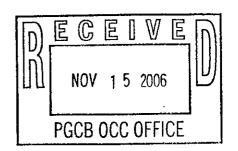


213 Market Street, 9th Floor, P.O. Box 865, Harrisburg, PA 17108-0865 Tel: (717) 237-7160 ■ Fax: (717) 237-7161 ■ www.WolfBlock.com

Direct Dial: (717) 237-7191 Direct Fax: (717) 237-2771

E-mail: mstewart@wolfblock.com



November 14, 2006

# **VIA HAND DELIVERY**

Frank T. Donaghue Chief Counsel Pennsylvania Gaming Control Board 5th Floor, Verizon Tower Strawberry Square Harrisburg, PA 17106-9060

Re: IOC Pittsburgh, Inc. – Traffic Study Report

Category 2 Slot Machine License Application;

Docket No. 1357

Dear Mr. Donaghue:

Enclosed please find the reply of IOC Pittsburgh, Inc.'s traffic expert, Trans Associates, to the comments of McCormick Taylor on IOC Pittsburgh's Pittsburgh First Master Plan Traffic and Parking Study and supplemental materials filed with the Pennsylvania Gaming Control Board at Supplement 5, Exhibit A. The response fully addresses, and we believe resolves, all comments made by McCormick Taylor. IOC Pittsburgh, Inc. and Trans Associates look forward to continuing to cooperate with McCormick Taylor, the City of Pittsburgh and the Board in considering the traffic issues related to its proposed Pittsburgh casino.

Notably, IOC Pittsburgh has learned from McCormick Taylor that it did not receive, and thus could not consider, IOC Pittsburgh and Trans Associates' response to its September 7, 2006 traffic study review of IOC Pittsburgh's traffic study. As a result, the comments from McCormick Taylor's earlier review reappear, unaltered, in its November 8, 2006 report to the Pennsylvania Department of Transportation. Accordingly, IOC Pittsburgh has again addressed and responded to these issues. We trust that this responsive information will be considered by McCormick Taylor in making its final comments to the Board.

HAR:69641.1/ISL005-221764

Frank T. Donaghue November 14, 2006 Page 2

Thank you for your consideration of our response. If you have any questions or require additional information, please feel free to contact me at your convenience.

Sincerely

Mark S. Stewart

For WOLF, BLOCK, SCHORR and SOLIS-COHEN LLP

MSS Enclosures

cc: Cynthia A. Jamople, P.E.

Richard Meister

Elizabeth Tranchina, Esq. Tami B. Steinberg, Esq.



November 14, 2006

Mr. Albert Federico, P.E., PTOE Senior Traffic Engineer McCormick Taylor, Inc. 75 Shannon Road Harrisburg, PA 17112

Subject: Response to McCormick Taylor Review of Traffic Study

Proposed Isle of Capri Casino City of Pittsburgh, Pennsylvania

Dear Mr. Federico:

Trans Associates (TA) has reviewed the comments from McCormick Taylor dated November 8, 2006 on our December 13, 2005 traffic study for the proposed Isle of Capri casino and Pittsburgh First Master Plan. In response to a September 7, 2006 review by McCormick Taylor, TA prepared a response in a letter dated October 11, 2005. We understand that this response was submitted to the Pennsylvania Gaming Board, but was not received by McCormick Taylor, and thus were not considered by McCormick Taylor in their most recent review. Accordingly, our responses below incorporate the responses in our previous letter, a copy of which is attached.

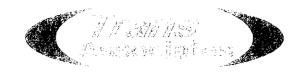
Following are McCormick Taylor's comments and findings for the initi0al review of the above-referenced submission, as well as TA's responses.

Comment 1. It would be appropriate for the engineer preparing this analysis to have stamped and signed the report.

The report is both signed and stamped by both Cynthia A. Jampole, P.E., Principal at Trans Associates, and Darryl C Phillips, P.E., PTOE, Senior Engineer at Trans Associates.

Comment 2. The study did not include evaluations of the new intersections internal to the site (e.g., Wylie Avenue and Mario Lemieux Place). An evaluation of these intersections is considered appropriate.

The exact components of this portion of the Master Plan, as well as square footages, land uses, driveway locations, and other parameters, were estimated for use in this analysis. As design



proceeds, details will be developed that allow for greater accuracy in trip projections. At that time, it will be appropriate to analyze the internal intersections.

Comment 3. Summary reports for the 2008 Saturday Casino Peak conditions capacity analysis was omitted from the technical appendix for numerous intersections, including but not limited to:

- Centre Avenue/Casino Exit
- Fifth Avenue/Washington Place
- Forbes Avenue/Armstrong Tunnels
- Grant Street/Boulevard of the Allies
- Grant Street/Fort Pitt Boulevard/I-376 Off-Ramp
- Second Street/Court Street/Ross Street

Copies of these pages from Appendix L are attached.

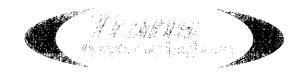
Comment 4. Numerous intersection counts were completed over the Thanksgiving Holiday weekend. Generally, data is not collected during holidays due to the atypical traffic patterns.

A.M. and P.M. peak hour counts were not conducted over the Thanksgiving holiday weekend. Only counts of evening/Saturdays were conducted, due to the short time window available for data collecting. Since this area is not typically active during evenings, background traffic variances were not expected to be significant.

Comment 5. The study does not indicate if seasonal or other adjustments were applied.

The City of Pittsburgh does not require or expect seasonal adjustment factors. As per Section 4.2.1, an annual traffic growth factor, obtained from the Southwestern Pennsylvania Commission, was used.

Comment 6. Due to lack of available data in ITE trip generation regarding gaming facilities, the trip generation estimates for the gaming facility were based upon patronage data as well as assumptions regarding mode split and vehicle occupancy provided by the Isle of Capri for a "similar" urban casino facility in Kansas City, Missouri.



This is the only urban casino data which we were able to obtain. In TA's opinion, it is important to use urban data, and there is essentially none of the type needed in the literature. It is also virtually impossible to obtain operational data from other casino operators. Therefore, TA used the Kansas City data for trip generation purposes and utilized very conservative travel mode assumptions.

Comment 7. The study does not clearly indicate if additional traffic was generated for the new Arena or if future Arena traffic is simulated by existing Arena capacity during the existing counts.

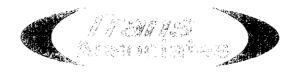
As documented in Section 4.1.1, trip generation for the new arena was calculated based on the parking supply assigned for arena patrons and staff. Counts of rates of parking entry and exit at the existing arena lots were performed during a maximum event to obtain trip rates. Base volume counts during the Friday and Saturday evening casino peak periods were conducted on non-event nights so that arena traffic would not need to be subtracted from base traffic. Parking assignments for arena executive staff, premium patrons, and patrons who would park in the study area are tabulated in Table 12 of the report. Trip generation to the parking used by these persons within the study area was performed on a per-space basis. The details are presented in the Technical Appendix to the report.

Comment 8. When comparing trip generation estimates for the gaming component of the three Pittsburgh gaming sites, the trip generation for Pittsburgh First is consistent with projections for Majestic Star Casino and higher than Station S quare.

Comment Acknowledged. The Pittsburgh First study used conservative assumptions to allow for variations in site-specific data and projections of traffic flow.

Comment 9. The 2008 Combined traffic volumes do not account for the effect of restricting the eastbound I-579 Off-Ramp left turns at the intersection of Centre Avenue/Washington Place/I-579 Off-Ramp.

As part of the proposed mitigations, a left-turn prohibition was proposed for eastbound Centre Avenue at the intersection with Washington Place and the I-579 Off-Ramp. "No left turn" restrictions were proposed for movements off the eastbound I-579 Off-Ramp, which operates as a separate, protected signal phase. Left turns from eastbound Centre Avenue were reassigned



via Centre Avenue, Bigelow Square, and Bigelow Boulevard to the intersection with Washington Place, Bedford Avenue, and the Bigelow Boulevard/I-579 On-Ramp.

Comment 10. This study does not include an evaluation of future conditions 10 years after the project build out, which is typically required by PennDOT.

The study was performed according to the requirements of the City of Pittsburgh Department of City Planning, which established the approved scope. DCP required analysis of existing and 2008 conditions both with and without the project.

Comment 11. The report identifies extensive queuing during the existing weekday evening peak hour period but does not provide an evaluation of vehicle queuing and the adequacy of existing and proposed turning bays at the study ar ea intersections.

In a similar fashion to existing conditions in the Pittsburgh CBD, and in most, if not all, other urban CBD areas, extensive queuing does exist during the afternoon rush hour period at the study intersections. This is a preexisting condition that is independent of the implementation of the casino project.

Comment 12. The capacity analysis did not address the effect of existing on-street parking or bus stops. Accounting for these items would be expected to reduce the capacity of study intersections resulting in less desirable operations.

The capacity analysis as shown in Appendices E, J, and L include the effect of on-street parking. Parking data was entered on an intersection- and approach-specific basis based upon our review of study area conditions. Parking is currently prohibited on a significant number of streets within the study area in order to accommodate traffic flows. Bus stop details were not included in the analysis. While this would slightly reduce intersection capacity, the same methodology was used for analysis of all scenarios, providing for a comparative analysis of existing and proposed conditions.

Comment 13. There are numerous inconsistencies between the traffic volumes presented in the exhibits and the volumes used for 2008 Build Conditions analysis, including but not limited to:



- The southbound right-turn volumes at Washington Place/Bedford Avenue during the morning peak.
- The northbound right-turn volumes at Grant Street/Sixth Avenue during the evening peak.
- The volumes for several movements at Sixth Avenue/Ross Street/Bigelow Boulevard during the evening peak.
- The eastbound left turn volumes at Washington Place/Bedford Avenue during the evening peak.
- The eastbound through volume at Washington Place/Centre Avneu/I-579 Off-Ramp during the evening peak.
- The eastbound through volume at Forbes Avenue/Armstrong Tunnel during the evening peak.

Upon review, we identified errors in a traffic reassignment calculation for Figure 30, showing 2008 combined traffic volumes for the weekday P.M. peak period. A revised Figure 30 is attached, along with revised HCS output. The HCS analysis was performed based upon the correct traffic volumes shown in the revised figure, and thus no changes in LOS occurred due to this revision.

Comment 14. The analysis of 2008 Build Condition analyses did not consistently assume the use of actuated traffic signals. This assumption should be verified with the City of Pittsburgh and PennDOT.

Consistent with City of Pittsburgh standard practice and existing conditions, pre-timed signal operation was assumed for analysis of most study area intersections. The City of Pittsburgh prefers pre-timed signal operation in the CBD and other areas with significant pedestrian flows. Actuated operation was assumed at the intersection of Bedford Avenue/Mario Lemieux Place which is currently actuated, and was proposed as a mitigation measure to accommodate varying traffic flows at the intersection of Centre Avenue and Washington Place.

Comment 15. The analysis of 2008 Build Conditions include permissive "Right Turn on Red" movements at locations (e.g. Centre Avenue Casino Access) where pedestrian activity may preclude these movements.



Right turn on red (RTOR) prohibitions used for analysis were based upon existing postings. For the proposed driveway intersection, it was assumed that adequate sight distance and low major-street volumes would permit the right turns on red to be made safely. The projected RTOR volumes are relatively low and are not anticipated to be precluded by pedestrian activity.

Comment 16. The analysis of Sixth Avenue/Ross Street/Bigelow Boulevard excluded the channelized southbound right turn movements. Including these movements is considered appropriate due to several factors, including the high volume of observed pedestrian activity and the lack of a dedicated receiving lane.

These intersections have been re-analyzed including the southbound tight turn movement. Capacity analysis is attached. Some changes in approach LOS occurred as a result of this change in analysis, but no change in intersection LOS was noted, and no movements failed due to this change in analysis methodology.

Comment 17. The location and operational interaction of the casino accesses to Centre Avenue may result in increased congestion, particularly if vehicles stopped at the egress queue past the entrance and possibly Logan Street. An analysis of vehicle queuing at the casino entrance and exit should be provided.

Queuing analysis was performed using the Synchro 6 analysis package, as shown in Appendix M. The maximum queue length of 112 feet projected for the eastbound through movement will extend to the site driveway and past Logan Street. However, a dedicated right turn lane is proposed for traffic entering the site, and a median will prohibit left turns into and out of Logan Street. Thus, the projected queue is not anticipated to interfere with either Logan Street traffic or traffic entering the site driveway.

Comment 18. The proposed mitigations for several locations are based on the "optimization" of traffic signal timings; however it does not appear as if the pre-development signal timings were optimized, which would be considered appropriate when assessing project impacts.

As noted in Section 4.2.4.1, optimized signal timings were used to analyze the 2008 base conditions.



Comment 19. The report recommends the installation of traffic signals at several intersections however, signal warrant analyses are not provided.

