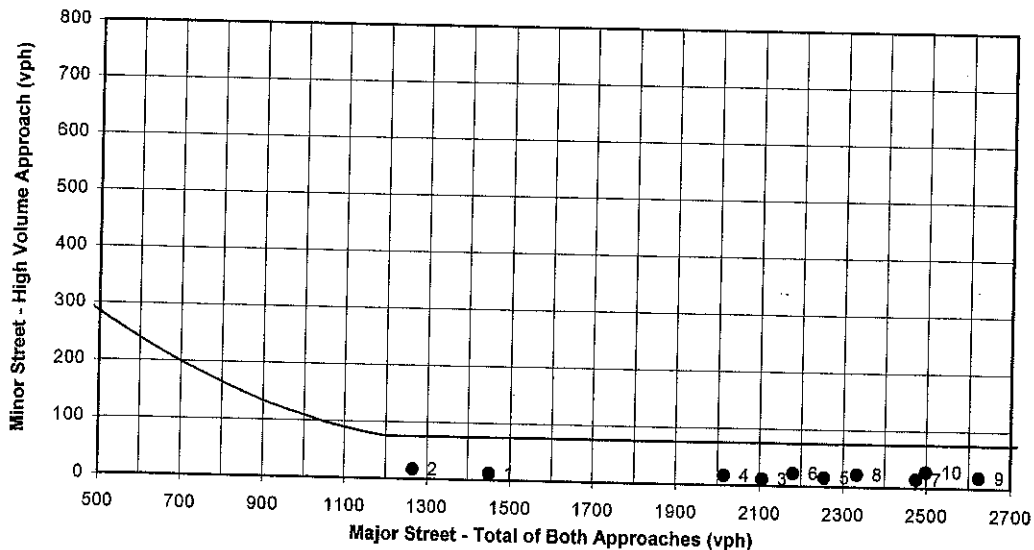
70% factor applied to volume thresholds

PennDOT Warrant xi - Peak Hour

Condition	Eastbound			Westbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2005 Existing FRI	0	0	3	1	1	10	3	786	1	5	654	1
2005 Existing SAT	1	0	3	3	6	8	6	705	1	1	551	1
2007 Base FRI	0	0	3	1	1	10	3	1142	1	5	954	1
2007 Base SAT	1	0	3	3	6	8	6	1070	1	1	936	1
2007 Projected FRI	0	0	3	1	1	14	3	1202	1	11	1037	1
2007 Projected SAT	1	0	3	3	6	14	6	1140	1	7	1025	1
2017 Base FRI	0	0	4	1	1	13	4	1342	1	6	1121	1
2017 Base SAT	1	0	4	4	8	10	8	1247	1	1	1075	1
2017 Projected FRI	0	0	4	1	1	17	4	1402	1	12	1204	1
2017 Projected SAT	1	0	4	4	8	16	8	1317	1	7	1164	1

Results

Condition	Major Street Volume	Minor Street Volume	Minor Street Warrant	Meets Warrant?
1 2005 Existing FRI	1450	12	75	no
2 2005 Existing SAT	1265	17	75	no
3 2007 Base FRI	2106	12	75	no
4 2007 Base SAT	2015	17	75	no
5 2007 Projected FRI	2255	16	75	no
6 2007 Projected SAT	2180	23	75	no
7 2017 Base FRI	2475	15	75	no
8 2017 Base SAT	2333	22	75	no
9 2017 Projected FRI	2624	19	75	no
10 2017 Projected SAT	2498	28	75	no



Traffic Signal Warrant Summary PennDOT Warrant (xi) - Peak Hour Volume

Municipality:	Paradise Township	Analyst:	EMM
County:	Monroe	TPD Project #:	CECO.A.00008

	Street Name	Lanes	Speed	Direction
Major Street:	Route 611	2	45	North-South
Minor Street:	Grange Road/Green Springs Driveway	1	X	East-West

Volume Level Criteria

1. Is the critical speed of major street > 40 mph? Yes _____
2. Is the intersection in a built-up area of isolated community of <10,000 population? No _____

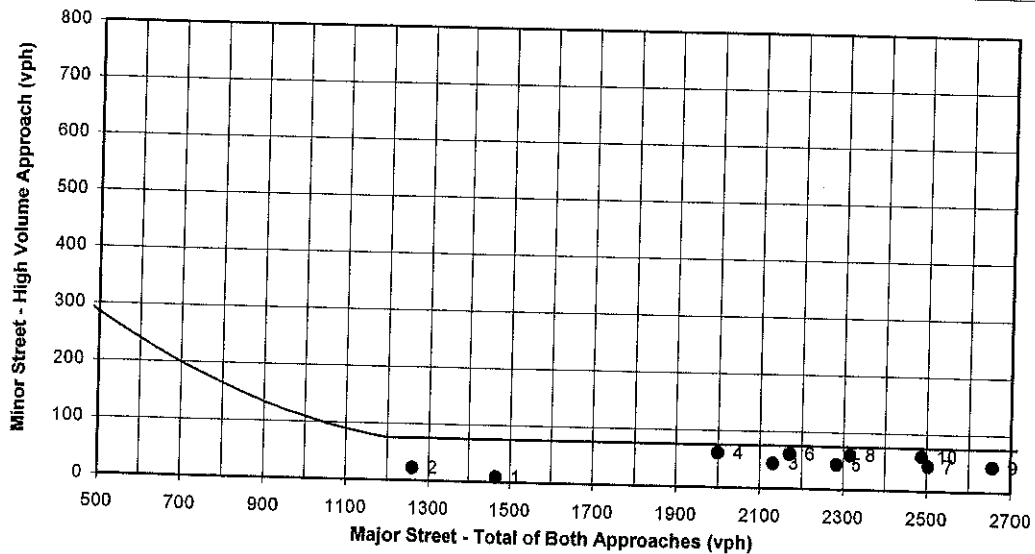
70% factor applied to volume thresholds

PennDOT Warrant xi - Peak Hour

Condition	Eastbound			Westbound			Northbound			Southbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2005 Existing FRI	0	0	0	2	0	7	0	788	8	12	658	0
2005 Existing SAT	0	0	0	15	0	8	0	699	15	7	538	0
2007 Base FRI	20	4	20	12	7	16	31	1103	18	19	928	31
2007 Base SAT	28	6	27	31	6	15	25	1021	33	14	880	26
2007 Projected FRI	20	4	20	12	7	16	31	1167	18	19	1017	31
2007 Projected SAT	28	6	27	31	6	15	25	1097	33	14	975	26
2017 Base FRI	20	4	20	13	7	18	31	1304	20	22	1095	31
2017 Base SAT	28	6	27	34	6	17	25	1197	36	16	1016	26
2017 Projected FRI	20	4	20	13	7	18	31	1368	20	22	1184	31
2017 Projected SAT	28	6	27	34	6	17	25	1273	36	16	1111	26

Results

Condition	Major Street Volume	Minor Street Volume	Minor Street Warrant	Meets Warrant?
1 2005 Existing FRI	1466	9	75	no
2 2005 Existing SAT	1259	23	75	no
3 2007 Base FRI	2130	44	75	no
4 2007 Base SAT	1999	61	75	no
5 2007 Projected FRI	2283	44	75	no
6 2007 Projected SAT	2170	61	75	no
7 2017 Base FRI	2503	44	75	no
8 2017 Base SAT	2316	61	75	no
9 2017 Projected FRI	2656	44	75	no
10 2017 Projected SAT	2487	61	75	no



APPENDIX I
AUXILARY LANE ANALYSIS WORKSHEETS

LEFT-TURN LANE WARRANTS

Unsignalized Intersection Left-Turn Storage Lane Warrant Analysis

Based on Highway Research Record 211

"Volume Warrants for Left-Turn Storage Lanes At Unsignalized Grade Intersection"

M.D. Harmelink

PROJECT INFORMATION

TPD Project Number:	CECO.A.00008
Intersection:	Woodland Road and Eastern Site Driveway
Movement:	Left Turns into Eastern Site Driveway
Analysis Period:	2007 Projected Conditions, Friday P.M. Peak Hour
Analyst:	EMM

INPUTS

Advancing Volume (V_A) =	324	
Opposing Volume (V_O) =	618	
Number of Left Turns =	24	$t_w = 2.92$
Speed Limit =	40	$t_A = 11.11$
Proportion of Left Turns =	0.07	$\lambda = 14.45$

Headway range, sec	Percent Headways in each range		Opposing Volume	Number of Headways in Range	Average Headway, sec	Duration, sec
0-1	7	0	618	41.07	0.5	20.53
1-2	37	7	618	187.56	1.5	281.35
2-3	58	37	618	132.34	2.5	330.84
			Total	360.97		632.72

Unblocked Time = 2324.70

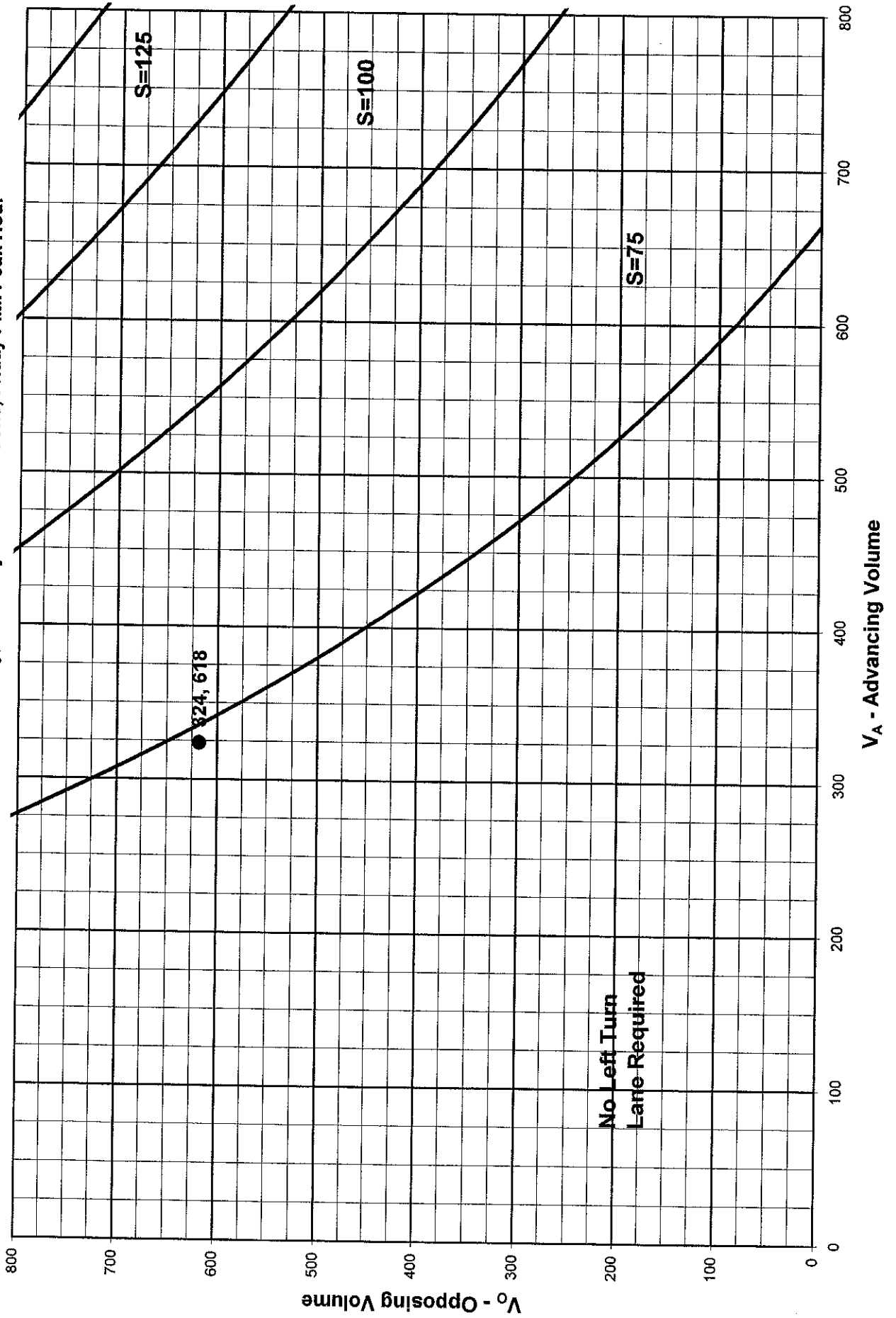
$\mu = 774.90$

Left turn lane warrants are for $\rho >$ than
8.00E-06 for 40 mph

$\rho =$	0.0187
75 foot lane $\rho^3 =$	6.49E-06
100 foot lane $\rho^4 =$	1.21E-07
125 foot lane $\rho^5 =$	2.26E-09
150 foot lane $\rho^6 =$	4.21E-11
175 foot lane $\rho^7 =$	7.85E-13

Volume Warrants for Left Turn Storage Lanes at Unsignalized Grade Intersections $V = 40$ m.p.h.; $L = 7\%$