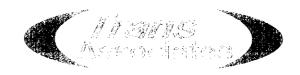
Only one new signal installation, on Centre Avenue at the exiting casino garage driveway, has been recommended. Signal warrant calculations for this location are contained in the Technical Appendix to the report. Additional signals may be proposed in the future as part of the Master Plan development. Full analysis including signal warrants, will be conducted as plans for that area progress.

Comment 20. The traffic impact for the majority of the study area intersections is either inconsequential or successfully mitigated by the proposed transportation program. However, the following intersections are projected to operate with deficient levels of service during at least two or more of the peak periods evaluated.

- Liberty Avenue/Eleventh Street
- Washington Place/Bedford Avenue
- Washington Place/Centre Avenue/I-579 Off-Ramp
- Grant Street/Fort Pitt Boulevard EB/I-376 Off Ramp
- Second Avenue/Court Place /Ross Street
- Grant Street/Sixth Avenue
- Centre Avenue/Mario Lemieux Place

Most of these intersections operate at poor levels of service currently in at least one of the time periods analyzed. This is not an unusual situation in downtown Pittsburgh and most urban central business districts. While LOS can be a useful tool to evaluate congestion impacts of development, in urban areas other measures must also be considered. In particular, queuing and presence or absence of gridlock reflect whether the roadway network can accommodate the projected traffic volumes. Poor LOS often cannot be mitigated without additional pavement, which would require removal of sidewalks, buildings or both, which is often not practical in urban areas. In such cases, the overall functionality of the network must be considered.

Comment 21. Except as noted above, it appears that the proposed improvements adequately mitigate the project impacts based on the results presented in the analysis. It should be noted that the omissions in the capacity analysis (as noted above) may be influencing the reported



results and the analyzed operation of the intersection. Additionally the inclusion of the evaluation of the 2018 design year may identify additional deficiencies requiring mitigation.

As noted above, revised analysis has been conducted in response to the comments from McCormick Taylor. This revised analysis did not significantly affect the findings or conclusions of our study.

The McCormack Taylor review letter also noted several issues raised by Mr. Sidney Kaikai of the City of Pittsburgh Department of City Planning. To address these issues, the following responses are provided:

The site has good regional access but local street traffic issues remain troublesome.

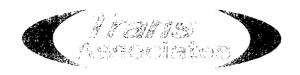
Our study demonstrates that the local roadway network is able to accommodate site traffic. Centre Avenue, Washington Place and the pair of Fifth Avenue and Forbes Avenue provide high-capacity roadways to connect the site to I-579, the Veterans Bridge, the Liberty Bridge, Bigelow Boulevard, the Boulevard of the Allies, and to downtown Pittsburgh. The developer will continue to work with the City and the adjacent communities to adequately mitigate any traffic issues on the local street network.

The loading area access on Fifth Avenue is problematic.

The proposed loading dock will be able to completely accommodate all site loading, including WB-67 trucks, completely internal to the site. The loading dock entrance will be at the signalized intersection of Fifth Avenue and Stevenson Street, and as shown in Figures 40 and 41 of the report, will be designed to safely accommodate all turning movements. Consistent with the City's truck route ordinance, we anticipate that essentially all truck traffic will enter the site via Forbes Avenue and Stevenson Street, and will exit the site via Fifth Avenue. Truck traffic within the site will be completely separated from automobile traffic.

The impact of traffic on local neighborhoods and Duquesne University are concerns.

The site driveways are located to provide for ready access to regional roadways, and to minimize traffic through the adjacent residential neighborhood. Our evaluation of site access



was based upon a review of regional roadways and the connections to the site on adjacent roadways, followed by a review of population by municipality and neighborhood. This analysis indicates that nearly all traffic will approach the site from the west, either directly from the regional highway network or through commercial sections of the downtown area and Uptown. Only 1.1 percent of site-generated traffic is anticipated to approach the site from the west, split between Bedford Avenue, Centre Avenue and Fifth and Forbes Avenues. Most of this traffic is anticipated to be patrons traveling to and from the adjacent neighborhoods.

A somewhat higher volume of site-generated traffic is anticipated to pass the borders of the Duquesne University campus on Stevenson Street, and on Forbes Avenue. These streets presently carry significant traffic volumes, and function as major arterials or connections from the regional highway system. Our recommendations include upgrading the traffic signal at the intersection of Forbes Avenue/Chatham Place/McAnulty Drive to include pedestrian signal heads to ensure safe pedestrian crossings at this location.

The developer will continue to work with the City of Pittsburgh, the adjacent community and Duquesne University to identify ways to satisfactorily mitigate any remaining traffic concerns.

Finally, the McCormick Taylor letter discusses a number of issues related to Highway Occupancy Permits (HOP) for the project. These issues, and our response, are as follows:

The project does not access a state highway, therefore an HOP is not required for the site driveways. Improvements proposed to mitigate project impacts do include modifications to state facilities, which would require an HOP. Additionally, PennDOT approval is typically required for the installation and/or modification of traffic signals and/or regional guide sighs.

As a city of the Second Class, the City of Pittsburgh generally has responsibility for installation and modifications to traffic signals. We do not anticipate that any of our recommended improvements will affect state facilities or require an HOP, with the possible exception of upgrades to the traffic signal at the intersection of Bedford Avenue/Mario Lemieux Place, and the I-579 HOV Ramp. This signal work may require coordination with PennDOT, but an HOP is not anticipated. Signage on local streets is anticipated to be included in the City's Wayfinder program. Regional signage on expressways and bridges, if appropriate, will follow PennDOT procedures.

The convergence of numerous regional routes and significant destinations within the vicinity of the Pittsburgh First site may present additional challenges to providing adequate destination signing for the proposed gaming facility.

The City's Wayfinder program is especially intended to provide for signing of complex routings and clustered destinations. It has been used successfully throughout the City, including the heavily-developed Downtown and Oakland areas. In addition, the construction of the proposed new arena immediately next to the site will provide a well-known regional landmark to assist in directing people to the site.

The study does not include an evaluation of future conditions 10 years after the project build-out date, which is typically required by PennDOT for a highway occupancy permit (HOP) submission.

As noted, none of the mitigations proposed for this project are anticipated to require an HOP. Should an HOP be necessary, additional analysis will be conducted of the future conditions to satisfy the HOP requirements.

Insufficient information was available to adequately assess the potential impacts of proposed improvement to existing utilities. However, transportation improvements within urban locations such as the proposed site typically require extensive utility coordination and relocation.

The roadway improvements for this project include the reconstruction of Centre Avenue from Washington Place to Crawford Street and of Washington Place from Centre Avenue to Fifth Avenue. It is anticipated that this work will require extensive utility coordination. Other improvements consist primarily of signage, pavement markings and signal upgrades. It is not anticipated that these improvements will have a significant utility impact.

To conclude, we believe that our responses address the concerns raised by McCormick Taylor, Inc., in their review of the Pittsburgh First Master Plan Traffic and Parking Study, dated December 13, 2005. In response to the comments received from McCormick Taylor, reanalysis was conducted of several intersections; however this did not result in any significant change in the findings of our study. We believe that our analysis accurately reflects the anticipated traffic



and parking impacts of the proposed casino, including accommodating traffic generated by a new arena as well as extensive adjacent residential and commercial development. We believe that our recommended plan will adequately mitigate the project impacts.

Respectfully submitted,

Cynthia A Jampole, P.E.

Principal

**Enclosure** 

CAJ:DCP:pah

cc: Alan Solomon - Isle of Capri

Les McMackin - Isle of Capri

Tami Bogutz Steinberg – WolfBlock

Mark Stewart - Wolf Block

File - iocap00/05380/McCormick Response 11.13.06



October 11, 2006

Mr. Richard Meister Isle of Capri Casinos 1641 Popps Ferry Road, Suite B1 Biloxi, Mississippi 39532

Subject: Pittsburgh First Master Plan Traffic and Parking Study

Response to McCormick Taylor Comments

Dear Mr. Meister:

The purpose of this letter is to provide Trans Associates' (TA's) responses to comments contained in the September 7, 2006 initial review letter prepared by McCormick Taylor and submitted to Mr. Glenn Rowe of the Pennsylvania Department of Transportation (PennDOT), Central Office.

The McCormick Taylor comments and TA's responses are detailed below, and are all in reference to the report entitled <u>Pittsburgh First Master Plan Traffic and parking Study</u> prepared by Trans Associates, Inc., dated December 13, 2005.

Following are McCormick Taylor's comments and findings for the initial review of the above-referenced submission, as well as TA's responses.

## <u>Approach</u>

Comment 1: The traffic assessment was completed in a manner generally consistent with

the accepted guidelines established by PennDOT and ITE, except as noted

within this review.

Response 1: Comment acknowledged. Scope was defined and approved by the City of

Pittsburgh Department of City Planning (DCP).

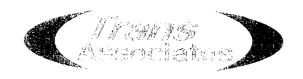
Comment 2: It would be appropriate for the engineer preparing this study to have stamped

and signed the report.

Response 2: The report is both signed and stamped by both Cynthia A. Jampole, P.E.,

Principal at Trans Associates, and Darryl C. Phillips, P.E. Senior Engineer at

Trans Associates, on the title page.



Comment 3:

The study indicates that a scoping meeting was held with the City staff to

establish the study area and analysis scenarios.

Response 3:

That is correct.

Comment 4:

The study included an evaluation of five peak periods: the weekday morning and evening, the Arena peak, Friday "Casino" peak and Saturday "Casino"

peak. The Arena and Casino peaks occur during the late-evening period.

Response 4:

That is correct.

Comment 5:

The study includes a discussion of available public transportation services and non-vehicular modes of travel and assumes that an appreciable portion of the

hotel and residential traffic will utilize these modes.

Response 5:

That is correct.

Comment 6:

The study did not include an evaluation of the new intersections internal to the

site (i.e. Wylie Avenue and Mario Lemieux Place). An evaluation of these

intersections is considered appropriate.

Response 6:

The exact components of this portion of the Master Plan, as well as square footages, land uses, driveway locations, and other parameters, were estimated for use in this analysis. As design proceeds, details will be developed that allow for greater accuracy in trip projections. At that time, it will be appropriate

to analyze the internal intersections.

### Data Collection

Comment 7:

All intersections were counted manually. The intersection turning movement traffic counts were conducted on the following dates:

- o Tuesday, November 15, 2005
- o Wednesday, November 16, 2005
- o Thursday, November 17, 2005
- o Saturday, November, 19, 2005
- o Tuesday, November 22, 2005
- o Friday, November 25, 2005
- o Tuesday, November 29, 2005
- o Friday, December 2, 2005
- o Saturday, December 3, 2005

The counted volumes were balanced between intersections.



Response 7: Comment acknowledged.

Comment 8: Numerous intersection counts were completed over the Thanksgiving Holiday

weekend. Generally data is not collected during holidays due to the atypical

traffic patterns.

Response 8: AM and PM peak hour counts were not conducted over the Thanksgiving

Holiday weekend. Only counts of evening/Saturdays were conducted then, due to the short time window available for data collecting. Since this area is not typically active during evenings, background traffic variances were not

expected to be significant.

Comment 9: Automatic count data was collected at fourteen locations around the perimeter

of the project area for two days in early November 2005.

Response 9: Comment acknowledged.

Comment 10: The study does not indicate if seasonal or other adjustments were applied.

Response 10: The City of Pittsburgh does not require or expect seasonal adjustment factors.

As per Section 4.2.1, an annual traffic growth factor, obtained from the

Southwestern Pennsylvania Commission, was used.

### **Trip Generation**

Comment 11: Due to the lack of available data in ITE Trip Generation regarding gaming

facilities, the trip generation estimates for the gaming facility were based upon patronage data as well as assumptions regarding mode split and vehicle occupancy provided by the Isle of Capri for a "similar" urban casino facility located in Kansas City, Missouri. A study based on a single site is considered

an insufficient sample size from which to base trip generation.

Response 11: This is the only *urban* casino data which we were able to obtain. In TA's

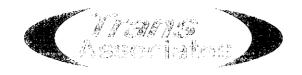
opinion, it is important to use urban data, and there is essentially none of the type needed in the literature. It is also virtually impossible to obtain operational data from other casino operators. Therefore, TA used the Kansas City data for trip generation purposes and utilized very conservative travel mode

assumptions.

Comment 12: The arrival patterns of the assumed gaming facility traffic were based on

information from the published article: Box, Paul C. and Bunte, William. "Gaming Casino Traffic." ITE Journal, Institute of Transportation Engineers,

March 1998.



Response 12: Comment acknowledged.

Comment 13: Trip generation for the hotel, residential, retail, and office components of the First Pittsburgh Master Plan were based on the data published by ITE in Trip

Generation, 7th Edition.

Response 13: Comment acknowledged.

Comment 14: The calculated trips for the Hotel and Residential components were reduced by

20% and 25% respectively to account for the urban setting of the development

and the availability of non-vehicular modes of travel.

Response 14: Comment acknowledged.

Comment 15: The study does not clearly indicate if additional traffic was generated for the

new Arena or if future Arena traffic is simulated by existing Arena traffic

captured during the existing counts.

Response 15: As documented in Section 4.1.1, trip generation for the new arena was

calculated based on the parking supply assigned for arena patrons and staff. Counts of rates of parking entry and exit at the existing arena lots were performed during a maximum event to obtain trip rates. Base volume counts during the Friday and Saturday evening casino peak periods were conducted on non-event nights so that arena traffic would not need to be subtracted from base traffic. Parking assignments for arena executive staff, premium patrons and patrons who would park in the study area are tabulated in Table 12. Trip generation to the parking used by these persons within the study area was performed on a per-space basis. The details are presented in the Technical

Appendix to the report.

Comment 16: Based on the information provided the applicant assumed that Pittsburgh First

Master Plan will generate 2,015 total trips during the weekday morning peak (1,096 from the gaming facility), 3,563 total trips during the weekday evening peak (2,456 from the gaming facility), 3,221 total trips during the \text{weekday} evening Arena peak (2,332 from the gaming facility), 4,557 total trips during the Friday evening Casino peak (3,851 from the gaming facility) and 4,212 total

trips during the Saturday Casino peak (3,558 from the gaming facility).

Response 16: Comment acknowledged.

Comment 17: The future volumes for the morning, evening and Arena peak periods were

adjusted (reduced) to account for the elimination of existing parking resulting

from the proposed development.



Response 17: Comment acknowledged.

Comment 18: The assumed trip distribution is based on county of origin information provided

by the Department of City Planning.

Response 18: Comment acknowledged.

Comment 19: The study included the evaluation of three conditions: 2004 existing, 2008 with

and without the proposed redevelopm ent.

Response 19: Comment acknowledged.

Comment 20: This study does not include an evaluation of future conditions 10 years after the

project build out, which is typically required by PennDOT.

Response 20: The study was performed according to the requirements of the City of

Pittsburgh Department of City Planning, which established the approved scope. DCP required analysis of existing, which was actually 2005, not 2004 conditions as listed in the comment, and 2008 conditions both with and without

the project.

Comment 21: An annual traffic growth factor of 0.5% per year was assumed based on

discussions with the Southwestern Pennsylvania Commission (SPC).

Response 21: Comment acknowledged. This was required in the DCP scope.

Comment 22: No other development projects in the vicinity of the site were identified based

on discussions with the Department of City Planning.

Response 22: Comment acknowledged.

Comment 23: The study included capacity and level-of-service analysis completed based on

the methodologies presented in the Highway Capacity Manual, using two types

of analysis software: HCS+, Version 5.2 and SYNCHRO Version 6.

Response 23: Comment acknowledged.

Comment 24: The report identifies extensive queuing during the Weekday Afternoon peak

hour period but does not evaluate the adequacy of existing and proposed

turning bays at the study area intersections.



Response 24:

In a similar fashion to existing conditions in the Pittsburgh CBD, and in most, if not all, other urban CBD areas, extensive queuing does exist during the afternoon rush hour period at the study intersections. This is a preexisting condition that is independent of the implementation of the casino project.

## Special Events and Opening Day Plans

Comment 25:

The report does not discuss special traffic control needs for opening day or

other special events.

Response 25:

Opening day is assumed to occur during 2008, the year of analysis. The analysis presented is conservative in that it includes traffic to be generated by ALL components of the master plan, all of which surely would not be in place by 2008. Special events traffic control is detailed in Section 8.4.

### Mitigation Measures

Comment 26:

The proposed mitigation for several locations is based on the "optimization" of traffic signal timings; however it does not appear as if the pre-development signal timings were optimized, which would be considered appropriate when assessing project impacts.

Response 26:

As noted in Section 4.2.4.1, optimized signal timings were used to analyze the 2008 base conditions.

Comment 27:

The analysis identifies unacceptable future operations at the following locations without presenting sufficient mitigation measures:

- o Grant Street & Liberty Avenue,
- o Liberty Avenue & Eleventh Street.
- o Grant Street & Seventh Avenue,
- o Grant Street & Sixth Avenue,
- o Sixth Avenue & Ross Street/Bigelow Boulevard,
- o Washington Place and Bedford Avenue/Bigelow Boulevard,
- o Bedford Avenue & Mario Lemieux Place,
- o Centre Avenue & Washington Place.
- o Centre Avenue & Crawford Street,
- o Fifth Avenue & Washington Place/Chatham Square,
- o Grant Street & Boulevard of the Allies/Court Place,
- o Grant Street & First Avenue.

Response 27:

Levels of service in an urban CBD routinely fall into the "unacceptable" categories of LOS E or F, which are measures of delay and congestion. In these types of locations, LOS is not a realistic measure of function, as virtually



all intersections in the CBD will function in these LOS categories. Queuing and presence or absence of gridlock are better indicators of conditions, while LOS is not really very meaningful. LOS cannot be mitigated without additional pavement, which would require removal of sidewalks, buildings or both, which is not practical.

Comment 28:

The study includes recommendations for improvements to the following intersections which were not analyzed as a part of this study:

- o Fifth Avenue & Magee Street/Site Driveway
- o Fifth Avenue & Stevenson Street/Loading Dock Driveway
- o Fifth Avenue & Pride Street
- o Forbes Avenue & Magee Street
- o Forbes Avenue & Stevenson Street
- o Forbes Avenue & Pride Street

Response 28:

These intersections were reviewed as the project design proceeded and additional intersection legs/driveways were determined to be added on Fifth Avenue at Magee and Stevenson Streets. These signals must be upgraded to include these changes. The upgrades are so extensive that the existing (old) signal equipment should be replaced. Pedestrian upgrades for ped equipment are also recommended. At the time of the scoping of the study, the access locations had not been determined, so the intersections were not identified for study at that time.

Comment 29:

The report recommends the installation of traffic signals at several intersections; however signal warrant analyses are not provided.

Response 29:

Only one new signal installation, on Centre Avenue at the exiting casino garage driveway, has been recommended. Signal warrant calculations for this location are contained in the Technical Appendix to the report.

This concludes TA's responses to McCormick Taylor's comments.

Sincerely,

Cynthia A. Jampole, P.E.

Principal

CAJ:pah

cc: File – iocap00/05380/Response to Comments of McCormick Taylor

	<u> </u>					S	HORT	REPO	ORT	·								
General Int	formation					Site	Inform	ation										
Analyst Agency or 0 Date Perfor Time Period	Co. <i>TRANS</i> med 1	3 AS 12/6/	2005					Area	section Type diction ysis Ye		CIT	NTRE A CBD or Y OF PIT OMBINED	Simila TSBL	ar JRG	SH .			
Volume and	d Timing Inpu	t						<u></u>										
			<u> </u>	_	EB	1 5		WE				NB				SB		
Number of L	2000		LT	-{-	TH 2	RT	LT	TH 2	l R	-	LT	TH	R		LT	TH	4	RT
Lane Group			┼┈	+	<u> </u>	+	<del>                                     </del>	$\frac{2}{T}$			1	+	0		<del> </del> -	<del> </del>	+	
Volume (vpt			+-	╁	150	+		221	<del></del>	+	L 058	LR	- 60		<u> </u>	┼—	+	
% Heavy Ve	· <u>·</u>		+-	╁	2	<del> </del>	+	2	+		2	<del> </del>	600	b .	<u> </u>	┿	+	
PHF			+	1	0.90	+-	+	0.90		<del></del>	90		0.90	2	<del> </del>	<del> </del>	+	<del></del>
Pretimed/Ac	tuated (P/A)		<del></del>	+	P	<del> </del>	<del> </del> -	P			. 90 P	┿—	P. P.			┼	┿	
Startup Lost	<del></del>		<del>                                     </del>	+	2.0	<del>                                     </del>	+	2.0			2.0	2.0	+			+-	+	
	Effective Gree	en	<del>                                     </del>		2.0	†	+	2.0	<del>-  </del>	-	.0	2.0	+		·	╀	+	
Arrival Type				+	3	<del>                                     </del>	<del>                                     </del>	3			3	3	╆			+-	+	
Unit Extension	on		†	† ;	3.0	<del> </del>	<del>                                     </del>	3.0			.0	3.0	┼─	┪		┼─	+	—
Ped/Bike/RT	OR Volume		0	1	0	<del>                                     </del>	124	0	╁┈	<del></del>	0	0	50	$\dashv$	<del></del>	<del> </del>	+	
Lane Width					11.0	<u> </u>		11.0			2.0	12.0	"			$\vdash$	╁	
Parking/Grad	rking/Grade/Parking				4	Y	N	-2	Y	17	V	0	N	Ť		<del> </del> -	十	
Parking/Hou				$\perp$		20			20							<u>†                                      </u>	丁	
Bus Stops/H			<u> </u>	<u> </u>	0	ļ		0			0	0						
	destrian Time	ī	<u> </u>	1.3	3.2		<u> </u>	14.0			·	3.2				<u></u>	$\perp$	
Phasing	Thru Only G = 20.0	G:	02		G =	03	G =	4	NB C		G	06 -	G	- 07 -	<u>,                                    </u>	G =	08	
Timing	Y = 5.5	Υ =	=		Y =		Y =		Y = 5		Ţ,		7			Y =		
	nalysis (hrs) =				<u>L</u>						Cy	cle Leng	th C =	- 9	0.0			
Lane Grou	ip Capacity	<u>, C</u> c	<u>ntrol</u>			and L	OS De		ation									
				E	B			WB		<u> </u>	,	NB		<u> </u>		SB		
Adjusted Flov	w Rate			167	7			246		670		1012		1	1			
ane Group (	Capacity			605	5			623		104	4	979		<u> </u>	$\neg \uparrow$	<del>-</del>		
/c Ratio				0.28	9 🗍			0.39	-	0.64	1	.03		1	一			
Green Ratio				0.22	2			0.22		0.66	0	.66	<u>-</u> -					
Jniform Delay	y d <sub>1</sub>			29.0	7			29.8		9.2	1	5.5	· · · · ·	t			$\vdash$	
elay Factor	k			0.50	,			0.50		0.50	- 0	.50		<b>†</b>	$\dashv$			
ncremental D	emental Delay d <sub>2</sub> 1.1				7	ĺ		1.9	_	3.0	1	37.8		T	_			
F Factor	<del></del>							1.000		1.00	0 1	.000		┢				
ontrol Delay								31.7		12.2	? ?	53.3	_		$\overline{}$	·	<u> </u>	
ane Group L	e Group LOS C							С		В	十	D		Т	$\neg \uparrow$			$\dashv$
pproach Del	roach Delay 30.1						·	31.7				36.9						$\dashv$
pproach LOS	oach LOS C							С				D		-				
	ction Delay 35.8																	!