Woodland Road and Eastern Site Driveway, 2007 Projected Conditions, Friday P.M. Peak Hour



Unsignalized Intersection Left-Turn Storage Lane Warrant Analysis

Based on Highway Research Record 211

"Volume Warrants for Left-Turn Storage Lanes At Unsignalized Grade Intersection"

M.D. Harmelink

PROJECT INFORMATION

TPD Project Number:	CECO.A.00008
Intersection:	Woodland Road and Western Site Driveway
Movement:	Left Turns into Western Site Driveway
Analysis Period:	2007 Projected Conditions, Friday P.M. Peak Hour
Analyst:	EMM

INPUTS

Advancing Volume (V_A) =	198	
Opposing Volume (V_O) =	268	
Number of Left Turns =	58	$t_w = 1.06$
Speed Limit =	40	$t_A = 18.18$
Proportion of Left Turns =	0.29	$\lambda = 10.01$

Headway range, sec	Percent Headways in each range		Opposing Volume	Number of Headways in Range	Average Headway, sec	Duration, sec
0-1	3	0	268	9.03	0.5	4.51
1-2	28	3	268	66.99	1.5	100.48
2-3	44	28	268	42.21	2.5	105.52
			Total	118.22		210.51

Unblocked Time = 3015.04

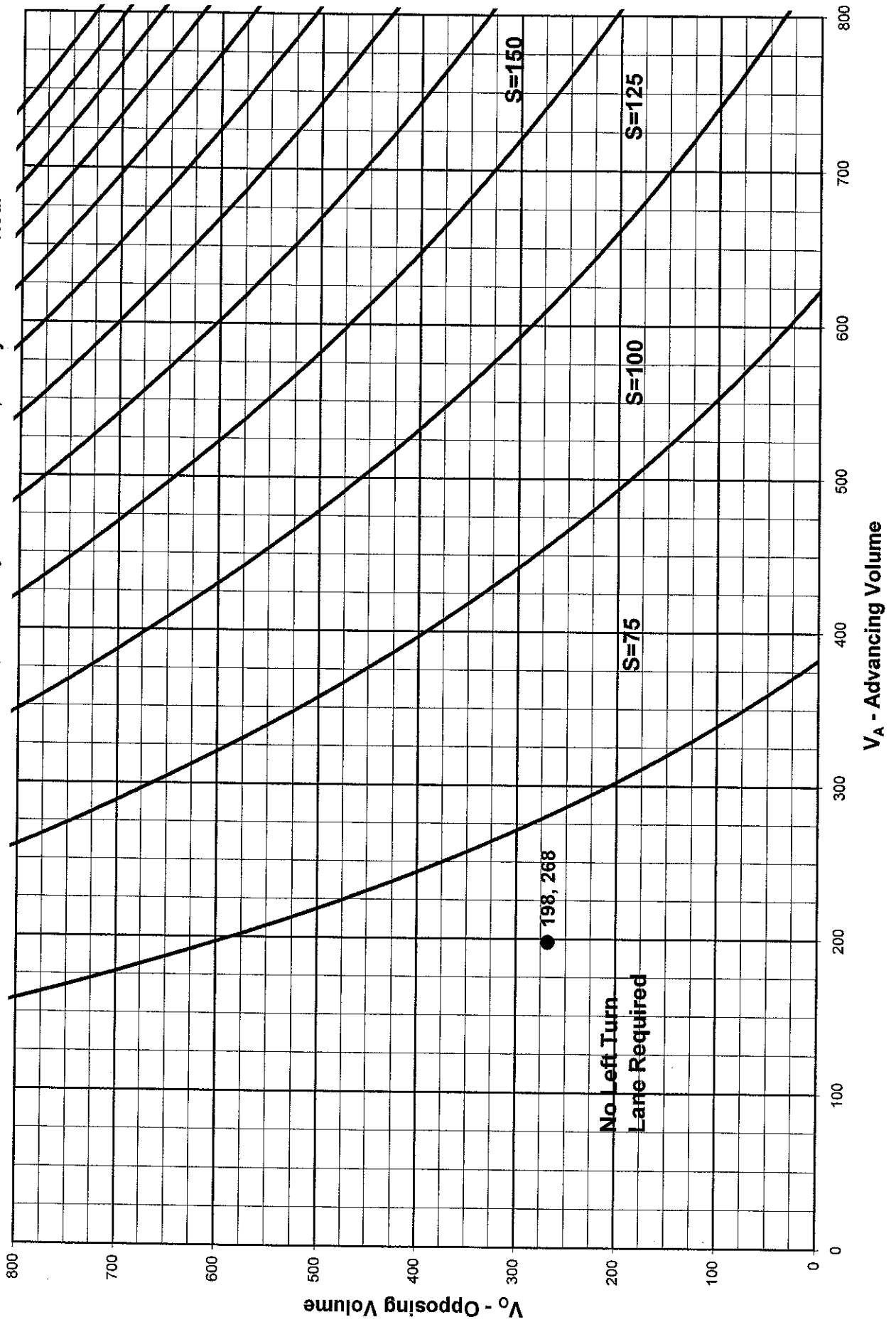
$\mu = 1005.01$

Left turn lane warrants are for $\rho >$ than
8.00E-06 for 40 mph

$\rho =$	0.0100
75 foot lane $\rho^3 =$	9.87E-07
100 foot lane $\rho^4 =$	9.83E-09
125 foot lane $\rho^5 =$	9.79E-11
150 foot lane $\rho^6 =$	9.74E-13
175 foot lane $\rho^7 =$	9.70E-15

Volume Warrants for Left Turn Storage Lanes at Unsignalized Grade Intersections $V = 40$ m.p.h.; $L = 29\%$

Woodland Road and Western Site Driveway, 2007 Projected Conditions, Friday P.M. Peak Hour



Unsignalized Intersection Left-Turn Storage Lane Warrant Analysis

Based on Highway Research Record 211

"Volume Warrants for Left-Turn Storage Lanes At Unsignalized Grade Intersection"

M.D. Harmelink

PROJECT INFORMATION

TPD Project Number:	CECO.A.00008
Intersection:	Woodland Road and Eastern Site Driveway
Movement:	Left Turns into Eastern Site Driveway
Analysis Period:	2007 Projected Conditions, Saturday P.M. Peak Hour
Analyst:	EMM

INPUTS

Advancing Volume (V_A) =	311	
Opposing Volume (V_O) =	616	
Number of Left Turns =	26	$t_w = 2.91$
Speed Limit =	40	$t_A = 11.58$
Proportion of Left Turns =	0.08	$\lambda = 14.84$

Headway range, sec	Percent Headways in each range		Opposing Volume	Number of Headways in Range	Average Headway, sec	Duration, sec
0-1	7	0	616	40.83	0.5	20.41
1-2	37	7	616	186.83	1.5	280.24
2-3	58	37	616	131.75	2.5	329.38
			Total	359.41		630.04

Unblocked Time = 2328.48

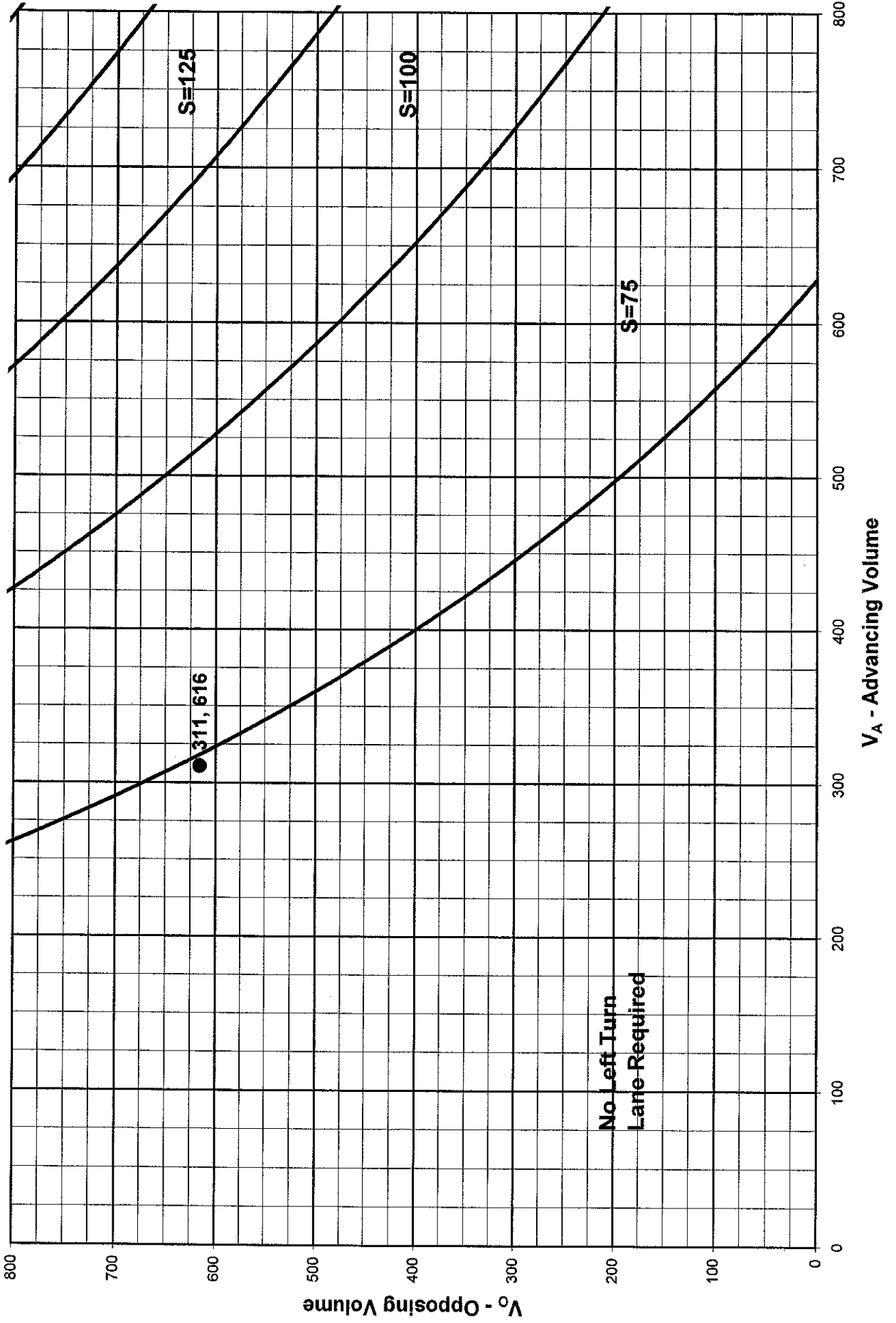
$\mu = 776.16$

Left turn lane warrants are for $\rho >$ than
8.00E-06 for 40 mph

$\rho =$	0.0191
75 foot lane $\rho^3 =$	6.98E-06
100 foot lane $\rho^4 =$	1.34E-07
125 foot lane $\rho^5 =$	2.55E-09
150 foot lane $\rho^6 =$	4.88E-11
175 foot lane $\rho^7 =$	9.32E-13

Volume Warrants for Left Turn Storage Lanes at Unsignalized Grade Intersections $V = 40$ m.p.h.; $L = 8\%$

Woodland Road and Eastern Site Driveway, 2007 Projected Conditions, Saturday P.M. Peak Hour



Unsignalized Intersection Left-Turn Storage Lane Warrant Analysis

Based on Highway Research Record 211

"Volume Warrants for Left-Turn Storage Lanes At Unsignalized Grade Intersection"

M.D. Harmelink

PROJECT INFORMATION

TPD Project Number:	CECO.A.00008
Intersection:	Woodland Road and Western Site Driveway
Movement:	Left Turns into Western Site Driveway
Analysis Period:	2007 Projected Conditions, Saturday P.M. Peak Hour
Analyst:	EMM

INPUTS

Advancing Volume (V_A) =	159	
Opposing Volume (V_O) =	243	
Number of Left Turns =	64	$t_w = 0.95$
Speed Limit =	40	$t_A = 22.64$
Proportion of Left Turns =	0.40	$\lambda = 7.21$

Headway range, sec	Percent Headways in each range		Opposing Volume	Number of Headways in Range	Average Headway, sec	Duration, sec
0-1	3	0	243	7.56	0.5	3.78
1-2	27	3	243	59.25	1.5	88.87
2-3	43	27	243	36.91	2.5	92.28
			Total	103.72		184.94