Generated: 12/9/2005 11:33 PM

General In	formation							Sit	e Inforn	nation						
Analyst Agency or Date Perfor Time Perior	rmed 1	S AS 12/6/	KR SSOCIA /2005 ASINO PE					Are Juri	ersection a Type isdiction alysis Ye	VVA C	FIFT ASHINGT CBD C CITY OF P OB COMBIN	ON/ or Sil	CHAT milar SBUF	THAM RGH		
Volume an	d Timing Inpu	t	<del></del>						11/010 12		TO COMBIN	<i></i>	JOND	THON	=	
					В			WB			NB			<u> </u>	SE	<del></del>
Number of I	anoc		LT	╀	ſΉ	RT	LT	TH	_	-		耳	RT	LT	TH	
Lane Group			┼──	┼			0	2	1	_ 1	1				1	
			┼	╀	$\dashv$		<u> </u>	LT	R	L	T				T	F
Volume (vpi			┿-	+			8	1091	284	26	185	$\perp$		<u> </u>	64	17
% Heavy Ve	enicies		<del>                                     </del>	┼			4	4	4	0	0				3	3
	4t- 4.1544		┼─-	—			0.81	0.81	0.81	0.84	4 0.84				0.73	0.7
	tuated (P/A)		<del> </del>	ــــ			P	P	P	P	P	$\int$			Р	F
Startup Lost			<del> </del> -	$\vdash$				2.0	2.0	2.0	2.0				2.0	2.
	Effective Gree	:n	<u> </u>	<del>  -</del>			<b></b> _	2.0	2.0	2.0	2.0				2.0	2.
Arrival Type			<u> </u>	igspace				3	3	3	3	T			3	3
Unit Extension				ㄴ				3.0	3.0	3.0	3.0	丁			3.0	3.0
Ped/Bike/RT	OR Volume			L			17	0	28	0	0	十		12	0	0
Lane Width				<u> </u>	$\bot$			11.0	12.0	10.0	9.0	7			11.0	12.
Parking/Grad	<del></del>			<u> </u>	_		Υ	-2	Y	N	-6		N	N	-2	1 N
Parking/Hou					4		20		.20			$oldsymbol{oldsymbol{oldsymbol{oldsymbol{\Box}}}$				1
Bus Stops/Hi	our destrian Time		<b> </b>	<u> </u>	$\dashv$			0	0	0	0				0	0
Phasing	WB Only	<del>,                                    </del>	02	<del></del>	<u></u>	20		25.3	<u></u>	<u></u>	3.2				15.3	
	G = 46.0	G =		$\dashv$	G =	03	G =	04	NSP G = 2		06 G≂		<del> </del> _	07		08
iming	Y = 5.5	Y	<del></del> -		<u> </u>		Y =		Y = 5		G = Y=	—	G = Y =		G = Y =	
	nalysis (hrs) =			$\prod$							Cycle Ler	ngth			1, =	<del></del> -
ane Grou	p Capacity,	Co	ntrol			and L	OS De	etermi	nation			<del></del>				
		$\bot$	<del></del>	EB				WB			NB				SB	
djusted Flov	v Rate	$\perp$			$\bot$			1357	316	31	220	Γ			88	233
ane Group C	Capacity	$\perp$			$\perp$			1581	640	327	456				466	710
c Ratio	<del>-</del>	丄			$\perp$			0.86	0.49	0.09	0.48		$\dashv$		0.19	0.33
reen Ratio		上				]		0.57	0.57	0.29	0.29		$\neg$		0.29	0.29
niform Delay	/ d <sub>1</sub>	$\perp$						14.3	10.1	20.9	23.6		_		21.5	22.4
elay Factor I					丁			0.50	0.50	0.50	0.50		一		0.50	0.50
cremental D	elay d <sub>2</sub>	T			$\top$	$\neg \uparrow$	-	6.3	2.7	0.6	3.6		$\dashv$		0.9	1.2
F Factor					$\top$			1.000	1.000	1.000	1.000		$\dashv$		1.000	1.000
ontrol Delay		T			+			20.5	12.8	21.5	27.2		╬		22.4	23.7
ne Group Lo	os	7			十	$\overline{}$		С	В	C	C		$\dashv$		-	
proach Dela		1				$\dashv$		19.1	<u> </u>	<del>-</del>	26.5		$\dashv$		С	С
proach LOS	<u> </u>	+-		—				- 19. T B			20.5 C		$\dashv$		23.3	
. — — <del>-</del>		1						ن			C.				С	

0					<u> </u>	S	HORT	r R													
General In	formation			_	Site	Infor	nat	tion													
Analyst Agency or Date Perfo Time Perio	rmed	SAS	KR SOCIA 2005 SINO PE						Area Juris	section Type diction sis Ye	1	(	CIT	FORBI ISTRO CBD o Y OF P	NG r S. ITT	TUNI imilar SBUR	VEL GH	,			
Volume an	nd Timing Inpu	ıt	<del></del>					<u>l</u>	Allaly	515 1	ad I	- 4	000	COMBI	VED	COND	HON	! =====			
					ĒΒ				WB			т—		NB			<del>, -</del>		SB		
			LT	I	TH	RT	Lī	<u> </u>	TH	R	Τ	1	T	TH	7	RT	+	.T	TH		Ri
Number of	Lanes		<u> </u>	1	2	0			, i	Ĩ		0			7	2	†	<u> </u>		╅	<u></u>
Lane Group	<u> </u>				TR					1	-			LR	7	R	+		┝──	+	
Volume (vp	h)			T	442	205	1			1		43	}	$\dagger$	+	272	<del> </del>		<del>                                     </del>	+	
% Heavy Ve	ehicles				3	3	1	7		<del> </del>	_	1	_	┼	+	1	┿			+	
PHF				Ţ	0.88	0.88		寸		<del>                                     </del>		0.7	2	<u> </u>	+	0.72	╁╴	-		+	
Pretimed/Ad	ctuated (P/A)			T	P	P	<del>                                     </del>	7				P			╅	P	╁	$\dashv$	<b></b>	+	
Startup Lost	t Time			T	2.0		$\top$	7		+		<del>                                     </del>		2.0	+	2.0	╁╌			+-	—
Extension o	f Effective Gree	en		1	2.0		1	$\forall$		_	_		_	2.0	+	2.0	$\vdash$			╀	
Arrival Type				T	3		1	+		<del></del>				3	╬	3	╁			+	
Unit Extensi	on			†	3.0		<del>                                     </del>	$\dagger$		<del> </del> -			_	3.0	+	3.0		$\dashv$		+	
Ped/Bike/RT	TOR Volume		50	Ť	0	0	<del>                                     </del>	+		+	-	0	_	0.0	+	16		-		╄	
ane Width	ne Width			1	11.0		╁	十	<del></del>	+	┪		_	11.0	╁	11.0	-	<del>-  </del>		╁-	
Parking/Grad	rking/Grade/Parking				3	Ν		†		†	┪	N	$\exists$	0	╁	N		$\dashv$		╀	—
Parking/Hou								T			┪		٦		t			$\dashv$		┢	
Bus Stops/H					0					1	7		╗	0	+	0	_	十	<del>-</del>	+	
	destrian Time			1	7.3				·		1			3.2	T			$\top$		╁╌	
hasing	EB Only	<del> </del> _	02		0	3		4		NB (				06			7	一	0	8	
iming	G = 41.0 $Y = 5$	G =			G = Y =		G = Y =			G = 2 Y = 5		2	G : Y =			G =			G=		
uration of A	nalysis (hrs) =									1 - 2	_			cle Len	oth	Y =	20.0		Υ=		
ane Grou	ıp Capacity	, Co	ntrol	De	lay, a	nd LC	S De	ter	rmin	ation			<u>-7</u>	0.0 2011	9.41	0 - 0	0.0				
				Ē					VB		T			NB			·····		SB	_	
djusted Flov	w Rate			735	,						T		72	10	206	3		$\overline{T}$	7		
ane Group (	Capacity			144	7						T	_	╅		892			十			
c Ratio			(	2.51							十		10	40	2.2	3		┿╌	<del> </del>		
reen Ratio			7	). 51					<u> </u>		十		+		2.36			十	<del>-  </del>	_	
niform Dela	y d <sub>1</sub>	$\top$	1	2.9	,						$\dagger$		+		17.7			╁	<del> </del>		
elay Factor	k	1	C	). 50	,	<del>-  </del>	<del></del>				+	_	┿		).50			十			
cremental D	Delay d <sub>2</sub>	十		1.3		-	+		+		+		+-	2.3	0.8			╀			
Factor	<u>-</u>	十	1	.00					+		┿		_		.00			+			
ontrol Delay					_			$\dashv$		+	-	<del></del>		18.			十				
ine Group L	e Group LOS B				<del></del>			<del></del> +		+	<del>-</del>	+∽	<del></del>		<del>-</del>		┼				
	roach Delay 14.1				 1	-+					╀		<u> </u>		В			<u></u>			_
proach LOS						<del></del>					╀			9.9	_						
<u></u>	ection Delay 16.2						-		<del>-</del>		<u></u>			В							
	niversity of Florida,								ln.	tersec		n LO	<u>s</u>			_ [		E	3		

General In	formation				SHORT		Informa	tion					
Analyst Agency or (		CKR S ASSOCIA	ATES	-		Inter	rsection Type		GRANT S' ALI CBD o	LIES		:	<del></del>
Date Perfor Time Period		1 <b>2/6/2005</b> CASINO BEAL	< 10.00				sdiction	(	CITY OF PI	r SIM TTSE	ilar BURGH		
			HOUR		· · · · · · · · · · · · · · · · · · ·	Ana	ysis Yea	20	08 COMBIN			V	
Volume and	Timing Inpu	<u>t</u>											
		h-LT	E		LT	WE TH	RT	+-	NB T TH	1 -	- I	SI	
Number of L	anes	0	2	1	<del> </del>	2	0	10	2	+-		T T	
Lane Group	· · ·		LT	R	-	TR	┪-	+	LTR	╁┷	<u></u>	——	
Volume (vph	1)	24	135	111	<del> </del>	142	29	26		35	-	TR	+-
% Heavy Ve	hicles	0	0	0	<del> </del>	0	0	1 0	0	0		826	3
PHF		0.94	0.94	0.94	<del>                                     </del>	0.81	0.81	0.80		0.8		0	(
Pretimed/Ac	tuated (P/A)	P	P	P	_	P	P	P	P 0.80	P	——————————————————————————————————————	0.88 P	0.8
Startup Lost	Time		2.0	2.0	1	2.0	<del>-                                    </del>	╁┷	2.0	+-	<del>-  </del>		F
	Effective Gree	n	2.0	2.0	+-	2.0	<del> </del>	<del> </del>	2.0	$\vdash$		2.0	<del> </del>
Arrival Type		_	3	3	<del>                                     </del>	3	<del>-</del>	+-	3	├	<del> </del>	2.0	+
Unit Extension	 Эп	-	3.0	3.0	<del>                                     </del>	3.0	<del>                                     </del>	├	3.0	-		3	ֈ
Ped/Bike/RT	OR Volume	100	0	6	100	0	10	100	0	-		3.0	+-
ane Width	<u>-</u> -	1.	11.0	12.0	1,00	12.0	1-	100	11.0	0	100		0
arking/Grad	e/Parking	N	0	N	N	-2	$+_{N}$	N	1	N	$\frac{1}{N}$	11.0 -1	
arking/Hour		1	<del>                                     </del>	1 -	<del>                                     </del>	<del>  -</del> -	<del>  ''-</del>	<del>- ''</del>	<del>-                                    </del>	14	-   N		N
Bus Stops/Ho			0	0	†	0	┼	<del>                                     </del>	0		┪	0	+
	lestrian Time		18.9			17.9			28.9	_	<del> </del> -	20.1	+
hasing	EW Perm	02		03		4	NS Per		06	T	07		08
īming	G = 33.0 Y = 5	G = Y =	G Y		G= Y=		G = 46		G =		3 =	G =	
uration of A	nalysis (hrs) =	1 -			<u> </u>		Y = 6	_	Y = Cycle Leng		/ <del>=</del>	Y =	
	p Capacity,		Delay	, and L	OS De	termin	ation		CACIE FELIC	Jui C	- 90.0	<del></del>	
			EΒ	,		WB	1		NB			SB	
djusted Flow	Rate		170	112		211	-	<del></del>	776			981	<del>1</del>
ane Group C	apacity		1022	547		1159			1392			1606	+
c Ratio	<u> </u>	<del></del> -	0.17	0.20		0.18			1				<del> </del>
reen Ratio	<del></del>		0.37	0.37	-	0.18			0.56		<u>-</u>	0.61	—
niform Delay	d,		19.2	19.5		19.3			0.51		<del> </del>	0.51	↓
elay Factor k			0.50	0.50		0.50			15.0		<del>- </del> -	15.6	<b>├</b>
cremental D		╅┈┪	0.4	0.8	-	0.3			0.50	-	-	0.50	<del> </del>
Factor	* 2	+	1.000	1.000		1.000	<del> </del> -		1.6 1.000			1.7	₩
ontrol Delay		<del>                                     </del>	19.6	20.4		19.7		*	16.7		-	1.000	<del> </del> -
ine Group LO	os	<del>                                     </del>	В	C C	_	B	<del> -</del>		<del> </del>			17.4	<del> </del> -
	roach Delay					19.7			B 16.7		<del> </del>	B	<u></u>
proach LOS	<u>,                                     </u>			19.7 B			16.7		<del> </del>	17.4			
,	<del></del>			В			В		1	В			
ersection De	lav	1	17.7				ntersectio		`				