Unblocked Time = 3066.87

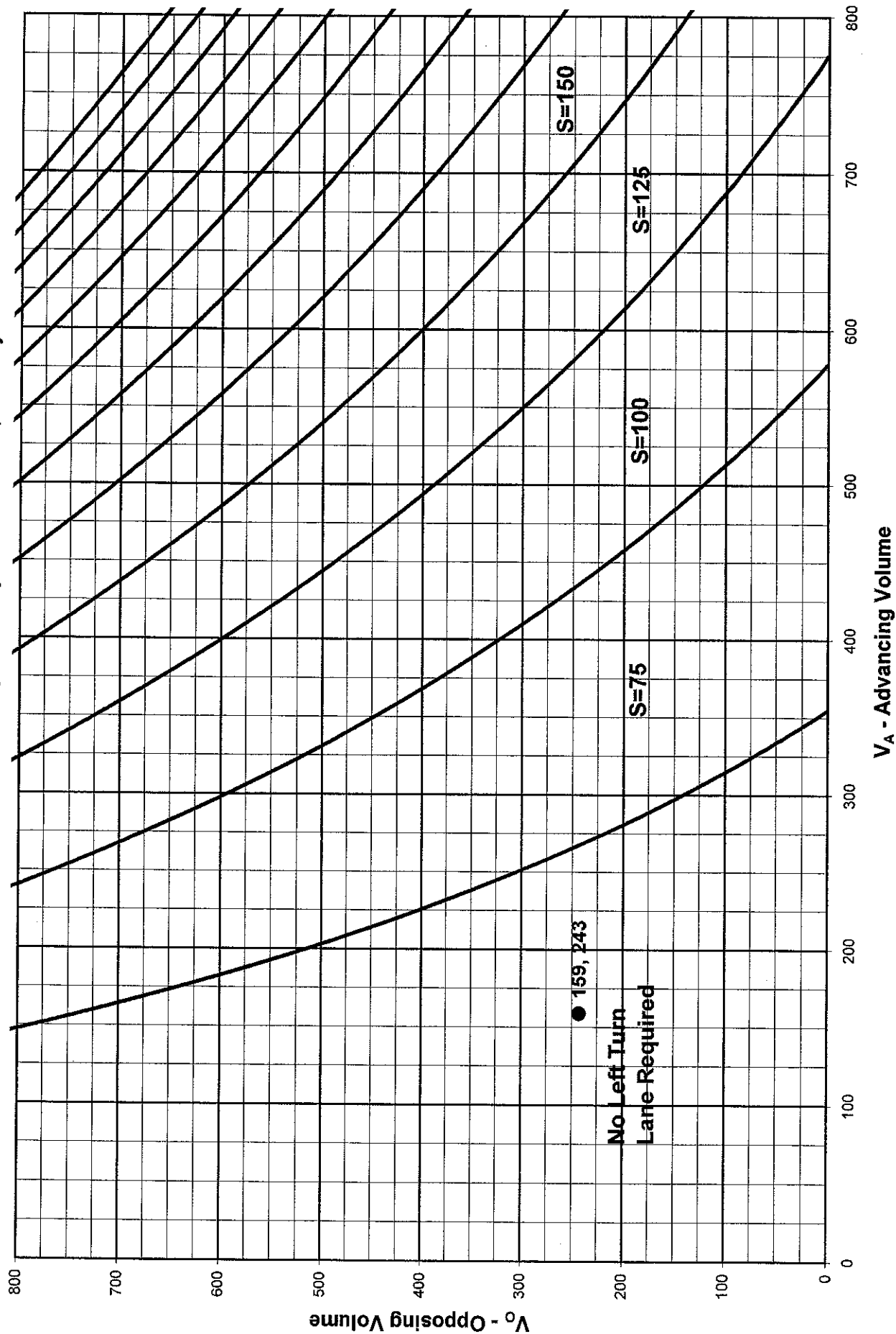
$\mu = 1022.29$

Left turn lane warrants are for $\rho >$ than
8.00E-06 for 40 mph

$\rho =$	0.0071
75 foot lane $\rho^3 =$	3.51E-07
100 foot lane $\rho^4 =$	2.48E-09
125 foot lane $\rho^5 =$	1.75E-11
150 foot lane $\rho^6 =$	1.23E-13
175 foot lane $\rho^7 =$	8.71E-16

Volume Warrants for Left Turn Storage Lanes at Unsignalized Grade Intersections $V = 40$ m.p.h.; $L = 40\%$

Woodland Road and Western Site Driveway, 2007 Projected Conditions, Saturday P.M. Peak Hour



Unsignalized Intersection Left-Turn Storage Lane Warrant Analysis

Based on Highway Research Record 211

"Volume Warrants for Left-Turn Storage Lanes At Unsignalized Grade Intersection"

M.D. Harmelink

PROJECT INFORMATION

TPD Project Number:	CECO.A.00008
Intersection:	Woodland Road and Eastern Site Driveway
Movement:	Left Turns into Eastern Site Driveway
Analysis Period:	2017 Projected Conditions, Friday P.M. Peak Hour
Analyst:	EMM

INPUTS

Advancing Volume (V_A) =	349	
Opposing Volume (V_O) =	641	
Number of Left Turns =	24	$t_w = 3.06$
Speed Limit =	40	$t_A = 10.32$
Proportion of Left Turns =	0.07	$\lambda = 16.13$

Headway range, sec	Percent Headways in each range		Opposing Volume	Number of Headways in Range	Average Headway, sec	Duration, sec
0-1	7	0	641	43.88	0.5	21.94
1-2	37	7	641	196.03	1.5	294.05
2-3	59	37	641	139.11	2.5	347.79
			Total	379.03		663.77

Unblocked Time = 2281.29

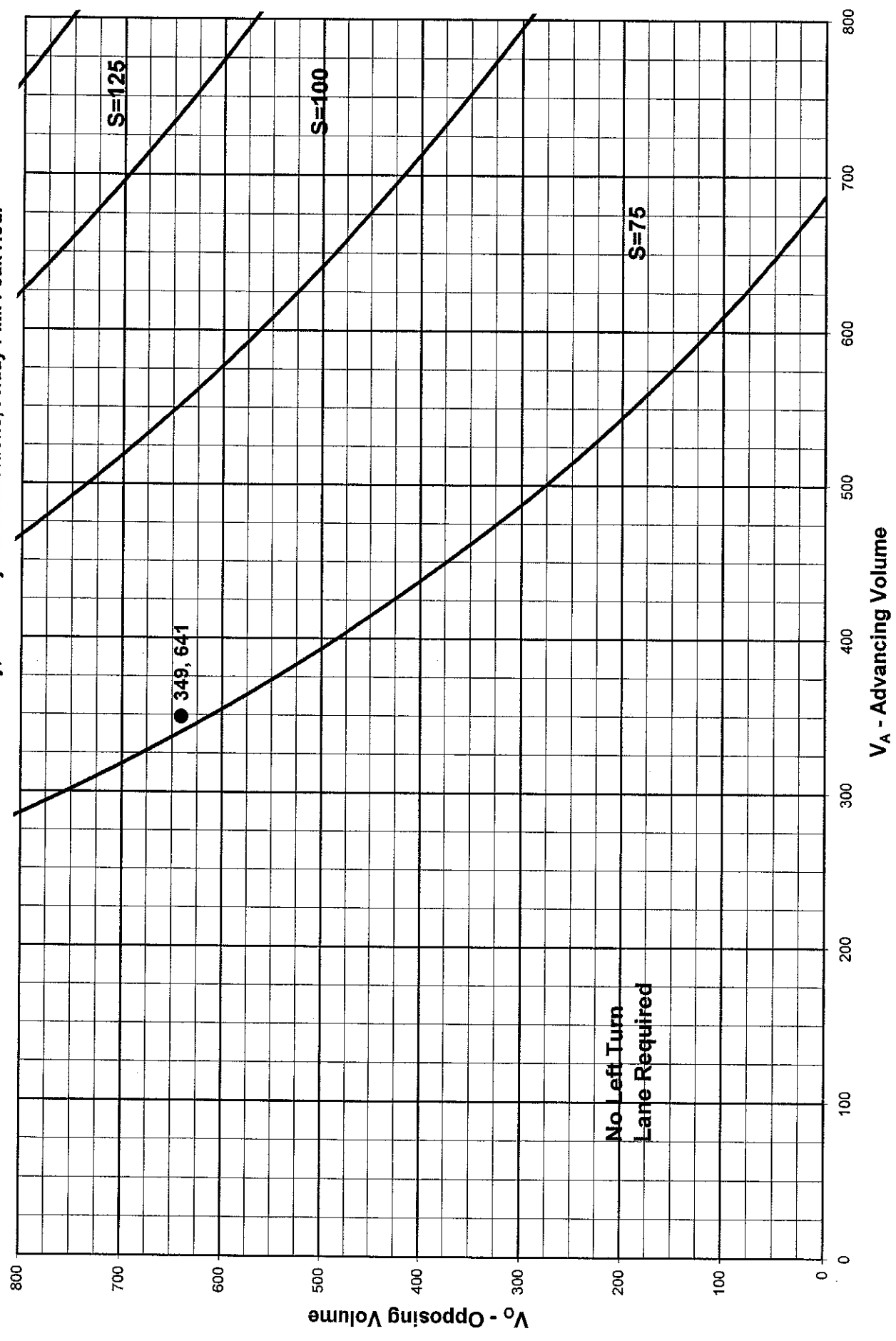
$\mu = 760.43$

Left turn lane warrants are for $\rho >$ than
8.00E-06 for 40 mph

$\rho =$	0.0212	
75 foot lane $\rho^3 =$	9.55E-06	satisfied
100 foot lane $\rho^4 =$	2.03E-07	
125 foot lane $\rho^5 =$	4.30E-09	
150 foot lane $\rho^6 =$	9.12E-11	
175 foot lane $\rho^7 =$	1.93E-12	

Volume Warrants for Left Turn Storage Lanes at Unsignalized Grade Intersections $V = 40$ m.p.h.; $L = 7\%$

Woodland Road and Eastern Site Driveway, 2017 Projected Conditions, Friday P.M. Peak Hour



Unsignalized Intersection Left-Turn Storage Lane Warrant Analysis

Based on Highway Research Record 211

"Volume Warrants for Left-Turn Storage Lanes At Unsignalized Grade Intersection"

M.D. Harmelink

PROJECT INFORMATION

TPD Project Number:	CECO.A.00008
Intersection:	Woodland Road and Western Site Driveway
Movement:	Left Turns into Western Site Driveway
Analysis Period:	2017 Projected Conditions, Friday P.M. Peak Hour
Analyst:	EMM

INPUTS

Advancing Volume (V_A) =	223	
Opposing Volume (V_O) =	291	
Number of Left Turns =	58	$t_w = 1.16$
Speed Limit =	40	$t_A = 16.14$
Proportion of Left Turns =	0.26	$\lambda = 12.21$

Headway range, sec	Percent Headways in each range		Opposing Volume	Number of Headways in Range	Average Headway, sec	Duration, sec
0-1	4	0	291	10.48	0.5	5.24
1-2	29	4	291	74.24	1.5	111.37
2-3	45	29	291	47.24	2.5	118.10
Total				131.97		234.71

Unblocked Time = 2967.71

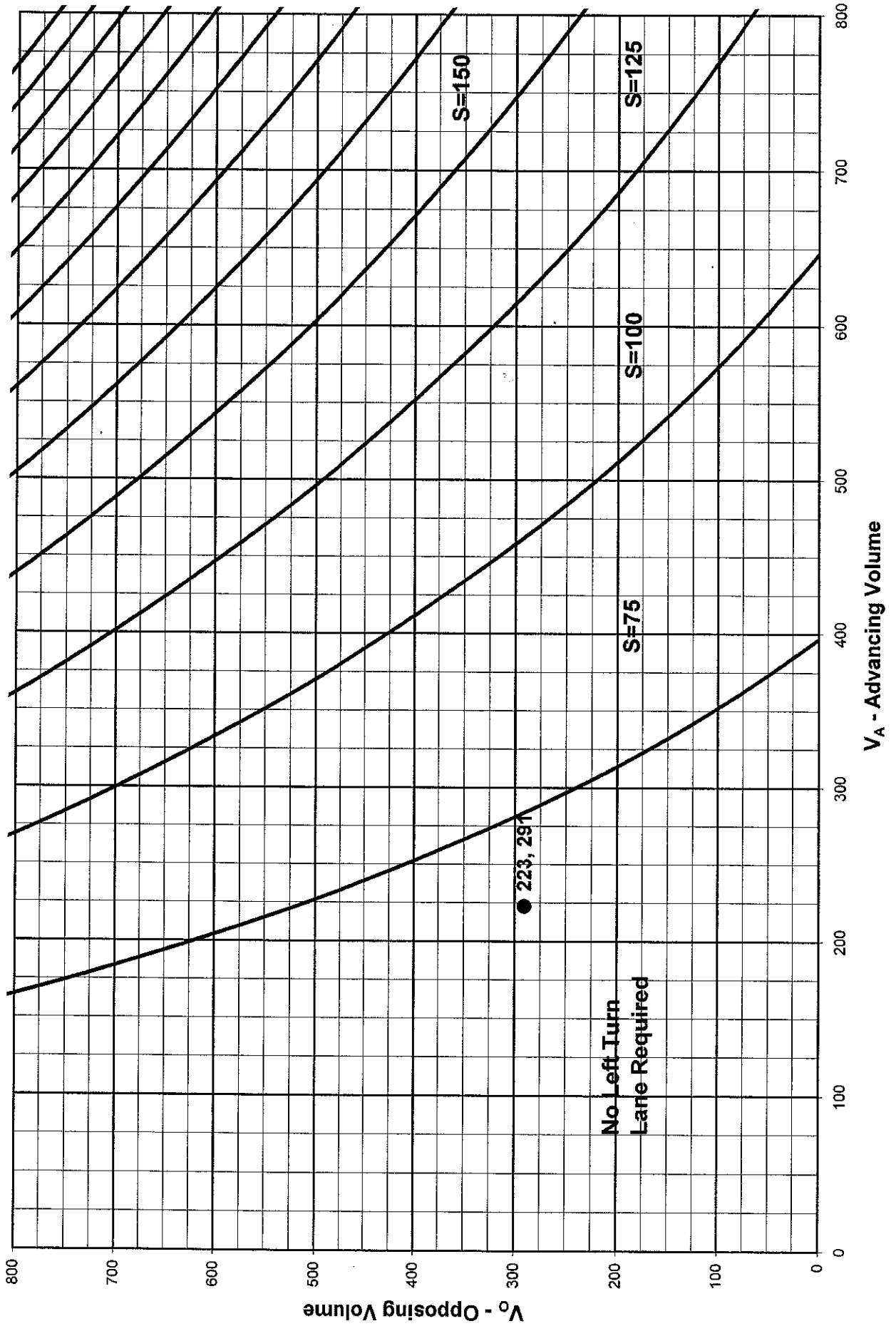
$\mu = 989.24$

Left turn lane warrants are for $\rho >$ than
8.00E-06 for 40 mph

$\rho =$	0.0123
75 foot lane $\rho^3 =$	1.88E-06
100 foot lane $\rho^4 =$	2.32E-08
125 foot lane $\rho^5 =$	2.86E-10
150 foot lane $\rho^6 =$	3.53E-12
175 foot lane $\rho^7 =$	4.36E-14