General Information				s	HOR									
Analyst Agency or Co. TRAN Date Performed Time Period SATURDAY C	<b>12/6/</b> 200 CASINO PE	)5				Inta Are Jur	ersecti ea Typ isdictional	ion e on	GR.	ANT ST & 3 CBD oi TY OF PI 08 COMBIN	76 <sup>-</sup> Simila TTSBU	r RGH		
Volume and Timing Inpu	ıt													
	F	LT	EB	1 07			/B			NB			SE	3
Number of Lanes		1	117	RT	LT	1	4	RT 1	<u>L</u> T	TH	RT	LT	TH	RT
Lane Group	<del></del>	$\frac{1}{L}$		┿┈	+-	TR	<del></del>	R	╂	2	<del> </del>	——	1	1
Volume (vph)		<del>-</del> 59		+	+	88		800	<del> </del> -	T	ļ	<del> </del> _	<i>T</i>	R
% Heavy Vehicles		0		+	┪—	1		1	<del> </del> -	592	╂	┨	728	180
PHF		71			+	0.85		.85	├	1 1	<del> </del>	<del> </del>	1	1
Pretimed/Actuated (P/A)	<del></del>	P		┼	┼	P				0.70	<del> </del> -		0.80	0.80
Startup Lost Time		.0		╁	┼	2.0	<del></del>	P 2.0		P	<b>↓</b>	<del> </del>	P	P
Extension of Effective Gree	<del></del>	.0		+	╁	2.0				2.0	<del> </del>	<u> </u>	2.0	2.0
Arrival Type	—-↓	3		┼	<del> </del> -	3		2.0	<b>_</b>	2.0	<u> </u>	<u> </u>	2.0	2.0
Jnit Extension		.0		┼	┼──	3.0		3		3	ļ	<del> </del>	3	3
Ped/Bike/RTOR Volume		20	0	<del> </del>	0	0	_	.0		3.0	ļ	ļ	3.0	3.0
ane Width	2.0	<del>-</del> -		-	13.0		0 2.0	0	0		0	0	0	
Parking/Grade/Parking			0	N	N	2		2.0 ·	N	10.0		<del>                                     </del>	12.0	12.0
Parking/Hour	$\neg \uparrow \neg$			<del>                                     </del>	<del>                                     </del>	<del> </del> -	+-	<del>`  </del>	70	3	N	N .	-1	N
Bus Stops/Hour	- (	,				0	+ (	<del>,  </del>		0		<del> </del>	0	0
//inimum Pedestrian Time			17.6			3.2				3.2			3.2	+ -
Phasing EB Only	WBC			)3	0	4	Thr	u & R	īŢ	06		07		<u>1                                    </u>
iming $G = 15.0$ $Y = 5$	G = 20 $Y = 5$	0.0	G = Y =		Ğ =			39.0		) =	G =	:	G =	
Ouration of Analysis (hrs) =			Y =		Y =		Υ =	6		= vola	Y =		Y =	
ane Group Capacity		ol De	lav a	and I (	OS De	tormi	natio	n	10	ycle Leng	in C =	90.0		<del></del>
	1		В		<del>50 50</del>	WB	nauc	<del>""</del>		NB			SB	
djusted Flow Rate	224		<u> </u>	1		122	335	+		846				loor
ane Group Capacity	271	$\vdash$		$\neg \neg$		376		-+		1284		·	910	225
							317						738	627
c Ratio	0.83	_				0.32	1.06			0.66			1.23	0.36
reen Ratio	0.17		_			0.22	0.22			0.43			0.43	0.43
niform Delay d <sub>1</sub>	36.2	<u> </u>				29.3	35.0			20.2			25.5	17.1
elay Factor k	0.50	$\bot$				0.50	0.50			0.50			0.50	0.50
cremental Delay d <sub>2</sub>	24.2					2.3	66.3			2.7			116.5	1.6
Factor 1.000						1.000	1.000	)		1.000			1.000	1.000
trol Delay 60.4						31.6	101.3	3		22.9			142.0	18.7
e Group LOS E						С	F			С			F	В
proach Delay								1		22.9			117.6	
ргоасh LOS	pach LOS E						-	_		С		·	F	
	ection Delay 76.7												•	

Generated: 12/9/2005 8:26 PM

General Info Analyst Agency or C Date Perform Time Period Volume and	o. <i>TRAN</i> S	CKR					REP		<del></del>					_	
Agency or Control Date Perform Time Period	o. TRANS	CVO				_	Site	informa	tion						
Volume and	ned 1. SATURDA	ASSOC 2/6/2005	5				Are:	rsection a Type sdiction lysis Yea	С	ECOND A ROS CBD o ITY OF P 8 COMBIN	SS S or Sir ITTS	ST milar SBU:	RGH		
	Timing Input				<del></del>	-	<u> </u>								
				EB			WE			NB				SB	
Number of La		L.	-	TH 1	RT	LT	TH	RT	LT	TH	+-	RT	LT	TH	RT
Lane Group	21109		$\dashv$	LTR	1	╁╌—	1 7	1	0	1	<u> </u>	0	1	1 1	0
Volume (vph)	···	1	$\dashv$	142	1	<del> </del>	119	R	<del>  _</del> _	LTR	+		L.	TR	<del> </del>
% Heavy Veh		1		142	1	<del> </del>	119	128	2	362	-	8	176	70	10
PHF		0.80	<del>-</del>	0.86	0.86	<del>                                     </del>	0.90	0.90	2	2	_	2	3	3	3
Pretimed/Actu	isted (P/A)	P	<del>"</del>	P	P	-	0.90 P	0.90 P	0.68	0.68	0.0		0.89	0.89	0.89
Startup Lost 1		+-	$\dashv$	2.0	<del>                                     </del>		2.0	2.0	P	P	F		P	P	P
<u> </u>	Effective Green	,		2.0	├──		2.0	2.0	<del>                                     </del>	2.0	<del> </del>		2.0	2.0	<u> </u>
Arrival Type		<del>`                                    </del>	$\dashv$	3			3	3		2.0	├		2.0	2.0	<u> </u>
Unit Extension	n	<del></del>	$\dashv$	3.0	-		3.0	3.0	<u> </u>	3	┝		3	3	<del> </del>
Ped/Bike/RTC		50		0	0	50	0	0	50	3.0	┡		3.0	3.0	<del> </del>
Lane Width	ne Width			13.0	<del>-</del>	30	11.0	14.0	30	14.0	3		50 10.0	0	0
Parking/Grade	rking/Grade/Parking			-2	Y	N	5	N	N	2	Нy	,	70.0	10.0	N
Parking/Hour	rking/Grade/Parking rking/Hour				10			<del>                                     </del>		<del>                                     </del>	10		74	-3	10
Bus Stops/Ho				0			0	0		0			0	0	-
Minimum Ped				13.0			13.0		-	12.5				8.5	<del>                                     </del>
Phasing	EW Perm	02			03		)4	NS Per		06			07	0	8
	G = 23.0 Y = 5.5	G = Y =		G = Y =		G =		G = 36 $Y = 5.5$	1	G =		G =		G =	
	alysis (hrs) = (					<u> </u>		11 = 5.0		Y ≃ Cycle Len	ath i	Y =		Y =	
Lane Group	Capacity,	Contr	<u> 1</u> 0	Delay,	and L	OS De	termi	nation		- J 0.10 E 0.1	3		70.0		<del></del>
				EB			WB			NB				SB	
Adjusted Flow	Rate		11	67			132	142	-	572			198	90	
_ane Group Ca	apacity		40	63			509	441		764			313	789	
//c Ratio			0.	36			0.26	0.32		0.75			0.63	0.11	
Green Ratio	<u> </u>		0.	33			0.33	0.33		0.51		—∔		0.51	_
Jniform Delay	d <sub>1</sub>	1 -	17	7.9	Ť		17.2	17.6		13.4		-	12.2	8.8	
Delay Factor k			0.5	50		- "	0.50	0.50		0.50				0.50	·
ncremental De	elay d <sub>2</sub>		2	2.2			1.2	1.9		6.6		1	9.4	0.3	
F Factor	<del>-</del>		1.0	000		<u> </u>	1.000	1.000		1.000		+		1.000	-
Control Delay			20	0.1			18.5	19.6		20.1			21.6	9.1	
ane Group LC			7	7			В	В		С		十	С	A	
pproach Dela				0.1		4	19.0	· -		20.1		+	-	17.7	
pproach LOS					$\neg \neg \uparrow$		В			C		$\dashv$			
ntersection De	lay		19	9.3				Intersecti	on LOS			十	· · ·	В	

#### SHORT REPORT General Information Site Information WASHINGTON PL & Intersection Analyst CKR BEDFORD/CENTRE Agency or Co. TRANS ASSOCIATES Area Type CBD or Similar Date Performed 12/6/2005 Jurisdiction CITY OF PITTSBURGH Time Period AM PEAK HOUR 2008 COMBINED Analysis Year CONDITION Volume and Timing Input EB WB NB SB LT $\mathsf{TH}$ RT LT RT TH LT TH RT ΙT RT TH Number of Lanes 1 1 1 2 1 1 1 1 Lane Group L LTR R T R L LTR Volume (vph) 525 349 340 79 566 155 169 88 % Heavy Vehicles 0 0 0 0 0 0 0 0 PHF 0.97 0.97 0.97 0.92 0.92 0.90 0.90 0.90 Pretimed/Actuated (P/A) P P P P P P P Startup Lost Time 2.0 2.0 2.0 2.0 2.0 20 2.0 2.0 Extension of Effective Green 2.0 2.0 2.0 2.0 2.0 2.0 20 2.0 Arrival Type 3 3 3 3 3 3 3 3 Unit Extension 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Ped/Bike/RTOR Volume 150 0 0 0 0 8 0 0 Lane Width 11.0 11.0 12.0 12.0 12.0 16.0 10.0 12.0 Parking/Grade/Parking N 5 Ν Ν -1 Ν Ν 6 Ν Parking/Hour Bus Stops/Hour 0 0 0 0 0 0 0 Minimum Pedestrian Time 15.5 3.2 3.2 Phasing EB Only 02 03 NB Only SB Only 07 08 G = 37.0G = G = G≖ G = 20.0G = 18.0G = G= Timina Y = 5Y = Y = Y = Y = 5 Y = Y = Duration of Analysis (hrs) = 0.25Cycle Length C = Lane Group Capacity, Control Delay, and LOS Determination FB NB SB Adjusted Flow Rate 449 557 246 615 77 143 217 98 Lane Group Capacity 629 617 476 727 325 357 308 282 v/c Ratio 0.71 0.90 0.52 0.85 0.24 0.40 0.70 0.35 Green Ratio 0.41 0.41 0.41 0.22 0.22 0.20 0.20 0.20 Uniform Delay d<sub>4</sub> 22.1 24.8 19.8 33.5 28.7 31.3 33.5 31.0 Delay Factor k 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.50 Incremental Delay do 6.8 18.9 40 11.6 1.7 3.3 12.7 34 PF Factor 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 Control Delay 28.9 43.7 23.8 45.2 30.4 34.6 46.2 34.3 Lane Group LOS C D C D C $\boldsymbol{c}$ D C Approach Delay 34.5 43.5 40.1 Approach LOS CD D

38.2

Intersection Delay

Intersection LOS

D

Generated: 11/13/2006 4:01 PM

General In	formation					REP(	nforma	ition					
Analyst									WASH	INGTO	VPI &		
Agency or	Co. TRAN	IS ASSOC	IATES			1	ection		BEDFC	RD/CE	VTRE		
Date Perfor	med	11/21/200				Area	Type liction	,		or Simi			
Time Period	d <i>PM</i>	PEAK HO	UR				sis Yea	ır <i>20</i>	OTY OF 08 COM	PH ISE	SURGH DITIONS	:	
Volume ал	d Timing Inp	ut			<del>-</del>						37710740		
			EB			WB	·		NB			SB	
Number of I	2000	LT 1	TH		LT	TH	RT	LT	TH	RT	LT	TH	R
Lane Group			LTR	1		╄—-	+		2	1	1	1	1
Volume (vpl	<del></del> -	895			-	<del>                                     </del>	<del> </del>	<del> </del>	7	R	L	LT	R
% Heavy Ve	<del></del>	1	219	63	+		<del> </del>	ļ	1399	278	277	214	124
PHF	THUES			1		<del> </del>		ļ	0	0	3	3	10
	4	0.97	0.97				<del> </del>	<u> </u>	0.92	0.92	0.89	0.89	0.89
	tuated (P/A)	P	P	P	-	<del> </del>			P	P	Ρ	P	P
Startup Lost		2.0	2.0	2.0	<del>-</del>	<u> </u>	<u> </u>		2.0	2.0	2.0	2.0	2.0
	f Effective Gre		2.0	2.0		<u> </u>	<u> </u>	ļ	2.0	2.0	2.0	2.0	2.0
Arrival Type		3	3	3	<del> </del>		ļ	<u> </u>	3	3	3	3	3
Unit Extensi		3.0	3.0	3.0					3.0	3.0	3.0	3.0	3.0
	OR Volume	150	0	0				0	0	28	0	0	0
Lane Width	do/Doddina	11.0	11.0		<del>  -</del>				12.0	12.0	16.0	10.0	12.0
Parking/Grad Parking/Hou		N	5	N N	<del> </del>	ļ <u>.</u>	<b> </b>	Ν	-1	N	N	6	Ν
Bus Stops/H		0	0	0									<u> </u>
	destrian Time		15.6	<del>                                     </del>	<del> </del>			-	0 3.2	0	0	0	0
hasing	EB Only	02	1	03		<u> </u>	NB O	nlv	SB On	lu I	07	3.2	)8
Fiming	G = 32.0	G =	G=		G=		G = 39		G = 14.			G =	<i>7</i> 0
	Y = 5	Y =	Υ=		Y =		Y = 5		Y = 5	Y		Y =	
	nalysis (hrs)		<del></del>						Cycle Le	ength C	= 100.0	)	
Lane Grou	ıp Capacit	y, Contr		ay, and	LOS		<u>minati</u>	on					
<del></del>			T EB			WB		<b>-</b>	NB			SB	
Adjusted Flo	w Rate	600	552	62					1521	272	156	395	139
ane Group	Capacity	485	495	344			†	1-	1276	57A	1040	1	1
			<del> </del>		<b> </b>		<del>  -</del>			570	243	206	197
/c Ratio		1.24	1.12	0.18			<del>                                     </del>	<u> </u>	1.19	0.48	0.64	1.92	0.71
reen Ratio		0.32	0.32	0.32				4	0.39	0.39	0.14	0.14	0.14
Iniform Dela		34.0	34.0	24.5					30.5	22.9	40.6	43.0	41.0
elay Factor		0.50	0.50	0.50			<u> </u>		0.50	0.50	0.50	0.50	0.50
cremental [	Delay d <sub>2</sub>	123.4	75.9	1.1					94.4	2.8	12.3	430.4	19.1
F Factor		1.000	1.000	1.000					1.000	1.000	1.000	1.000	1.00
ontrol Delay	·	157.4	109.9	25.7					124.9	25.7	53.0	473.4	60.2
ane Group L	.os	F	F	Ç					F	С	D	F	E
pproach De	lay		129.1						109.9		<del>                                     </del>	295.1	<del></del>
	S		F				_		F			F	
pproach LO	_												