Volume Warrants for Left Turn Storage Lanes at Unsignalized Grade Intersections $V = 40$ m.p.h.; $L = 26\%$

Woodland Road and Western Site Driveway, 2017 Projected Conditions, Friday P.M. Peak Hour



Unsignalized Intersection Left-Turn Storage Lane Warrant Analysis

Based on Highway Research Record 211

"Volume Warrants for Left-Turn Storage Lanes At Unsignalized Grade Intersection"

M.D. Harmelink

PROJECT INFORMATION

TPD Project Number:	CECO.A.00008
Intersection:	Woodland Road and Eastern Site Driveway
Movement:	Left Turns into Eastern Site Driveway
Analysis Period:	2017 Projected Conditions, Saturday P.M. Peak Hour
Analyst:	EMM

INPUTS

Advancing Volume (V_A) =	327	
Opposing Volume (V_O) =	632	
Number of Left Turns =	26	$t_w = 3.01$
Speed Limit =	40	$t_A = 11.01$
Proportion of Left Turns =	0.08	$\lambda = 16.00$

Headway range, sec	Percent Headways in each range		Opposing Volume	Number of Headways in Range	Average Headway, sec	Duration, sec
0-1	7	0	632	42.77	0.5	21.38
1-2	37	7	632	192.71	1.5	289.07
2-3	59	37	632	136.45	2.5	341.13
			Total	371.93		651.58

Unblocked Time = 2298.25

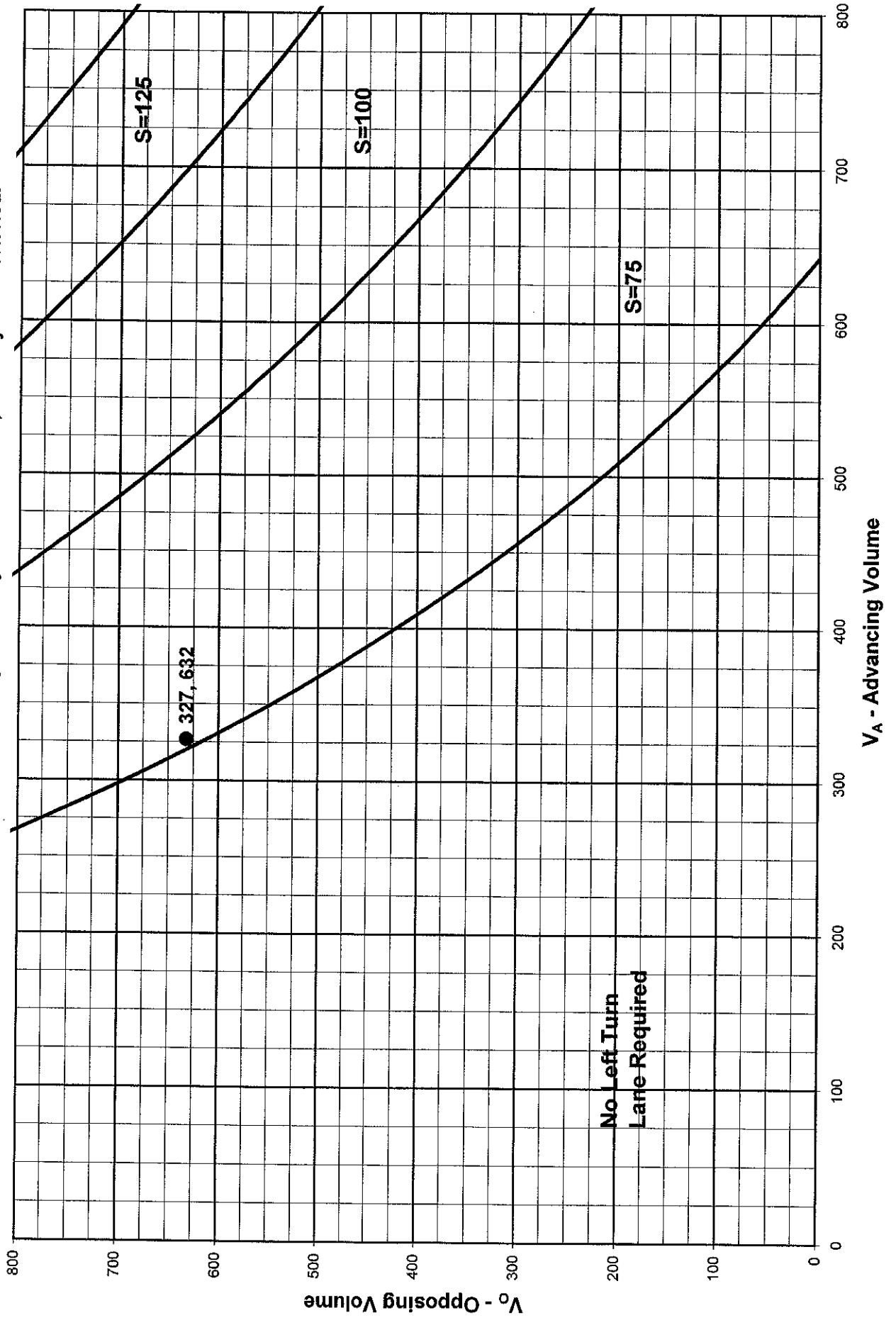
$\mu = 766.08$

Left turn lane warrants are for $\rho >$ than
8.00E-06 for 40 mph

$\rho =$	0.0209	
75 foot lane $\rho^3 =$	9.11E-06	satisfied
100 foot lane $\rho^4 =$	1.90E-07	
125 foot lane $\rho^5 =$	3.97E-09	
150 foot lane $\rho^6 =$	8.30E-11	
175 foot lane $\rho^7 =$	1.73E-12	

Volume Warrants for Left Turn Storage Lanes at Unsignalized Grade Intersections $V = 40$ m.p.h.; $L = 8\%$

Woodland Road and Eastern Site Driveway, 2017 Projected Conditions, Saturday P.M. Peak Hour



Unsignalized Intersection Left-Turn Storage Lane Warrant Analysis

Based on Highway Research Record 211

"Volume Warrants for Left-Turn Storage Lanes At Unsignalized Grade Intersection"

M.D. Harmelink

PROJECT INFORMATION

TPD Project Number:	CECO.A.00008
Intersection:	Woodland Road and Western Site Driveway
Movement:	Left Turns into Western Site Driveway
Analysis Period:	2017 Projected Conditions, Saturday P.M. Peak Hour
Analyst:	EMM

INPUTS

Advancing Volume (V_A) =	175	
Opposing Volume (V_O) =	259	
Number of Left Turns =	64	$t_w = 1.02$
Speed Limit =	40	$t_A = 20.57$
Proportion of Left Turns =	0.37	$\lambda = 8.64$

Headway range, sec	Percent Headways in each range		Opposing Volume	Number of Headways in Range	Average Headway, sec	Duration, sec
0-1	3	0	259	8.49	0.5	4.24
1-2	28	3	259	64.18	1.5	96.27
2-3	44	28	259	40.28	2.5	100.70
			Total	112.95		201.21

Unblocked Time = 3033.65

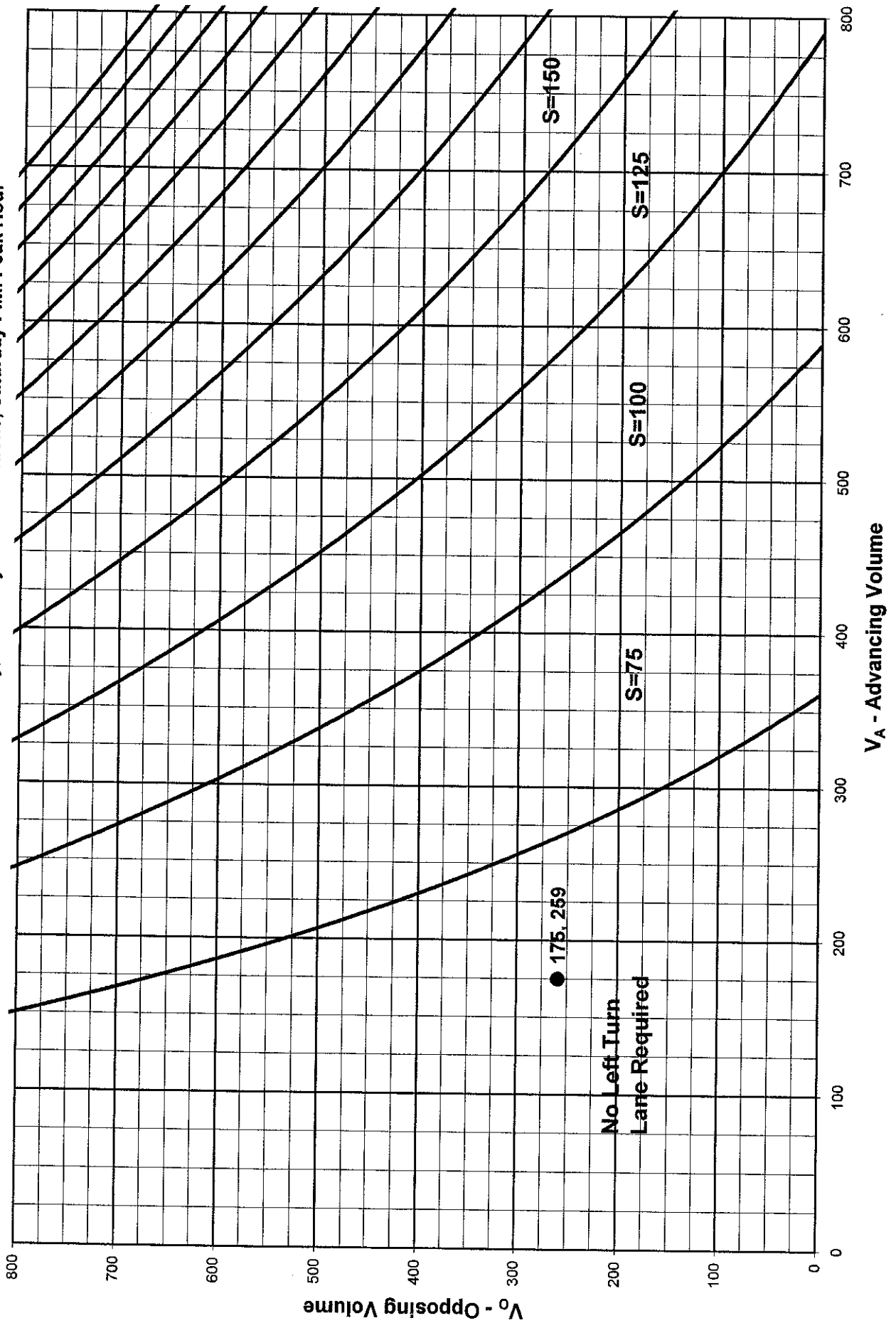
$\mu = 1011.22$

Left turn lane warrants are for $\rho >$ than
8.00E-06 for 40 mph

$\rho =$	0.0085
75 foot lane $\rho^3 =$	6.23E-07
100 foot lane $\rho^4 =$	5.32E-09
125 foot lane $\rho^5 =$	4.54E-11
150 foot lane $\rho^6 =$	3.88E-13
175 foot lane $\rho^7 =$	3.31E-15

Volume Warrants for Left Turn Storage Lanes at Unsignalized Grade Intersections $V = 40$ m.p.h.; $L = 37\%$

Woodland Road and Western Site Driveway, 2017 Projected Conditions, Saturday P.M. Peak Hour



RIGHT-TURN DECELERATION LANE WARRANTS

Guidelines for Right Turn Treatments

Two-Lane Highways

NCHRP Report 279

"Intersection Channelization Guide"

Project Number:

CECO.A.00008

Intersection:

Woodland Road & Eastern Site Driveway

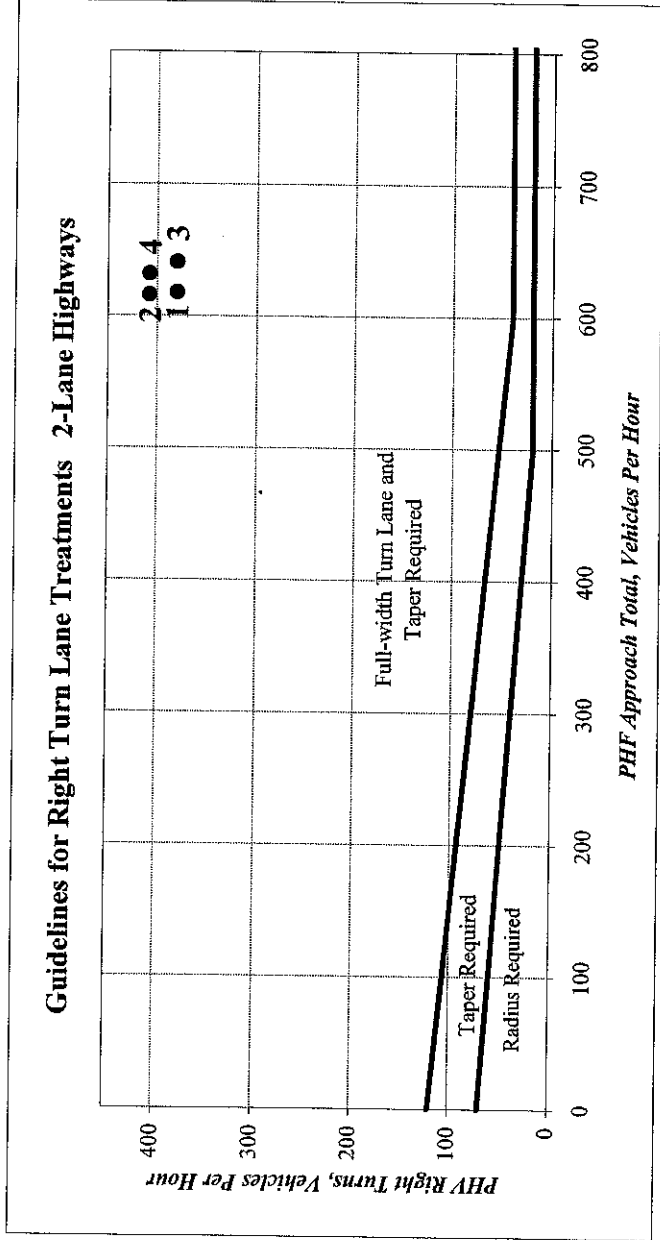
Movement:

Right Turns from Eastbound Woodland Road into Eastern Site Driveway

Analyst:

EMM

Condition	PHV Approach Total	PHV Right Turns	Taper Threshold	Full Lane Threshold	Treatment
1 2007 Projected FRI	618	380	20	40	Full-width Lane and Taper Required
2 2007 Projected SAT	616	408	20	40	Full-width Lane and Taper Required
3 2017 Projected FRI	641	380	20	40	Full-width Lane and Taper Required
4 2017 Projected SAT	632	408	20	40	Full-width Lane and Taper Required



Guidelines for Right Turn Treatments
Two-Lane Highways
 NCHRP Report 279
 "Intersection Channelization Guide"

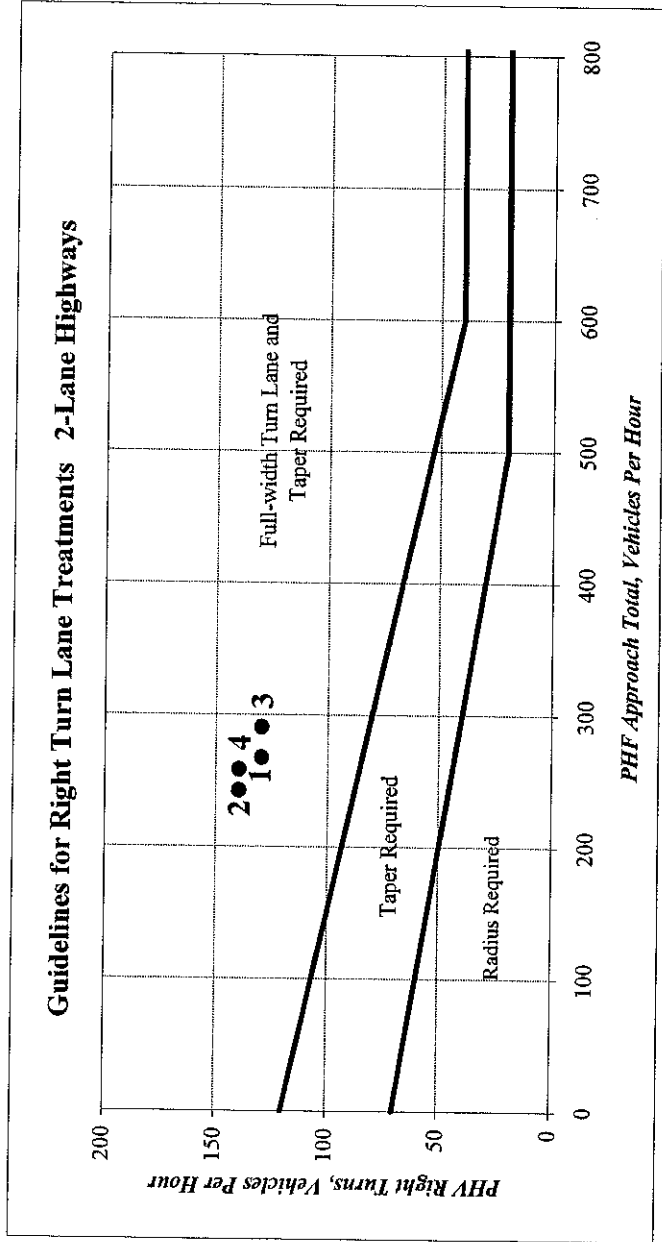
Project Number: CECO.A.00008

Intersection: Woodland Road & Western Site Driveway

Movement: Right Turns from Eastbound Woodland Road into Western Site Driveway

Analyst: EMM

Condition	PHV Approach Total	PHV Right Turns	Taper Threshold	Full Lane Threshold	Treatment
1 2007 Projected FRI	268	130	43	84	Full-width Lane and Taper Required
2 2007 Projected SAT	243	140	46	88	Full-width Lane and Taper Required
3 2017 Projected FRI	291	130	41	81	Full-width Lane and Taper Required
4 2017 Projected SAT	259	140	44	85	Full-width Lane and Taper Required



APPENDIX J
95TH PERCENTILE QUEUE LENGTH SUMMARY

TABLE J-1
95TH PERCENTILE QUEUE LENGTHS
FRIDAY P.M. PEAK HOUR

Intersection	Approach/ Movement		Storage Length	2005	2007	2007	2017	2017
				Existing	Base	Projected*	Base	Projected*
Route 611 & Route 314 (Eastern Leg)	WB	L	---	160'	200'	369'	247'	410'
	WB	R	72'	30'	45'	94'	61'	105'
	NB	T/R	---	459'	702'	941'	879'	1229'
	SB	L	175'	44'	59'	88'	78'	109'
	SB	T	---	142'	232'	265'	294'	391'
Route 611 & Route 314 (Western Leg)	EB	L	50'	31'	**	**	**	**
	EB	R	---	40'	96'	155'	200'	316'
	NB	L	143'	49'	118'	223'	302'	517'
Route 611 & Woodland Road/ Strickland's Road/ Big Daddy's Driveway	EB	L/T/R	---	25'	25'	25'	25'	25'
	WB	L	250'	119'	175'	163'	189'	174'
	WB	L/T	---			171'		184'
	WB	R	250'			33'		42'
	NB	L	73'	25'	25'	25'	25'	25'
	NB	T	---	245'	417'	285'	650'	405'
	NB	R	350'			25'		25'
	SB	L	183'	26'	35''	59'	46'	66'
	SB	T/R	---	148'	242'	167'	349'	210'
	SWB	L/T/R	---	25'	25'	---	25'	---
Route 611 & Meadowside Road/ Trinity Hill Road	EB	L/T/R	---	25'	25'	25'	25'	25'
	WB	L/T/R	---	25'	25'	25'	25'	32'
	NB	L	100'	25'	25'	25'	25'	25'
	SB	L	100'	25'	25'	25'	25'	25'
Route 611 & Grange Road/ Green Springs Driveway	EB	L/T/R	---	---	111'	136'	166'	**
	WB	L/T/R	---	25'	91'	111'	148'	164'
	NB	L	100'	25'	25'	25'	25'	25'
	SB	L	100'	25'	25'	25'	25'	25'

* = With Site-Related Recommendations

** = Unable to Calculate (No Capacity Available)

Existing Storage Length (Proposed Storage Length)

TABLE J-1 (CONTINUED)

Intersection	Approach/ Movement		Storage Length	2005	2007	2007	2017	2017
				Existing	Base	Projected*	Base	Projected*
Woodland Road & Pocono Mountain School Road	WB	L	100'	25'	25'	25'	25'	25'
	NB	L/R	---	25'	25'	40'	25'	60'
Woodland Road & Bowman Road	WB	L	100'	25'	25'	25'	25'	25'
	NB	L/R	---	25'	30'	41'	33'	43'
Woodland Road & Meadowside Road	EB	L	100'	25'	25'	25'	25'	25'
	SB	L/R	---	25'	25'	25'	25'	25'
Woodland Road & Carlton Road	EB	L/T	---	25'	25'	41'	31'	61'
	NB	L	---	25'	25'	25'	25'	25'
Route 940 & Carlton Road/ Caesar's Resort Driveway	EB	L	---	25'	314'	266'	439'	418'
	EB	T/R	---	---		201'	457'	66'
	WB	L	100'	25'	45'			
	WB	T/R	---	---	91'			
	NB	L/T/R	---	121'	84'	112'	100'	155'
	SB	L/T/R	---	25'	25'	25'	25'	25'
Route 940 & Route 390	EB	L	---	34'	328'	286'	531'	784'
	EB	T	---	---				
	WB	T/R	---	---	204	211'	241'	259'
	SB	L	50'	25'	52'	54'	54'	51'
	SB	R	---	67'				
Route 940 & Route 191/ Red Rock Road	EB	L/T/R	---	25'	25'	25'	25'	25'
	WB	L/T/R	---	25'	25'	25'	25'	25'
	NB	L/T/R	---	25'	25'	25'	25'	25'
	SB	L/T/R	---	25'	43'	46'	85'	91'
Woodland Road & Eastern Site Driveway	WB	L	100'	---	---	25'	---	25'
	NB	L	---	---	---	47'	---	53'
	NB	R	---	---	---	25'	---	25'
Woodland Road & Western Site Driveway	WB	L	100'	---	---	25'	---	25'
	NB	L	---	---	---	79'	---	91'
	NB	R	---	---	---	25'	---	25'

Existing Storage Length (Proposed Storage Length)

* = With Site-Related Recommendations

TABLE J-2
95TH PERCENTILE QUEUE LENGTHS
SATURDAY P.M. PEAK HOUR

Intersection	Approach/ Movement		Storage Length	2005	2007	2007	2017	2017
				Existing	Base	Projected*	Base	Projected*
Route 611 & Route 314 (Eastern Leg)	WB	L	---	72'	75'	169'	89'	211'
	WB	R	72'	25'	28'	38'	30'	43'
	NB	T/R	---	232'	443'	520'	590'	809'
	SB	L	175'	25'	25'	31'	26'	39'
	SB	T	---	75'	138'	148'	184'	180'
Route 611 & Route 314 (Western Leg)	EB	L	50'	25'	47'	**	90'	**
	EB	R	---	25'	28'	39'	43'	61'
	NB	L	143'	25'	25'	34'	36'	54'
Route 611 & Woodland Road/ Strickland's Road/ Big Daddy's Driveway	EB	L/T/R	---	25'	25'	25'	25'	25'
	WB	L	250'	61'	63'	163'	81'	171'
	WB	L/T	---			166'		175'
	WB	R	250'			26'		26'
	NB	L	73'	25'	25'	25'	25'	25'
	NB	T	---	203'	382'	290'	506'	410'
	NB	R	350'			25'		25'
	SB	L	183'	25'	25'	37'	25'	37'
	SB	T/R	---	100'	187'	161'	238'	193'
	SWB	L/T/R	---	25'	25'	---	25'	---
Route 611 & Meadowside Road/ Trinity Hill Road	EB	L/T/R	---	25'	25'	25'	25'	**
	WB	L/T/R	---	25'	60'	92'	131'	180'
	NB	L	100'	25'	25'	25'	25'	25'
	SB	L	100'	25'	25'	25'	25'	25'
Route 611 & Grange Road/ Green Springs Driveway	EB	L/T/R	---	---	94'	123'	147'	176'
	WB	L/T/R	---	25'	137'	163'	210'	233'
	NB	L	100'	---	25'	25'	25'	25'
	SB	L	100'	25'	25'	25'	25'	25'