					_		HOR	T REP	ORT								
General In	formation								Informa	ation			-				
Analyst Agency or Date Perfor Time Perior	rmed 1 d <i>PM</i>	1 1/2 PE	SSOC 21/2005 AK HO	5				Area Juris	section Type diction ysis Yea		Ci	RANT ST CBD TY OF I	or S PIT	Simil TSB	ar URGH		<u></u>
Volume an	d Timing Inpu	ut															
			LT	_	EB	1 57		WB		Ţ.,		NB	_			SB	
Number of	Lanes		0	╁	TH 1	RT 0	LT	TH 2	RT 0	1 1		TH 2	+	RT 0	LT	TH	RT
Lane Group	<del></del>	_		1,	TR	+-	<del>†</del> •	LTR	┿	$\frac{1}{L}$		TR	╁		1 L	2 TR	0
Volume (vp	h)		82		22	50	145	290	200	11	<u> </u>	376	1	64	171	540	44
% Heavy Ve	ehicles		7	+-	7	7	4	4	4	12		12	+-	2	4	4	44
PHF			0.83	0.	.83	0.83	0.94	0.94	0.94	0.9		0.95	+-	<u>-</u> 95	0.75	0.75	0.75
Pretimed/Ad	ctuated (P/A)		P	Τ.	P	P	P	P	P	P		P	+	5	P	0.75 P	D.75
Startup Lost	t Time			2	2.0	<del>                                     </del>	<del>                                     </del>	2.0	<del>                                     </del>	2.0		2.0	╁		2.0	2.0	+-
Extension o	f Effective Gre	en	-	2	.0		_	2.0	<del> </del>	2.0		2.0	┢		2.0	2.0	┼──
Arrival Type	:			Ì,	3			3	$\vdash$	3		3	$\vdash$		3	3	<del>                                     </del>
Unit Extensi	on			3	.0			3.0	_	3.0	)	3.0			3.0	3.0	┼
Ped/Bike/R1	ed/Bike/RTOR Volume				0	0	340	0	0	569		0	0	)	236	0	0
Lane Width	ne Width				2.0			11.0	<del>                                     </del>	12.		10.0	H		12.0	10.0	<del>                                     </del>
Parking/Gra	arking/Grade/Parking				4	N	N	-5	N	N		-1	٨	1	N	2	N
Parking/Hou		$\perp$															┢┷┪
Bus Stops/H		4		_	0			0		0		0			0	0	
	destrian Time			18				19.2				19.5				13.8	
Phasing	EB Only G = 7.0		N Perm = 35.0		G =	03	G =	14	Excl. L		_	S Perm			07		18
Timing		_	- <u>55.0</u> - 5	_	Y =		Y =		G = 7.0 $Y = 3$	0		= 25.0 = 5	-	G =		G = Y =	——
	Analysis (hrs) =						- <del> </del>	<u>-                                 </u>			_	ycle Len	 gth		90.0	11-	$\overline{}$
Lane Grou	up Capacity	, C	ontro	<u> </u>	Dela	y, and	LOS	Deter	minati	on				_			
					E8			WB				NB				SB	
Adjusted Flo				5.	47			676		12	1	779	Π		228	779	T
Lane Group	Capacity			39	97			870		189	,	576			194	779	<del>                                     </del>
v/c Ratio			_	1.	38			0.78		0.6	4	1.35	Γ		1.18	1.00	$\overline{}$
Green Ratio				0.	50			0.39		0.3	9	0.28	T		0.39	0.28	<del>                                     </del>
Uniform Dela	ıy d <sub>1</sub>			22	2.5			24.1		20.9	<del>,</del>	32.5		_	33.0	32.5	<del>                                     </del>
Delay Factor	k			0.1	50			0.50		0.50	,	0.50			0.50	0.50	<del>                                     </del>
Incremental [	remental Delay d <sub>2</sub>				5.2			6.8	<del>                                     </del>	15.5	5	169.8			119.8	32.2	$\vdash$
PF Factor	Factor						<del>                                     </del>	1.000		1.00		1.000	┝		1.000	1.000	<del> </del>
Control Delay								30.8		36.	3	202.3			152.9	64.7	
Lane Group L	e Group LOS F							С	1	D		F		_	F	E	<u> </u>
Approach De	roach Delay 207.7							30.8	·			180.0	Щ.	_	<u> </u>	84.7	
Approach LO								С	-		-	F	_			F	
ntersection D	pach LOS F section Delay 122.0					· .			Intersec	tion I	09					F	
	ection Delay 12														L_	_ •	

						SHOR	REP	ORT		_				-		
General In	formation							Informa	ation						<del>-</del>	
Analyst Agency or Date Perfor Time Perior	rmed	11/2	SSOC 21/200: AK HC	5			Inter Area Juris	section Type diction ysis Yea			CENT WASH CBD TY OF 8 COM	ING or - PIT	TO! Simil TSB	N PL lar URGH	s	
Volume an	d Timing Inp	ut								_				7710740		
				EB			WB				NB			<u> </u>	SB	
Number of	lance		LT	TH	RT	LT_	TH	RT	L	Γ	TH	+-	RT	LT	TH	RT
Lane Group			<del> </del>	2	0	1	2	0	0		3	╀	0	0	2	1
Volume (vp				TR		L	TR	<u> </u>			LTR	Ļ.		DefL	T	R
	<u> </u>		<del> </del>	1000	96	148	374	413	13		878	[3	32	49	97	41
% Heavy Ve	enicies			2	2	2	2	2	1		1	+	1	9	9	9
				0.77	0.77	0.87	0.87	0.87	0.84	<i>*</i>	0.84	0.	84	0.91	0.91	0.91
	tuated (P/A)			P	P	P	P_	P	P	_	P	1	<u> </u>	Ρ	P	P
Startup Losi				2.0	<u> </u>	2.0	2.0				2.0			2.0	2.0	2.0
	f Effective Gre	en		2.0		2.0	2.0	<u> </u>			2.0			2.0	2.0	2.0
Arrival Type			_	3	_	3	3				3			3	3	3
Unit Extensi	<del></del>	_		3.0	<u> </u>	3.0	3.0				3.0			3.0	3.0	3.0
Ped/Bike/R1	OR Volume		138	0	0	291	0	0	12		0	(	_	57	0	0
Lane Width				13.0		12.0	10.0				12.0			12.0	12.0	12.0
Parking/Grad		_	N	-1	N	N	-6	Ν	Ν		6		′	N	-3	N
Parking/Hou		-			- <b> </b>							1	0			
Bus Stops/H	our destrian Time	-		0	<del> </del>	0	0				0			0	0	0
Phasing	WB Only		Al Dave	24.6	1 0 1		25.7		<u> </u>	_	21.3				3.6	
-	G = 6.0		N Pern = 30.0		eds Only = 15.0	G =	4	NS Pe G = 20		_	= 06 =		G=	07		08
Timing	Y= 3	Υ =	= 5		= 5	Y =		Y = 5.8		Y			Y =		G = Y =	
	nalysis (hrs)									C	ycle Ler	ngth	<u> </u>		<u>L'-</u>	
Lane Grou	ıp Capacit	<u>y, C</u>	ontro			J LOS	Deter	minati	on							
			<u> </u>	EB			WB				NB				SB	
Adjusted Flor	w Rate			1424		170	905				1098			54	107	45
ane Group	Capacity			1079		189	1056				899			80	363	285
//c Ratio				1.32		0.90	0.86			_	1.22			0.68	0.29	0.16
Green Ratio				0.33		0.43	0.43				0.23			0.23	0.23	0.23
Jniform Dela	y d <sub>1</sub>		, 	30.0		21.3	23.0				34.8	↾		31.7	28.8	27.8
Delay Factor	lay Factor k					0.50	0.50				0.50	T		0.50	0.50	0.50
ncremental [	Delay d <sub>2</sub>		150.5		43.5	9.0				109.6			37.2	2.1	1.2	
PF Factor			1.000		1.000	1.000				1.000			1.000	1.000	1.000	
Control Delay			180.5		64.8	32.0				144.4			69.0	30.8	29.0	
ane Group L	.os	_	F		Ε	С				F			Ε	С	C	
pproach Del							37.1				144.4			<u> </u>	40.4	<u> </u>
pproach LO	S			D				F				D				
ntersection D	elay			122.0	_			Intersec	tion L	OS	3				F	
pyright © 2005 I	Iniversity of Florida	a All	Pichte P	esenied		<del></del>	HOD							Щ.	<u> </u>	

General In	formation				S	HORT									
Analyst Agency or Date Perfo Time Perio	Co. TRAN rmed d PN	IS A 11/2 1 PE	CKR SSOCI 21/2005 AK HOI				Inters Area Juriso	Informa section Type diction vsis Yea		ARMSTR	RONG FOR S	Similar TSBU	INEL RGH		
Volume an	d Timing Inp	ut	<u> </u>												
			LT	EB	RT	LT	WB TH		<u> </u>	NI T T				SB	
Number of	Lanes			2	0	╅╧	<del>                                     </del>	1/1	<u> </u>		<del>'</del> +	RT 2	LT	TH	RT
Lane Group	)			TR		1	<del>                                     </del>		+ -	LR	+	<u>-</u>	╄┈─	┿	<del>                                     </del>
Volume (vp	h)			746	608	$\dagger$	┼	+	10		_	492	<del>                                     </del>	+	<del> </del> -
% Heavy V	ehicles			4	4	<del>                                     </del>	† –	+ -	1	<del></del>	┯	1	┾	├──	<del> </del> -
PHF			1 -	0.90	0.90	1	<del> </del> -	† -	0.9	7	+	0.97	┼	<del>                                     </del>	
Pretimed/Ad	tuated (P/A)		1	P	P	+	<del>                                     </del>	+	P		_	P		<del> </del>	<u> </u>
Startup Los	Time			2.0		† –	<del> </del>	+	┝	2.0		2.0		-	
Extension o	f Effective Gre	en	<b>†</b>	2.0	<del>                                     </del>	<del>† -</del>	<del>                                     </del>	+		2.0	_	2.0		}	<u> </u>
Arrival Type				3		+	-	┪—	<del> </del>	3	-	3			
Unit Extensi	on		+	3.0		†	<del>                                     </del>	╁		3.0	<del></del>	3.0		├─┤	
Ped/Bike/R1	OR Volume		50	0	0	+			0	0		0			<u> </u>
Lane Width		<del>                                     </del>	11.0		<del>                                     </del>		<del> </del> -		11.0		1.0				
Parking/Gra	de/Parking		N	3	N		<del>                                     </del>	<del> </del>	N	0	<del>-</del>	1.0 N	-		
Parking/Hou	r							<del>                                     </del>	···	╅	<del>- -</del>	7.4		<del>  </del>	
Bus Stops/H				0					_	0	+-	0			
	destrian Time			17.3						3.2	<del>                                     </del>	_			
Phasing	EB Only		02	0:	3	04		NB On		06		0	7	08	}
Timing	G = 46.0 Y = 5	G = Y =		G = Y =		G = Y =		G = 24.	0	G =		G =		G =	
Duration of A	nalysis (hrs)			<del>                                     </del>		<u> </u>		Y = 5		Y = Cycle Le		Υ = • -	80 O	Y =	
Lane Grou	ıp Capacit	y, C	ontro	l Delay	. and	LOS D	)eterr	ninatio	n Sn	Cycle Le	ngui	<u> </u>	00.0		
			_	EB	<u> </u>		WB		Ϊ	NB				SB	
Adjusted Flor	w Rate			1505				T -				<del>_  </del> -		<del></del>	
<u> </u>				1567		_				308	30	9			
ane Group	Capacity						_	<u> </u>		436	739	9	- 1		
//c Ratio				0.96						0.71	0.4	2			
Green Ratio				0.57			•			0.30	0.30	0			
Jniform Dela	y d <sub>1</sub>			16.1	T					24.9	22.	4			
Delay Factor	k	$\Box$		0.50						0.50	0.50		+		
ncremental [	Delay d <sub>2</sub>		15.1				_	_	9.3	1.7		$\dashv$			
F Factor			1.000						1.000	1.00					
Control Delay			31.2		$\neg \uparrow$			_	34.2	24.		<del>-+</del>			
ane Group L	.os	С		-+				C	C	<del>-</del>	<del>-  </del>	<del>-</del>			
pproach Del	ay	31.2			<del></del> .	<del>-  </del>		29.1	1 ~	+					
pproach LO	S	С	_		·			C C		_					
ntersection D	elay		30.6			1.	ntersecti	on I o							
	Iniversity of Florida	. Ali i					_	M Version						C 10:	