* = With Site-Related Recommendations

** = Unable to Calculate (No Capacity Available)

Existing Storage Length (Proposed Storage Length)

TABLE J-2 (CONTINUED)

Intersection	Approach/ Movement		Storage Length	2005	2007	2007	2017	2017
				Existing	Base	Projected*	Base	Projected*
Woodland Road & Pocono Mountain School Road	WB	L	100'	25'	25'	25'	25'	25'
	NB	L/R	---	25'	25'	25'	25'	25'
Woodland Road & Bowman Road	WB	L	100'	25'	25'	25'	25'	25'
	NB	L/R	---	25'	25'	36	25'	40'
Woodland Road & Meadowside Road	EB	L	100'	25'	25'	25'	25'	25'
	SB	L/R	---	25'	25'	25'	25'	25'
Woodland Road & Carlton Road	EB	L/T	---	25'	25'	27'	25'	36'
	NB	L	---	25'	25'	25'	25'	25'
Route 940 & Carlton Road/ Caesar's Resort Driveway	EB	L	---	25'	224'	191'	297'	238'
	EB	T/R	---	---				
	WB	L	100'	25'	172'	41'	230'	49'
	WB	T/R	---	---		86'		108'
	NB	L/T/R	---	50'	33'	44'	35'	45'
SB	L/T/R	---	25'	25'	25'	25'	25'	
Route 940 & Route 390	EB	L	---	25'	183'	118'	307'	206'
	EB	T	---	---				
	WB	T/R	---	---	198'	153'	234'	184'
	SB	L	50'	25'	105'	75'	171'	154'
	SB	R	---	43'				
Route 940 & Route 191/ Red Rock Road	EB	L/T/R	---	25'	25'	25'	25'	25'
	WB	L/T/R	---	25'	25'	25'	25'	25'
	NB	L/T/R	---	25'	25'	25'	25'	25'
	SB	L/T/R	---	25'	49'	52'	94'	100'
Woodland Road & Eastern Site Driveway	WB	L	100'	---	---	25'	---	25'
	NB	L	---	---	---	49'	---	53'
	NB	R	---	---	---	25'	---	25'
Woodland Road & Western Site Driveway	WB	L	100'	---	---	25'	---	25'
	NB	L	---	---	---	83'	---	91'
	NB	R	---	---	---	25'	---	25'

Existing Storage Length (Proposed Storage Length)

* = With Site-Related Recommendations

APPENDIX K
SIGNAL DIAGRAMS

DISTRICT	COUNTY	ROUTE	SECTION	SHEET
5-0	MONROE	611	035	1 OF 1
POCONO TOWNSHIP				
SR 611 AND SR 314 WESTBOUND				
PERMIT NO. 45-209-12		SHEET 2 OF 2		
DATE ISSUED 9-18-91		DATE REVISED 10-21-04		

▲ CONDITION DIAGRAM ONLY
GENERAL NOTES

Installation, operation and maintenance of this traffic signal to be in accordance with Pennsylvania Department of Transportation Regulations on Official Traffic Control Devices.

No modifications of this installation are permitted unless prior approval is granted, in writing, by the Department.

All maintenance necessary for proper visibility of the signals, including trimming trees, is the responsibility of the Permittee.

All signs and pavement markings indicated on this drawing are considered part of the permit and are to be installed and maintained by the Permittee, unless otherwise indicated, except the longitudinal pavement markings on State highways which will be maintained by the Department.

Install post mounted signals with the signal heads a minimum of 2 feet behind the face of the curb or edge of the shoulder. Support poles for overhead signals will have a minimum horizontal clearance of 2 feet.

The bottom of signal heads and signs erected over the roadway are not to be less than 15 feet nor more than 19 feet above the roadway. The bottom of post mounted signal heads are to be not less than 8 feet nor more than 15 feet above the sidewalk or pavement grade.

The minimum horizontal distance between signal heads measured at right angles to the approach is to be 8 feet.

In addition to this signal permit, the Permittee will obtain a Highway Occupancy Permit prior to any openings being made in or under any portion of a State Highway.

This drawing cannot be used as a construction drawing unless the Permittee complies with the provisions of Act 172, Prevention of Damage to Underground Utilities. Prior to construction consult with utility companies to resolve any problems which may be created due to the location of utilities.

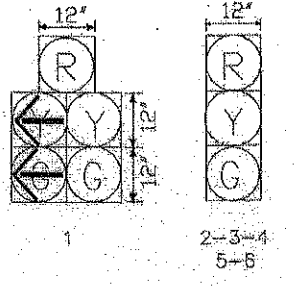
Place pavement markings in accordance with the Department of Transportation Pavement Marking Handbook.

PHASING, TIMING and COLOR SEQUENCE CHART

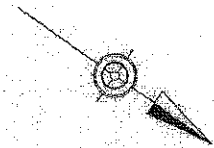
SIGNALS	PHASE 1		PHASE 2		PHASE 3		FLASHING OPERATION
	1	2	3	4	5	6	
1	G	Y	G	Y	R	R	Y
2	G	G	G	Y	R	R	Y
3,4	R	R	G	Y	R	R	Y
5,6	R	R	R	R	G	Y	R
FIXED		6	5	2	6	2	
MINIMUM SEC/ACT	0		5		0		
MAX INT. PASSAGE TO REDUCE BEFORE RED	2		6		2		
MIN. GAP			10				
MAXIMUM 1	7		22		15		
MAXIMUM 2	7		26		26		
MEMORY	NI		mR		NI		

① EMERGENCY FLASHING OPERATION

SIGNAL INDICATIONS



"POCONO TOWNSHIP"



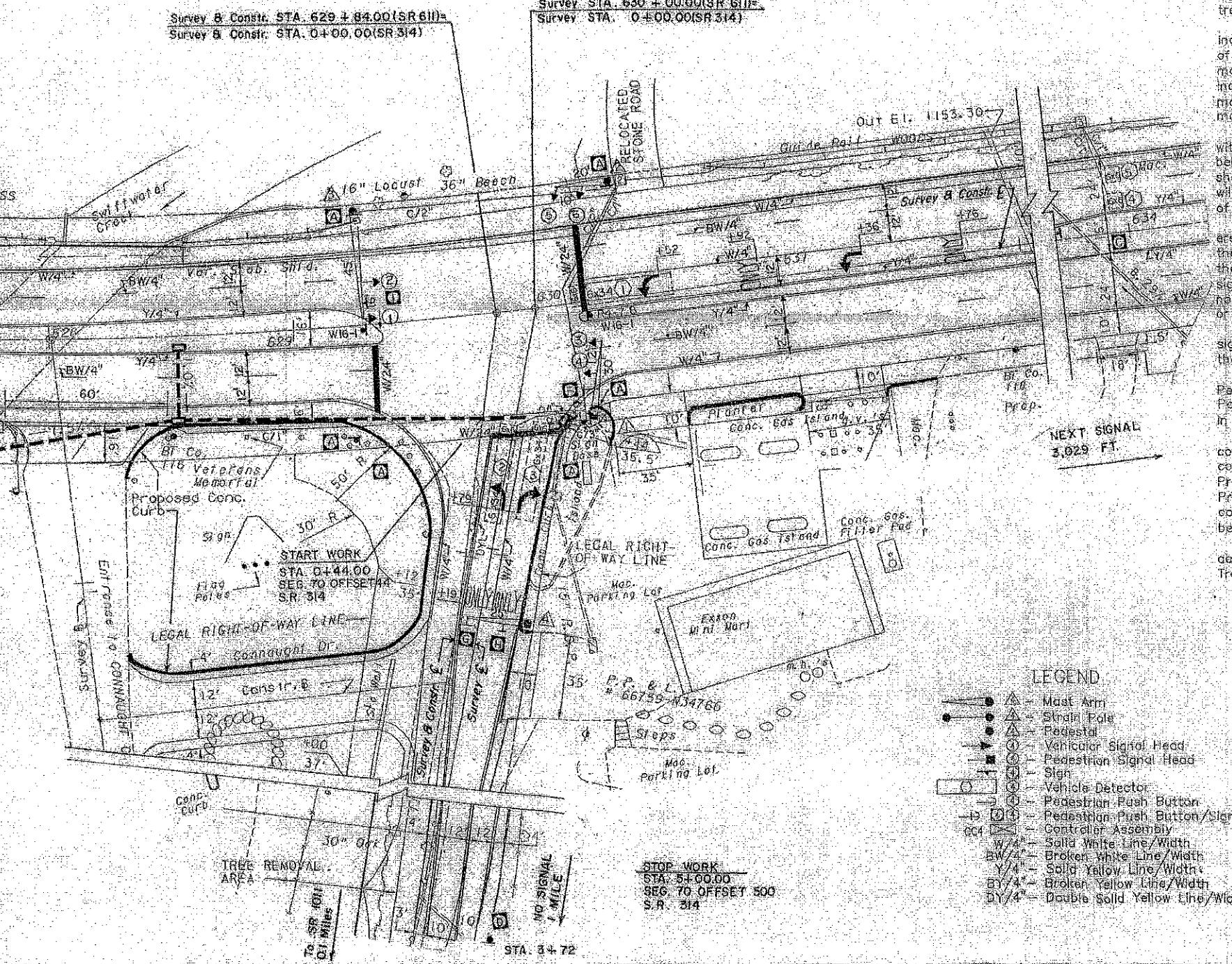
Survey & Constr. STA. 629 + 84.00 (SR 611)
Survey & Constr. STA. 0 + 00.00 (SR 314)

Survey STA. 630 + 00.00 (SR 611)
Survey STA. 0 + 00.00 (SR 314)

COORDINATION PROGRAM CHART

EVENT NO.	DAY OF WEEK	TIME	DIAL	CYCLE	OFFSET (SEC)	REMARKS
1	* * * * *	06:00:00	1	55		
2	* * * * *	07:15:00	2	80		AM PEAK
3	* * * * *	08:15:00	1	65		
4	* * * * *	14:30:00	2	80		PM PEAK
5	* * * * *	17:00:00	1	85		

PLAN SYMBOL	SERIES	SIZE	QTY.	MESSAGE
A	R9-3	16" x 16"	6	NO PEDESTRIAN CROSSING
C	R3-7L	30" x 30"	1	LEFT LANE MUST TURN LEFT
D	W3-3	16" x 36"	1	SIGNAL AHEAD
F	R10-11	30" x 36"	1	NO TURN ON RED 4:00 PM TO 5:00 PM
G	R3-5L	30" x 36"	1	LEFT TURN SYMBOL
H	R3-5R	30" x 36"	1	RIGHT TURN SYMBOL
I	R10-12	30" x 36"	1	LEFT TURN ON GREEN



- LEGEND
- ▲ Mast Arm
 - Strain Pole
 - Pedestal
 - Vehicle Signal Head
 - Pedestrian Signal Head
 - Sign
 - Vehicle Detector
 - Pedestrian Push Button
 - Pedestrian Push Button/Sign
 - Controller Assembly
 - W/4" Solid White Line/Width
 - BW/4" Broken White Line/Width
 - Y/4" Solid Yellow Line/Width
 - BY/4" Broken Yellow Line/Width
 - DY/4" Double Solid Yellow Line/Width

County: MONROE

Municipality: POCONO TOWNSHIP

Intersection: SR 611 AND SR 314 WESTBOUND

Reviewed: *[Signature]* 4/2/91
Municipal Official Date

Reviewed: *[Signature]* 4/2/91
District Traffic Signals Div. Date

Recommended: *[Signature]* shb
District Traffic Engineer Date

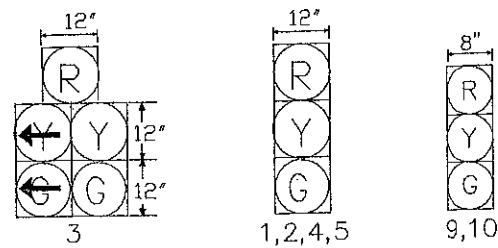
Scale: 1" = 25'

DISTRICT	COUNTY	ROUTE	SECTION	SHEET
5-0	MONROE	611	03S	1 OF 1
PARADISE TOWNSHIP				
SR 611, SR 1013 (WOODLAND ROAD) AND T-612 (STRICKLAND ROAD)				
PERMIT NO. 45-208-003	SHEET 2 OF 2			
DATE ISSUED 9-8-91	DATE REVISED 10/20/93			

PHASING, TIMING and COLOR SEQUENCE CHART

SIGNALS	PHASE 1		PHASE 2		PHASE 3		PHASE 4		FLASHER OPERATION
	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS	INTERVALS		
1,2	R	R	G	Y	R	R	R	R	Y
3	G	G	Y	R	R	R	R	R	Y
4	G	G	Y	R	R	R	R	R	Y
5,6	R	R	R	R	G	Y	R	R	R
7,8	R	R	R	R	R	R	G	Y	R
9,10	R	R	R	R	G	Y	R	R	R
FIXED TIME	5.5		5.5		2		4		2
MINIMUM	0		10		7		7		
SEC./ACT.	2		2						
MAX INITIAL PASSAGE	2		5		4		4		
TO REDUCE BEFORE RED	15		17						
MIN. GAP	3		3						
MAXIMUM 1	7		24		14		10		
MAXIMUM 2	7		27		28		13		
MAXIMUM 3	7		17				13		
MEMORY	NL		m R		NL		NL		

SIGNAL INDICATIONS



NOTE: SIGNALS # 3,4,7 & 8 TO HAVE TUNNEL VISORS AND LOUVERS

GENERAL NOTES

Installation, operation and maintenance of this traffic signal to be in accordance with Pennsylvania Department of Transportation Regulations on Official Traffic Control Devices.

No modifications of this installation are permitted unless prior approval is granted, in writing, by the Department.

All maintenance necessary for proper visibility of the signals, including trimming trees, is the responsibility of the Permittee.

All signs and pavement markings indicated on this drawing are considered part of the permit and are to be installed and maintained by the Permittee, unless otherwise indicated, except the longitudinal pavement markings on State highways which will be maintained by the Department.

Install post mounted signals with the signal heads a minimum of 2 feet behind the face of the curb or edge of the shoulder. Support poles for overhead signals will have a minimum horizontal clearance of 2 feet.

The bottom of signal heads and signs erected over the roadway are not to be less than 15 feet nor more than 19 feet above the roadway. The bottom of post mounted signal heads are to be not less than 8 feet nor more than 15 feet above the sidewalk or pavement grade.

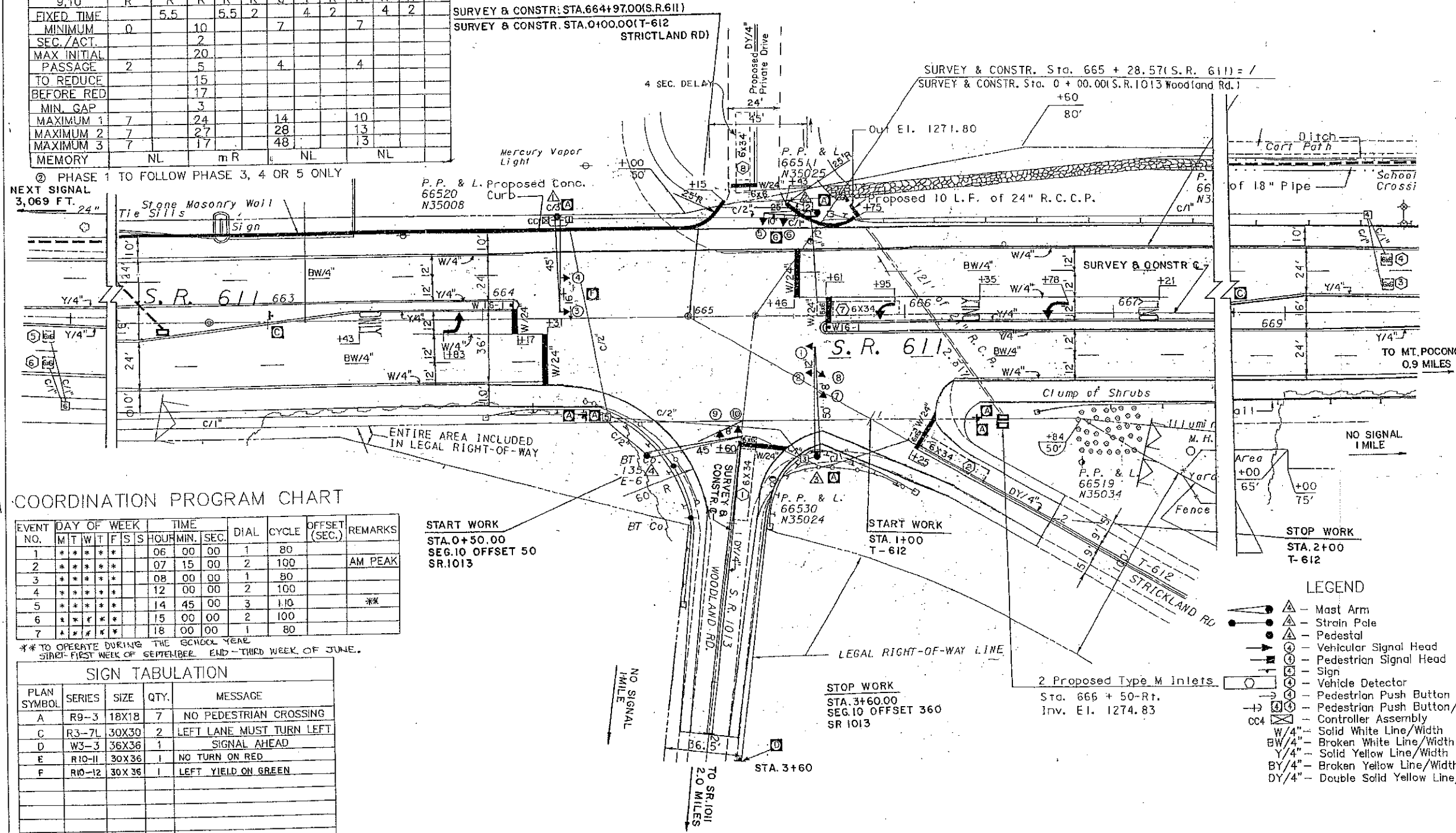
The minimum horizontal distance between signal heads measured at right angles to the approach is to be 8 feet.

In addition to this signal permit, the Permittee will obtain a Highway Occupancy Permit prior to any openings being made in or under any portion of a State Highway.

This drawing cannot be used as a construction drawing unless the Permittee complies with the provisions of Act 172, Prevention of Damage to Underground Utilities. Prior to construction consult with utility companies to resolve any problems which may be created due to the location of utilities.

Place pavement markings in accordance with the Department of Transportation Pavement Marking Handbook.

PHASE 1 TO FOLLOW PHASE 3, 4 OR 5 ONLY
NEXT SIGNAL 3,069 FT.



COORDINATION PROGRAM CHART

EVENT NO.	DAY OF WEEK					TIME		DIAL	CYCLE	OFFSET (SEC.)	REMARKS
	M	T	W	T	F	S	SA				
1	*	*	*	*	*	06	00	00	1	80	
2	*	*	*	*	*	07	15	00	2	100	AM PEAK
3	*	*	*	*	*	08	00	00	1	80	
4	*	*	*	*	*	12	00	00	2	100	
5	*	*	*	*	*	14	45	00	3	110	**
6	*	*	*	*	*	15	00	00	2	100	
7	*	*	*	*	*	18	00	00	1	80	

** TO OPERATE DURING THE SCHOOL YEAR
START- FIRST WEEK OF SEPTEMBER. END- THIRD WEEK OF JUNE.

PLAN SYMBOL	SERIES	SIZE	QTY.	MESSAGE
A	R9-3	18X18	7	NO PEDESTRIAN CROSSING
C	R3-7L	30X30	2	LEFT LANE MUST TURN LEFT
D	W3-3	36X36	1	SIGNAL AHEAD
E	R10-II	30X36	1	NO TURN ON RED
F	R10-12	30X36	1	LEFT YIELD ON GREEN

LEGEND

- ▲ - Mast Arm
- △ - Strain Pole
- - Pedestal
- ⊙ - Vehicular Signal Head
- ⊙ - Pedestrian Signal Head
- ⊙ - Sign
- ⊙ - Vehicle Detector
- ⊙ - Pedestrian Push Button
- ⊙ - Pedestrian Push Button/Sign
- ⊙ - Controller Assembly
- W/4" - Solid White Line/Width
- BW/4" - Broken White Line/Width
- Y/4" - Solid Yellow Line/Width
- BY/4" - Broken Yellow Line/Width
- DY/4" - Double Solid Yellow Line/Width

County: MONROE

Municipality: PARADISE TOWNSHIP

Intersection: SR 611, SR 1013 (WOODLAND ROAD) AND T-612 (STRICKLAND ROAD) & PRIVATE DRIVE

Reviewed: *John A. Bowman Jr.* 8-26-91
Municipal Official Date

Reviewed: *John A. Bowman Jr.* 9-1-91
District Traffic Signals Div. Date

Recommended: *David G. Long* 4/24/94
District Traffic Engineer Date

Scale: 25 0 25 50

3

PHASING, TIMING and COLOR SEQUENCE CHART

SIGNALS	PHASE 1+6			PHASE 2+6			PHASE 4+8			EMERGENCY FLASHING OPERATION	
	1	2	3	4	5	6	7	8	9		10
1,2	R	R	R	G	G	G	R	R	R	Y	Y
3	G	Y	R	G	G	G	R	R	R	Y	Y
4	R	R	R	G	G	G	R	R	R	Y	Y
5,6	R	R	R	G	G	G	R	R	R	Y	Y
7,8	H	H	H	H	H	H	H	H	H	OFF	OFF
9,10,11,12	H	H	H	H	H	H	H	H	H	OFF	OFF
FIXED	4	4	2	4	4	2	4	4	2	3,5,11,5	
MINIMUM INITIAL	4			10			6				
MAX INITIAL				2							
PASSAGE	3			5			3				
TIME TO REDUCTION				12							
TIME BEFORE REDUCTION				20							
MINIMUM GAP				3							
MAXIMUM	9			23			15				
PEDESTRIAN*	11			7	10		7	11			
MEMORY	NL			OR			NL				

* UPON PEDESTRIAN ACTUATION ONLY, OTHERWISE HAND AT ALL TIMES

OPERATION NOTES

① 1/2 IF FOLLOWED BY 2+6
 ② G IF FOLLOWED BY 2+6
 ③ DURING VOLUME DENSITY OPERATION, VOLUME DENSITY TIMINGS TO BEGIN AFTER SIDE STREET ACTUATION
 ④ PHASE 1+6 TO FOLLOW PHASE 4+8 ONLY

DISTRICT	COUNTY	ROUTE	SECTION	SHEET
5-0	MONROE	0940		
PARADISE TOWNSHIP				
PERMIT NO. 45-208-4		SHEET 2 OF 2		
DATE ISSUED		DATE REVISED		

GENERAL NOTES

Installation, operation and maintenance of this traffic signal to be in accordance with Pennsylvania Department of Transportation Regulations on Official Traffic Control Devices.

No modifications of this installation are permitted unless prior approval is granted, in writing, by the Department.

All maintenance necessary for proper visibility of the signals, including trimming trees, is the responsibility of the Permittee.

All signs and pavement markings indicated on this drawing are considered part of the permit and are to be installed and maintained by the Permittee, unless otherwise indicated, except the longitudinal pavement markings on State highways which will be maintained by the Department.

Install post mounted signals with the signal heads a minimum of 2 feet behind the face of the curb or edge of the shoulder. Support poles for overhead signals will have a minimum horizontal clearance of 2 feet.

The bottom of signal heads and signs erected over the roadway are not to be less than 15 feet nor more than 19 feet above the roadway. The bottom of post mounted signal heads are to be not less than 8 feet nor more than 15 feet above the sidewalk or pavement grade.