					SI	IORT	REPO	ORT						
General Ini	ormation							nforma	tion					
Analyst Agency or C Date Perfor Time Period	med	11/2	SSOCI/ 1/2005 AK HOL		-		Inters Area Jurisd	ection Type	c	HATHAI	or Simila PITTSBU	ULTY ir IRGH	.,	
Volume and	d Timing Inp	ut												
				EB	<del></del>		WB			NB			SB	
Number of L	anes		LT 1	TH 2	RT 0	LT	TH	RT	LT_	TH 1	RT	LT	TH	RT
Lane Group			1 -	TR	<del>                                     </del>		+-		<del> </del>	$\frac{1}{T}$	1 R	0	LT	<del> </del>
Volume (vpl			368	783	71	┪-	+	<del> </del>		170	136	154	54	├
% Heavy Ve	hicles		4	4	4	╅	╁	†	1	0	0	1	1	<del>                                     </del>
PHF	.,,		0.88	0.88	0.88	+-	† -	╁──	<del> </del>	0.71	0.71	0.80	0.80	
Pretimed/Ac	tuated (P/A)	•	P	P	P	+	$\vdash$	<del>                                     </del>	<del>                                     </del>	P	P	P	P	
Startup Lost	Time		2.0	2.0		†	†			2.0	2.0	<del>  '</del>	2.0	
Extension of	Effective Gre	en	2.0	2.0					<del>  -</del> -	2.0	2.0		2.0	
Arrival Type			3	3						3	3	<b>-</b>	3	
Unit Extension	on .		3.0	3.0						3.0	3.0		3.0	
Ped/Bike/RT	OR Volume		87	0	0	T	1 -		59	0	14	21	0	
Lane Width			10.0	11.0						10.0	11.0		10.0	
Parking/Grad			N	3	N				Ν	-6	N	N	10	Ν
Parking/Hour			-			<u> </u>								
Bus Stops/He	our destrian Time		0	0		<del> </del>		ļ		0	0		0	
Phasing	EB Only		02	14.2	3	04	<u>L.                                     </u>	NS Pe		12.6			12.3	
	G = 36.0	G =		G=	3	G=		G = 34		06 G =	G=	07	G =	
Timing	Y = 5	Υ=		Υ=		Υ =		Y = 5	,	Y =	Y =		Y =	
Duration of A									(	Cycle Lei	ngth C =	80.0		
Lane Grou	ip Capacit	<u>y, C</u>	ontro		, and	LOS E		ninati	<u>on</u>					
Adjusted Flov	v Pato		418	EB 971		1	WB	<del></del>	<del> </del>	NB	Т		SB	
			_	1314				├─	<del> </del>	239	172	<b> </b>	259	
Lane Group (	Capacity —————		646							699	589		342	
v/c Ratio	<u> </u>	-+	0.65	0.74					<u> </u>	0.34	0.29		0.76	
Green Ratio		$\rightarrow$		0.45						0.43	0.43		0.43	
Uniform Delay	<u></u>	<del></del>		18.1				<u> </u>		15.5	15.1		19.5	
Delay Factor				0.50				<u> </u>		0.50	0.50		0.50	
ncremental C	elay d <sub>2</sub>		5.0	3.8						1.3	1.3		14.5	
PF Factor		-		1.000						1.000	1.000		1.000	
Control Delay		22.0	21.9						16.8	16.4		34.0		
ane Group L		$\bot$	С	С						В	В		С	
Approach Del										16.6			34.0	
Approach LOS			С	[					В			C		
ntersection D	elay	[_		22.4			1	ntersec	tion LC	os			С	

					S	HORT	REP	ORT							
General In	formation			_		_	Site	Informa	tion						
Analyst Agency or 0 Date Perfor Time Period	med 1	S A 1/2 PE	CKR SSOCI 21/2005 AK HO	i			Area Juris	section Type diction /sis Yea	r 2	ST	/BIG D or F PIT	ELOI Simil TTSB	ar URGH	3	
Volume an	d Timing Inpu	ıt													
İ			LT	EB	RT	LT	WB TH		↓.	NI				SB	
Number of I	anes		0	3	0	10	2	RT 0	L	_	┪	RT 0	LT	TH 2	RT 0
Lane Group			DefL	TR	<del>                                     </del>	<del>                                     </del>	LTR	<del>                                     </del>	╁┷	LTF	<del>,  </del>		DefL	TR	<del>  "</del>
Volume (vpl	<del></del>		276	538	42	38	374	213	88	_	-+	266	174	264	173
% Heavy Ve	hicles		1	1	1	2	2	2	2	2	╁	2	1	1	1
PHF			0.82	0.82	0.82	0.93	0.93	0.93	0.7	7 0.77	0	.77	0.87	0.87	0.87
Pretimed/Ad	tuated (P/A)		P	Р	P	P	P	P	P	P	_	P	P	P	P
Startup Lost	Time		2.0	2.0			2.0			2.0	十		2.0	2.0	<del>                                     </del>
Extension or	f Effective Gre	en	2.0	2.0			2.0	1		2.0			2.0	2.0	
Arrival Type			3	3			3			3	$\top$		3	3	
Unit Extensi	on		3.0	3.0			3.0			3.0			3.0	3.0	
Ped/Bike/RT	OR Volume		200	0	4	200	0	21	200	0	7	27	200	0	17
Lane Width	<del></del> -	4	12.0	11.0			11.0			12.0			12.0	11.0	
Parking/Grad	<del></del>	4	N	5	N	N	-6	N	N	-1		N	N	-1	N
Parking/Hou Bus Stops/H		$\dashv$	0	0	<del>  -</del>	<del> </del>		_		<del>  _</del>	+		_		
	destrian Time	┪	<del></del>	18.3			0 17.8	+	_	20.8			0	0 8.0	
Phasing	EW Perm		02	T	03	1 0		NS Per	rm	06	!_	┰╌	07	<u>'                                    </u>	8
Timing	G = 32.0	G		G	=	G =		G = 29	.0	G =		G =		G=	
	Y = 5.5 Analysis (hrs) =	Υ: : Λ		Υ =		Y =		Y = 5.5	i	Y =		Y =		Υ=	
	up Capacity	_		ol Dela	v and	LLOS	Deter	minati	^n	Cycle L	engi	n C =	12.0		
	<u></u>	, -		EB	., <del>a</del>		WB	iiiiii e tii	<del>                                      </del>	Ni	3		1	SB	
Adjusted Flo	w Rate		337	702	T		649	Ţ	<del>                                     </del>	795		_	200	482	
Lane Group	Capacity		248	1325			1121			889	$\top$		171	591	<del> </del>
v/c Ratio	•		1.36	0.53			0.58			0.89	十		1.17	0.82	<del> </del>
Green Ratio			0.44	0.44			0.44			0.40			0.40	0.40	
Uniform Dela	ıy d <sub>1</sub>		20.0	14.5			15.0	<u> </u>		20.1	$\top$		21.5	19.1	
Delay Factor	k		0.50	0.50			0.50	<del>                                     </del>	Ť	0.50		_	0.50	0.50	_
Incremental I	Delay d <sub>2</sub>		185.4	1.5			2.2			13.4			121.8	11.8	
PF Factor		1.000	1.000			1.000			1.00	0		1.000	1.000	<del></del>	
Control Delay							17.1			33.4			143.3	30.9	
Lane Group I	ne Group LOS F B						В			С			F	С	
Approach De	roach Delay 77.5									33.4				63.9	<del></del>
Approach LO	roach LOS E									С		<del>-, , , , , , , , , , , , , , , , , , , </del>		E	
Intersection D	Delay			51.1				Intersect	tion l	os				D	<del></del>
opyright © 2005 (	Jniversity of Florida	Rights R	eserved			HCS-	+ <sup>™</sup> Versio	n 5.2			Gener	ated: 11/1	4/2006	10:50 AM	

					S	HOR	REP	ORT								
General In	nformation	Site	Site Information													
Analyst Agency or Date Perfo Time Perio	rmed d AN	Area Juris	Intersection  SIXTH AVE & ROSS ST/BIGELOW Area Type CBD or Similar Jurisdiction CITY OF PITTSBURGH Analysis Year CONDITIONS													
Volume an	id Timing Inp	ut						-			<u> </u>	DITTON				
		<u> </u>	1 7	EB			WB				NB		7	SB		
Number of	l anes		<u>LT</u> 0	TH 3	RT 0	LT	TH	RT	L		TH	RT	LT	TH	RT	
Lane Group			DefL	TR	<del>  -</del>	0	2	0	0		2	0	0	2	0	
Volume (vp			132	373	46	23	LTR 441	407	+-	_	TR_		ļ	LTR	ļ	
% Heavy Vo		<del>-  -</del>	2	2	2	3	3	107	52			240	174	398	285	
PHF		0	.93	0.93	0.93	0.84	0.84	3	2	2		2	1	1	1	
Pretimed/Ad	ctuated (P/A)		P	P	P	P	P	0.84 P	0.8			0.88	0.89	0.89	0.89	
Startup Los			2.0	2.0		-	2.0	+-	P	F		P	P	P	P	
	f Effective Gre		2.0	2.0		├	2.0	<del> </del>	-	2.		<b></b>	<u> </u>	2.0	ļ	
Arrival Type			3	3	<del> </del>	<del>-</del>	3	+	-	2.			<u> </u>	2.0		
Unit Extensi			3.0	3.0			3.0	┼	<u> </u>	13				3	ļ	
Ped/Bike/RTOR Volume			00	0	5	200	0	11	200	3.		0.4	-	3.0	ļ	
Lane Width			2.0	11.0		200	11.0	<del>  ''</del>	200	12		24	200	0	29	
Parking/Grade/Parking			N	5	N	N	-6	N -	N	-1		N	N	11.0	<del>                                     </del>	
Parking/Hour			1				Ť		<del>  '`</del>	+-'		70	/V	-1	N	
Bus Stops/Hour			o	0			0	1		10				0	<del> </del>	
Minimum Pedestrian Time				18.3			17.8			20.	8			8.0		
Phasing	EW Perm		)2	03		04	4	NS Per	_	06		T	07	T 0	8	
Γiming	G = 26.0 Y = 5.5	G = Y =				G = Y =		G = 33.		G =		G =		G =		
Ouration of A	nalysis (hrs)		<u> </u>	+ ' -		T =		Y ≃ 5.5		Y =	Lon	Y =	= Y = = 70.0			
	ıp Capacit			l Dela	v. and	LOS	Deter	minatio	<u>1</u>	Cycle	Len	giri C =	70.0			
				EB	,		WB		<del>]                                    </del>		IB		Γ	SB		
djusted Flo	w Rate	14	142 445				666	T	_		449		931		<del></del>	
ane Group (	Capacity	18	83	1083	_		1019			96	-			995		
/c Ratio		0.1	78	0.41			0.65			0.4	7			0.94		
reen Ratio		0.0	37	0.37			0.37			0.4	7			0.47	<del>                                     </del>	
Iniform Dela	y d <sub>1</sub>	19	.4	16.3			18.3			12.	12.5			17.5		
elay Factor	k	0.5	50 (	0.50			0.50			0.50	$\rightarrow$			0.50		
cremental [	Delay d <sub>2</sub>	26	5.9	1.2			3.3			1.6				16.7		
F Factor			200	1.000			1.000	<del>  </del>		1.00				1.000		
ontrol Delay			5.3 17.5				21.5			14.	$\rightarrow$			34.2		
ane Group L	.OS	D	, 1	В			С			В	┪			C .		
pproach Del	ay	$\top$		24.5			21.5	<del>'  </del>		14.	<u>_</u> 1		34.2			
							, ,,									
oproach LOS	S			С	}		С	Γ		В				С		