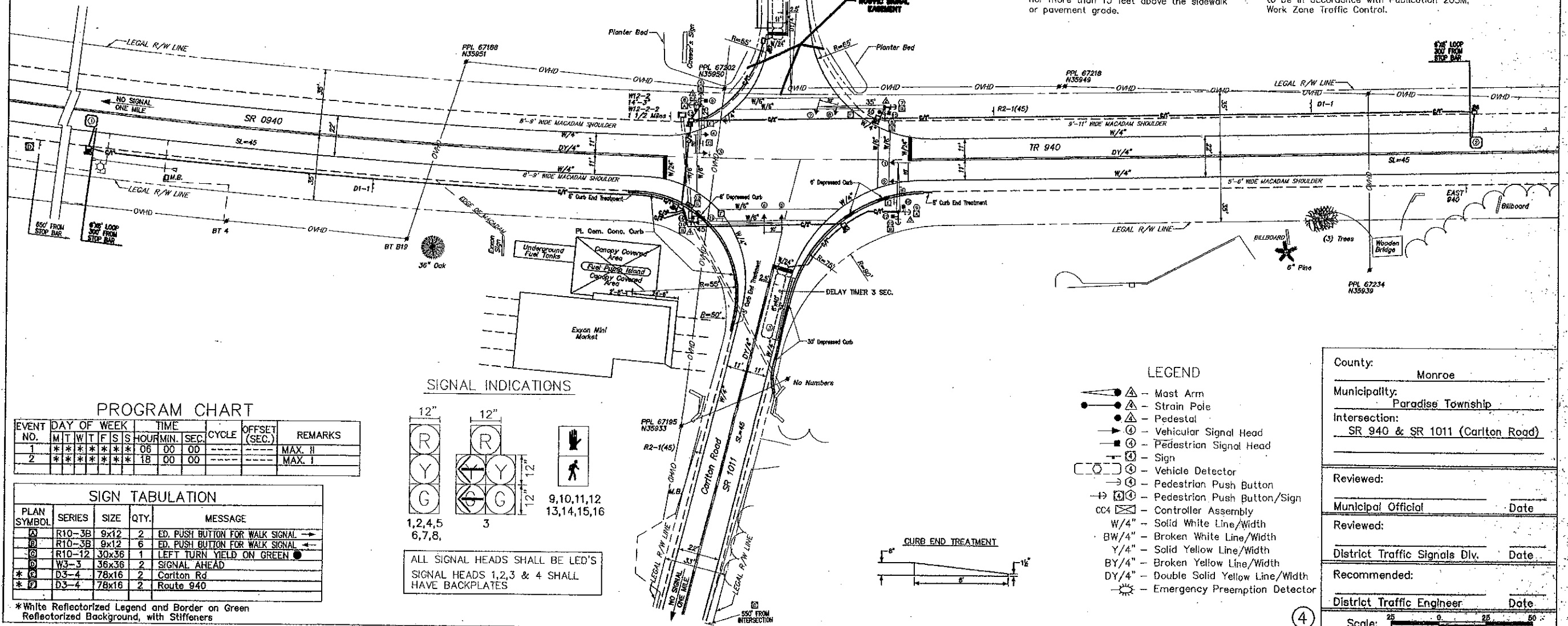
The minimum horizontal distance between signal heads measured at right angles to the approach is to be 8 feet.

In addition to this signal permit, the Permittee will obtain a Highway Occupancy Permit prior to any openings being made in or under any portion of a State Highway, if applicable.

This drawing cannot be used as a construction drawing unless the Permittee complies with the provisions of Act 187, Prevention of Damage to Underground Utilities. Prior to construction consult with utility companies to resolve any problems which may be created due to the location of utilities.

Place pavement markings in accordance with the Department of Transportation Pavement Marking Standards TC-7600 Series.

Maintenance and protection of traffic for the installation and maintenance of this traffic signal to be in accordance with Publication 203M, Work Zone Traffic Control.



PROGRAM CHART

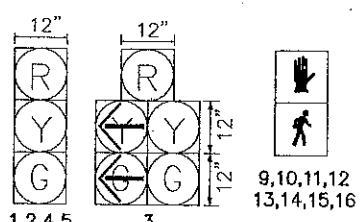
EVENT NO.	DAY OF WEEK	TIME	CYCLE	OFFSET (SEC.)	REMARKS
1	* * * * *	06 00 00	---	---	MAX. H
2	* * * * *	18 00 00	---	---	MAX. I

SIGN TABULATION

PLAN SYMBOL	SERIES	SIZE	QTY.	MESSAGE
ⓐ	R10-3B	9x12	2	ED. PUSH BUTTON FOR WALK SIGNAL →
ⓑ	R10-3B	9x12	6	ED. PUSH BUTTON FOR WALK SIGNAL ←
ⓒ	R10-12	30x36	1	LEFT TURN YIELD ON GREEN ●
ⓓ	W3-3	36x36	2	SIGNAL AHEAD
* ⓔ	D3-4	78x16	2	Carlton Rd
* ⓕ	D3-4	78x16	2	Route 940

*White Reflectorized Legend and Border on Green Reflectorized Background, with Stiffeners

SIGNAL INDICATIONS



LEGEND

- ▲ - Mast Arm
- △ - Strain Pole
- - Pedestal
- ⓐ - Vehicular Signal Head
- ⓑ - Pedestrian Signal Head
- ⓓ - Sign
- ⓔ - Vehicle Detector
- ⓕ - Pedestrian Push Button
- ⓖ - Pedestrian Push button/Sign
- CC4 - Controller Assembly
- W/4" - Solid White Line/Width
- BW/4" - Broken White Line/Width
- Y/4" - Solid Yellow Line/Width
- BY/4" - Broken Yellow Line/Width
- DY/4" - Double Solid Yellow Line/Width
- ☀ - Emergency Preemption Detector

County: Monroe

Municipality: Paradise Township

Intersection: SR 940 & SR 1011 (Carlton Road)

Reviewed: _____ Date _____

Municipal Official _____ Date _____

Reviewed: _____ Date _____

District Traffic Signals Div. _____ Date _____

Recommended: _____ Date _____

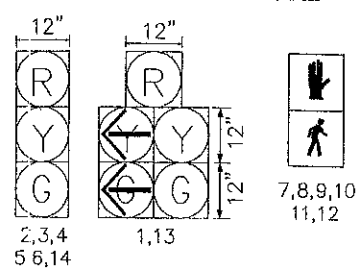
District Traffic Engineer _____ Date _____

Scale: 1" = 25'

PHASING, TIMING and COLOR SEQUENCE CHART

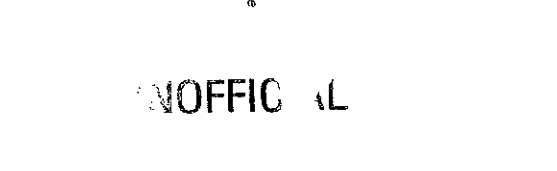
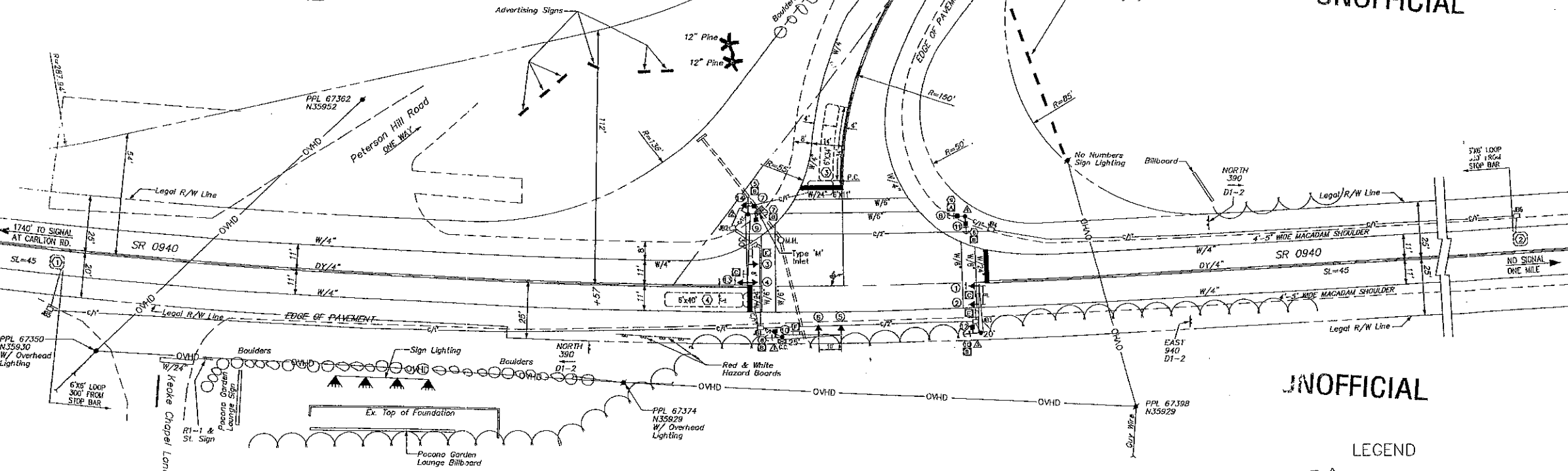
SIGNALS	PHASE 1+6			PHASE 2+6			PHASE 4+8		
	1	2	3	4	5	6	7	8	9
1,13	G	Y	R	G	C	Y	R	R	R
2,4	R	R	R	G	C	Y	R	R	R
3,7	R	R	R	G	C	Y	R	R	R
5,6,14	R	R	R	G	C	Y	R	R	R
7,8	H	H	H	M	F	H	H	H	H
9,10,11,12	H	H	H	M	F	H	H	H	H
EMERGENCY FLASHING OPERATION	OFF								
INTERVALS	4.5	1.5	10	4.5	1.5	7	4.5	1.5	3

SIGNAL INDICATIONS



A.L. SIGNAL HEADS SHALL BE LED'S
SIGNAL HEADS 1,2,3 & 4 SHALL
HAVE BACKPLATES

OPERATION NOTES
 ① G IF FOLLOWED BY 2+6
 ② G IF FOLLOWED BY 2+6
 ③ DURING VOLUME DENSITY
 OPERATION, VOLUME
 DENSITY TIMINGS TO BEGIN
 AFTER SIDE STREET
 ACTUATION
 ④ PHASE 1+6 TO FOLLOW
 PHASE 4+8 ONLY



SIGN TABULATION

PLAN SYMBOL	SERIES	SIZE	QTY.	MESSAGE
⊙	R10-3B	9x12	1	ED. PUSH BUTTON FOR WALK SIGNAL
⊙	R10-3B	9x12	5	ED. PUSH BUTTON FOR WALK SIGNAL
⊙	R10-12	30x36	2	LEFT TURN YIELD ON GREEN
⊙	W3-3	36x36	1	SIGNAL AHEAD
* ⊙	D3-4	78x16	2	Route 390
* ⊙	D3-4	78x16	1	Route 940

* White Reflectorized Legend and Border on Green Reflectorized Background, with Stiffeners

PROGRAM CHART

EVENT NO.	DAY OF WEEK	TIME	CYCLE	OFFSET (SEC.)	REMARKS
1	* * * * *	06 00 00	---	---	MAX. II
2	* * * * *	18 00 00	---	---	MAX. I

Code	Series	Size	Qty.	Message
D3-4	78x16	78x16	2	Route 390
D3-4	78x16	78x16	1	Route 940

- LEGEND
- ▲ - Mast Arm
 - - Strain Pole
 - ⊙ - Pedestal
 - ⊙ - Vehicular Signal Head
 - ⊙ - Pedestrian Signal Head
 - ⊙ - Sign
 - ⊙ - Vehicle Detector
 - ⊙ - Pedestrian Push Button
 - ⊙ - Pedestrian Push Button/Sign
 - CC4 - Controller Assembly
 - W/4 - Solid White Line/Width
 - BW/4 - Broken White Line/Width
 - Y/4 - Solid Yellow Line/Width
 - BY/4 - Broken Yellow Line/Width
 - DY/4 - Double Solid Yellow Line/Width
 - ☀ - Emergency Preemption Detector

DISTRICT	COUNTY	ROUTE	SECTION	SHEET
5-0	MONROE	0940		
PARADISE TOWNSHIP				
PERMIT NO. 45-208-5		SHEET 2 OF 2		
DATE UNOFFICIAL		DATE REVISED		

GENERAL NOTES

Installation, operation and maintenance of this traffic signal to be in accordance with Pennsylvania Department of Transportation Regulations on Official Traffic Control Devices.
 No modifications of this installation are permitted unless prior approval is granted, in writing, by the Department.
 All maintenance necessary for proper visibility of the signals, including trimming trees, is the responsibility of the Permittee.
 All signs and pavement markings indicated on this drawing are considered part of the permit and are to be installed and maintained by the Permittee, unless otherwise indicated, except the longitudinal pavement markings on State highways which will be maintained by the Department.
 Install post mounted signals with the signal heads a minimum of 2 feet behind the face of the curb or edge of the shoulder. Support poles for overhead signals will have a minimum horizontal clearance c. 2 feet.
 The bottom of signal heads and signs erected over the roadway are not to be less than 15 feet nor more than 19 feet above the roadway. The bottom of post mounted signal heads are to be not less than 8 feet nor more than 15 feet above the sidewalk or pavement grade.
 The minimum horizontal distance between signal heads measured at right angles to the approach is to be 8 feet.
 In addition to this signal permit, the Permittee will obtain a Highway Occupancy Permit prior to any openings being made in or under any portion of a State Highway, if applicable.
 This drawing cannot be used as a construction drawing unless the Permittee complies with the provisions of Act 187, Prevention of Damage to Underground Utilities. Prior to construction consult with utility companies to resolve any problems which may be created due to the location of utilities.
 Place pavement markings in accordance with the Department of Transportation Pavement Marking Standards TC-7600 Series.
 Maintenance and protection of traffic for the installation and maintenance of this traffic signal to be in accordance with Publication 203M, Work Zone Traffic Control.

County: Monroe

Municipality: Paradise Township

Intersection: SR 940 & SR 390

Reviewed: _____ Date _____

Municipal Engineer: _____ Date _____

Reviewed: _____ Date _____

District Traffic Signals Div. _____ Date _____

Recommended: _____

District Traffic Engineer: _____ Date _____

Scale: 25' = 1" (0' to 25')