						S	HORT	REP	ORI					-					
General In	formation		CKR					Site	Info	mat	ion								
Analyst Agency or ( Date Perfor	Intersection  SIXTH AVE & ROSS  ST/BIGELOW  Area Type  CBD or Similar																		
Time Period	Jurisdiction CITY OF PITTSBURGH Analysis Year 2008 COMB CONDITIONS																		
Volume an	d Timing Inp	ut							<u> </u>		-								
EB												_	ΝB				SB		
Niconshanad			LT	-	TH	RT	LT	ТН	_	RT	Ľ		TH	-	RT	LT	TH	RT	
Number of Lane Group	-		0 DefL	┷	3 TR	0	0	2 LTR	+	)	0	12		╀	0	0	2	0	
Volume (vp			250	+	64	25	15	227	15	-0	27		R	+	0.7	122	LTR	<del> </del>	
% Heavy Ve			0	-	0	0	2	227	1 2		27 2	12		+	97	175	170	170	
PHF			0.87	┿	87	0.87	0.78	0.78	0.		0.86	<del>_</del>		₩	2 86	1	1	1	
	tuated (P/A)		P	+	<del>D.</del>	P	P	P	F		P	F		4—	00 D	0.83 P	0.83 P	0.83	
Startup Lost	Time		2.0	1 2	.0	<del></del>	<del>                                     </del>	2.0	+		<u> </u>	2.		╁		<del>                                     </del>	2.0	+	
Extension o	f Effective Gre	en	2.0	2.0		_	<del>                                     </del>	2.0	╁┈			2.		H		<del> </del>	2.0	<del> </del>	
Arrival Type			3	3 3			_	3	╅			3		┢		<u> </u>	3	<del>                                     </del>	
Unit Extensi	on		3.0	3	.0			3.0			_	3.	0	T		<del> </del>	3.0	<del>                                     </del>	
Ped/Bike/R1	OR Volume		100	0		3	100	0	16	5	100	0	,	3	0	100	0	17	
Lane Width		12.0	11.0				11.0	1		_	12	.0				11.0	<del>                                     </del>		
Parking/Gra	_	N	Ĭ	۲. ا	Ν	Ν	-6	Ν		N	-1		۸	I	Ν	-1	Ν		
Parking/Hou			_					_											
Bus Stops/Hour Minimum Pedestrian Time			0	_	0			0	$oldsymbol{oldsymbol{\perp}}$			- 0					0		
Phasing	EW Perm		02	17		03	1 ^	17.2	<u> </u>	<u></u>		20.	لــــــــــــــــــــــــــــــــــــــ	<u> </u>			7.5		
	G = 32.0	G:			03	0  G≃	4		NS Perm G = 27.0					G =	07	G =	)8		
Timing	Y ≃ 5.5	Υ =			Y =		Y =		) Y=		Ť	<u>Y</u> =		$\dashv$	Y =		Y=		
	Analysis (hrs)											Cycle	Len	gth	C =	70.0			
Lane Gro	up Capacit	y, C	ontro			y, and	LOS		min	<u>atic</u>	n					<del>-</del>	_		
Adjusted Flo	u Boto	_	EB 287 903			<del></del>	-	WB					NB	_		SB		<del>,</del>	
ane Group			339	1204				492 1213	+			$\neg$	481 943			├─-	600	+	
//c Ratio			0.85					0.44	+-			-				—	735	<u> </u>	
Green Ratio			0.65	0.4			_	0.41				_	0.51				0.82	<del> </del>	
Jniform Dela	w d.		16.8	14			<u> </u>	0.46 12.7	╁				0.39			<del> </del>	0.39	<del> </del>	
Delay Factor	<u> </u>		0.50	0.5			<del>      </del>	0.50	┿		_	-	16.4			<b> </b>	19.3	<del></del>	
ncremental Delay d <sub>2</sub>			22.2	╌	3			1.0	╁			_	0.50				0.50	<u> </u>	
PF Factor			1.000		000			1.000	+	-	_		2.0 1.000			<b></b> -	9.7 1.000		
Control Delay			39.0	17.0		_		13.7	†	$\dashv$	+-		18.4				29.0	<del> </del>	
ane Group I	.os	$\neg$	D B				В		$\dashv$		B	├──┼		—	<del></del>	C	<del> </del>		
Approach De	lay			Ц_	.3			13.7				18.	_		—	29.0			
oproach LO	s				<del>-</del>			В		$\dashv$					$\dashv$	29.0 C			
ntersection [	Delay	$\dashv$		21	.5				Inter	secti	ion I						C		
	Jniversity of Florid									/ersio									

Canavalda	- <b>F</b> = 1 = 1 = 1 = 1 = 1 = 1 = 1 = 1 = 1 =				s	HORT												
Analyst	nformation		WD.			Site	Site Information											
Agency or Date Perfo	040/4/6	VS AS 12/7	/2005			Area	Intersection  SIXTH AVE & ROSS  ST/BIGELOW  Area Type  CBD or Similar											
Time Perio		Jurisdiction CITY OF PITTSBURGH Analysis Year 2008 COMBINED																
Volume ar	nd Timing Inp	ut					2000 OCIVIDINED											
:		ŀ	LT	EB TH	RT		WB TH		1	NB	T ===		SB					
Number of	Lanes		0	3	0	0	2	RT 0	O	TH 2	RT 0	LT	TH	RT				
Lane Group	D			LTR	+-	+ -	LTR	+	+ -	LTR	-	0	2	0				
Volume (vp	oh)		104	637	25	7	267	46	41	56	367	38	LTR	100				
% Heavy V	ehicles		1	1	1	2	2	2	2	2	2	1	106	106				
PHF			0.82	0.82	0.82	0.93	0.93	0.93	0.77	0.77	0.77	0.87	0.87	0.87				
Pretimed/A	ctuated (P/A)		P	P	P	P	P	P	P	P	P	P	P	0.87				
Startup Los				2.0	<del>                                     </del>	<del>                                     </del>	2.0	<del>-                                    </del>	<del>                                     </del>	2.0	+	+	2.0	┼				
Extension of	of Effective Gre	een	_	2.0		<del></del>	2.0	<del>                                     </del>	<del> </del>	2.0		┼	2.0	<del> </del> -				
Arrival Type	<del></del>	$\neg$		3	<del>                                     </del>		3	<del> </del> -		3	<del> </del>	╁	3	├				
Unit Extens	ion	十		3.0			3.0	+	<del> </del>	3.0		├	3.0	<del>                                     </del>				
Ped/Bike/RTOR Volume			200	0	3	200	0	0	200	0	37	200	0	144				
Lane Width				11.0			11.0	┼┷	200	12.0	37	200	11.0	11				
Parking/Grade/Parking			N	5	N	N	-6	N	N	-1	l N	N	-1	N				
Parking/Hour										<del>                                     </del>			<del> </del>					
Bus Stops/Hour				0			0		-	0	_		0					
	edestrian Time		_	18.3			17.8			20.8		8.0						
Phasing	EW Perm		02		03		4	NS Per		06		07	_	8				
Γiming	G = 30.0 $Y = 5.5$	G = Y =		G = Y =		G= Y=				3 = / =	G = Y =							
Duration of	Analysis (hrs)			<del>-  </del>	_	<u> I.'                                   </u>		1 - 5.5		r = Cycle Ler		_	Y =					
ane Gro	up Capacit	y, C	ontro	l Dela	y, and	LOS	Deter	minatio	on	,	.3 0	7 0.0						
				EB			WB		T	NB		Τ	SB					
djusted Flo	w Rate			931			344	T ~		555	Ī	275						
ane Group	Capacity		1497				1215			923		-	991	<del>                                     </del>				
/c Ratio				0.62			0.28			0.60			0.28					
Preen Ratio			0.43				0.43			0.41		<del>                                     </del>	0.41					
Iniform Dela	ay d <sub>1</sub>	Ţ	15.6				13.0			16.0	<del> </del>		13.6	<del>                                     </del>				
elay Factor	·k		0.50				0.50			0.50		<del>                                     </del>	0.50	_				
ncremental	Delay d <sub>2</sub>		2.0						<del> </del>	2.9	<u> </u>	<del>                                     </del>	0.7	<u> </u>				
F Factor				1.000			1.000	1		1.000			1.000					
Control Delay				17.5			13.6			18.9		-	14.3					
ane Group I	LOS			В			В			В			B					
pproach De	elay			17.5			13.6			18.9	<u> </u>	14.3						
pproach LC	s	十		В			В			B		14.5 B						
itersection [	Delay	_		16.8				Intersect	ion I C		<del></del>							
pyright © 2005 University of Florida, All Rights Reserved								TM Version					B ed: 11/14/2006 10:45 AM					

						s	HORT	REP	ORT						<u> </u>	-		
General In	formation			Site Information														
Analyst Agency or ( Date Perfor Time Period	med		<u>-</u>	Intersection  SIXTH AVE & ROSS ST/BIGELOW Area Type CBD or Similar Jurisdiction CITY OF PITTSBURGH 2008 COMBINED CONDITIONS														
Volume an	d Timing Inpu	ıţ		<del></del>			CONDITIONS											
	EB											ΝB		SB				
Number of t	anes		LT O	7	TH_	RT 0	LT 0	TH 2	RT 0	L O	r   "	ГН	RT 0	LT	TH 2	RT 0		
Lane Group			<del>                                     </del>	L7		-	-	LTR	+-	<del>Ť</del>	-	r <sub>R</sub>	<del>                                     </del>	<del>                                     </del>	LTR	+ -		
Volume (vpl			84	48		8	5	232	28	44	<del></del>		278	36	96	99		
% Heavy Ve			1	1		1	2	2	2	1	-   -		1	1	1	1		
PHF			0.89	0.8	39	0.89	0.84	0.84	0.84	0.79			0.79	0.81	0.81	0.81		
Pretimed/Ad	tuated (P/A)		Р	F	,	P	P	P	P	P	F	,	P	P	P	P		
Startup Lost	Time			2.0				2.0	†		2.	0		<del>                                     </del>	2.0	<del>  ` </del>		
Extension of	f Effective Gre	en		2.	0			2.0	<del> </del>		2.			<del> </del>	2.0			
Arrival Type				3			-	3			1 3	}		†	3			
Unit Extensi	Unit Extension			3.	0		_	3.0			3.	0			3.0			
Ped/Bike/RTOR Volume			100	0		1	100	0	3	100	7	)	28	100	0	10		
Lane Width				11	.0			11.0			12	.0		<del> </del>	11.0			
Parking/Grade/Parking			N	5		N	Ν	-6	N	N			N	N	-1	Ν		
Parking/Hour			_					<u> </u>										
Bus Stops/H		4		0				0	ļ		(				0			
Phasing	destrian Time		02	17.	_	03	<del></del>	17.2	1000	<u> </u>	20		<u> </u>		7.5			
	G = 32.0	G		$\dashv$	G =	03	G =	4	NS Per G = 27		0 G =	6	G	07	G =	8		
Timing	Y = 5.5	Ÿ	=		Y =		Y =		Y = 5.5		Y =			<u>-</u>	Y =			
	Analysis (hrs)										Cycle	Len	gth C =	= 70.0				
Lane Gro	up Capacit	<u>, (</u>	Contro			y, and	LOS	•	<u>minati</u>	<u>on</u>				<del>,</del>				
Adjusted Fla	Data		EB			<u> </u>	<del> </del>	WB		ļ	NB			SB				
Adjusted Flo				648 1643		<u> </u>	<del> </del> -	312 1341			40	9	ļ	<del>  -</del> -	273	<del></del>		
Lane Group	Capacity			1043			<u> </u>	1341			89	6		920				
v/c Ratio	,		<u> </u>	0.3		ļ	<u> </u>	0.23			0.4	_	ļ	<u> </u>	0.30	<u> </u>		
Green Ratio			<u> </u>	0.4		<u> </u>	<u> </u>	0.46		<u> </u>	0.3				0.39	<u> </u>		
Uniform Delay d <sub>1</sub>				12.		<u> </u>	<u> </u>	11.5	—	_	16	_		ļ	14.9			
Delay Factor k			<del> </del>	0.5				0.50 0.4	-		0.5			<u> </u>	0.50	<del>                                     </del>		
Incremental Delay d <sub>2</sub>				0.7					<u> </u>			7		<b>↓</b>	0.8	<u> </u>		
PF Factor			<del> </del>	1.000 13.3			-	1.000	<del> </del>	<del> </del> —		00	<u> </u>	<del> </del>	1.000			
Control Delag		,		┿		-	-	11.9	╀	<del> </del>		17.7		-	15.7	<del> </del>		
			<del>                                     </del>	B		<u> </u>	<b> </b>	B	1	<del> </del>	B		L.	<del> </del>	В			
Approach De			├	13		<del></del>		11.9		<del>  -</del>		7		<del> </del>	15.7			
Approach LC			<u> </u>	E			<u> </u>	В		<u> </u>	E	3			В			
Intersection [	<u></u>	14	5		<u> </u>		Intersec	tion L	.os			В						

